

Appendix G

Traffic Impact Study

January 3, 2020

Prepared for



Prepared by





Draft Report

Hamilton Transit Maintenance Storage Facility Traffic Impact Study



Prepared for City of Hamilton
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1 Introduction

The City of Hamilton is seeking to build a new 430,000 ft² Maintenance Storage Facility (MSF) for a fleet of approximately 200 regular and articulated buses with a planned future expansion that will store an additional 100 buses. The MSF will include storage for the fleet, a maintenance garage with 20 bays, paint booth and body shop, two bus CNG fueling and washing lanes, stores, shipping and receiving spaces, administrative offices, reception, meeting rooms, and training spaces. Outdoor functions are also provided with plans to include vehicle circulation, CNG compressor station, fluids tank farm and generators, and a multi-level parking garage (“parkade”) for staff use.

1.1 Study Objective

The objective of the traffic impact study is to aid in the design and environmental assessment study of the MSF. The surrounding road network was analysed for traffic impacts of the new facility using an existing conditions year, 2019, and two horizon years: 2022 (post-build) and 2027 (5 years post-build).

2 Context

2.1 Study Area

The study area is located in the northeast section of the City of Hamilton. The proposed site is an irregular shaped parcel of land, bordered by Brant Street, Birch Avenue, Rosemary Avenue and Hillyard Street. City of Hamilton staff were consulted as to the scope of this traffic study and the study area. As shown in Exhibit 2-1, the City has confirmed the study area to include six intersections, which are as follows:

- Wentworth Street North & Brant Street
- Hillyard Street & Brant Street
- Wentworth Street North & Burlington Street East
- Wentworth Street North & Munroe Street
- Burlington Street East & Birch Avenue
- Birch Avenue & Brant Street

Birch Avenue is subject to future Environmental Assessment (EA) for two-way conversion, therefore intersections and accesses on Birch Avenue were not analyzed in detail.

Exhibit 2-1: Study Area



2.2 Land Use

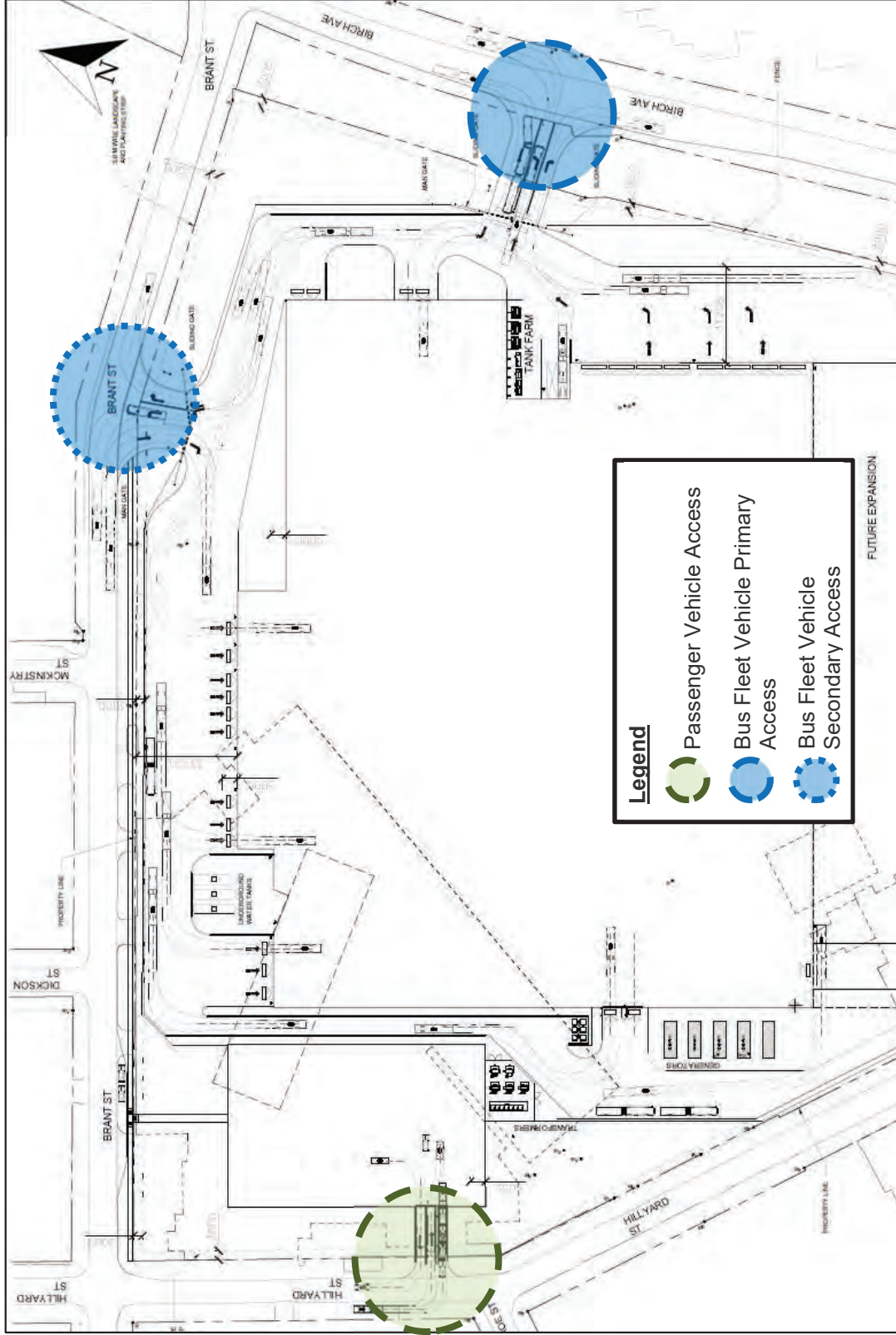
The proposed site area is currently zoned as M6 – for light industrial uses. The site was required to be rezoned due to the project spanning multiple lots and by-law zones. Rezoning is to be completed once property acquisition is finalized. Currently, the site is occupied by a steel fabrication factory as well as an operations centre for the City of Hamilton. Areas surrounding the site are either zoned as a mix of industrial and residential uses.

The site is proposed as a Maintenance Storage Facility (MSF), with capacity for maintenance space for 30 buses, storage for 200 buses, with the future possibility of adding further storage for 100 additional buses. The site will also accommodate a multi-level parking structure on the northwest corner with approximately 400-420 employees/visitor parking spaces.

2.3 Site Plan

The proposed site is to have three accesses; the main bus access on Birch Avenue, the secondary bus access on Brant Street, and the passenger vehicle access on Hillyard Street as shown in Exhibit 2-2. The passenger vehicle access location may lead to some site traffic accessing the site via residential streets.

Exhibit 2-2: Proposed Site Plan (dated April 16th, 2019)



2.4 Existing Road Network

The following provides a summary and review of the road network adjacent to the development site.

- **Burlington Street** is an east-west major arterial road. It connects central Hamilton to the Queen Elizabeth Way, a 400-series highway. Burlington Street is a two to four lane road and has a posted speed limit of 50 km/h from Wentworth Street N to McKinstry Street and a posted speed limit of 60 km/h from McKinstry to Sherman Avenue N within the study area. It predominantly serves employment areas.
- **Birch Avenue** is a southbound one-way minor arterial road. It connects Burlington Street to Wilson Street. Within the study area, Birch Avenue is a three lane road with an assumed speed limit of 50 km/h, and serves an employment area. Birch Avenue has the potential to be converted to a two-way street.
- **Barton Street** is an east-west minor arterial road. It connects central Hamilton to the community of Winona, located west of the City. Within the study area, Barton Street is a four lane road, and has an assumed speed limit of 50 km/h. It services neighbourhoods.
- **Wentworth Street** is a north-south minor arterial road located. It has a four lane cross section, and has an assumed speed limit of 50 km/h within the study area. It connects the Hamilton Harbour at its north end and turns into Charlton Avenue East at its south end. Wentworth Street mostly services employment locales within the study area.
- **Brant Street** is a local east-west two lane road. It connects Wentworth Street to Sherman Avenue, both minor arterial roads. It serves an employment area and has an assumed speed limit of 50 km/h.
- **Niagara Street** is a local north-west two lane road that serves employment and residential areas. A speed limit of 50 km/h is assumed.
- **Hillyard Street** is a local north-west two lane road that serves an employment area. A 50 km/h speed limit is assumed.
- **Munroe Street** is a local east-west two lane road. It has an assumed speed limit of 50 km/h and serves an employment area, as well as a small section zoned for residential use.

Hamilton Street Railway (HSR) operates two bus routes within the study area. Details are provided below:

- **Route 12 (Wentworth)** – travels in the south direction along Wentworth Street with stops at intersections of Burlington Street, Mars Avenue and Burton Street. Service runs only on weekdays from 6:30 AM to 7:30 PM. The route is served every 30 minutes.
- **Route 4 (Bayfront)** – travels in both the east and west direction along Burlington Street with stops at Wentworth Street, Hillyard Street, McKinstry Street, and Birch Avenue. Service runs on weekdays, weekends and holidays with schedule service every 15 minutes during peak hours and every 30 minutes during off-peak hours. Service runs from approximately 5:00 AM to 2:00 AM the next day.

3 Existing Conditions

3.1 Traffic Volumes

Traffic volumes were obtained from the City of Hamilton. Exhibit 3-1 provides a summary of the date at which the counts were undertaken for each study intersection. As shown, the intersections are dated between years 2014 to 2019. As per City of Hamilton’s guidelines, a few of these TMCs are considered old (> 2years). Given that volumes and expected growth are low, a 2% compound annual rate was applied to scale volumes to a consistent base year (i.e. 2019). These scaled up values are expected to be reflective of existing conditions and are summarized in Exhibit 3-3. TMC reports are provided in Appendix A.

Exhibit 3-1: Data Collection Summary Table

| INTERSECTION | CONTROL TYPE | COUNT DATE |
|---|--|--------------|
| Wentworth Street North & Brant Street | T-intersection <i>(stop control on minor approach)</i> | 2016-09-20 |
| Brant Street & Hillyard Street | TWSC | 2019-03-06 |
| Wentworth Street & Burlington Street East | Signalized | 2016-09-21 |
| Wentworth Street North & Munroe Street | T-intersection* <i>(stop control on minor approach)</i> | 2017-09-17 |
| Birch Avenue & Burlington Street East | Signalized | 2014-12-08** |
| Birch Avenue & Brant Street | Signalized | 2018-04-25** |

*Currently operates with an intersection pedestrian signal (IPS) – City staff reported that the signal is to be removed in 2019

**Given the future EA of Birch Avenue two-way conversion, Birch Avenue intersections were not analyzed in detail. Analysis of Birch Avenue intersections subject to future EA study.

3.2 Existing Traffic Operations

Intersection operations analysis was conducted using Synchro (version 9) and following Highway Capacity Manual (HCM 2000) methodologies of intersection analysis. Analysis periods were limited to the weekday AM and PM peak hours, when general background traffic is considered highest.

All critical traffic movements are identified with the following conditions (from City’s TIS guidelines):

- For signalized intersections,
 - Volume-to-capacity (v/c) ratios for through movements or shared through/turning movements will operate at 0.85 or greater (0.85 is considered the maximum acceptable level-of-service for these movements);
 - V/C ratios for exclusive turning movements increase to 0.90 or greater (0.90 is considered the maximum acceptable level-of-service for these movements);
 - Queues for an individual movement are projected to exceed available turning lane storage at 95th percentile volumes.
- For unsignalized intersections,
 - Level-of-service, based on average delay per vehicle or individual movements is LOS ‘D’ or greater;

- The estimated 95th percentile queue length for an individual movement exceeds the available queue storage.

Level-of-service (LOS) is a measure of performance based on the control delay, as defined in Exhibit 3-2

Exhibit 3-2: Intersection LOS Reference

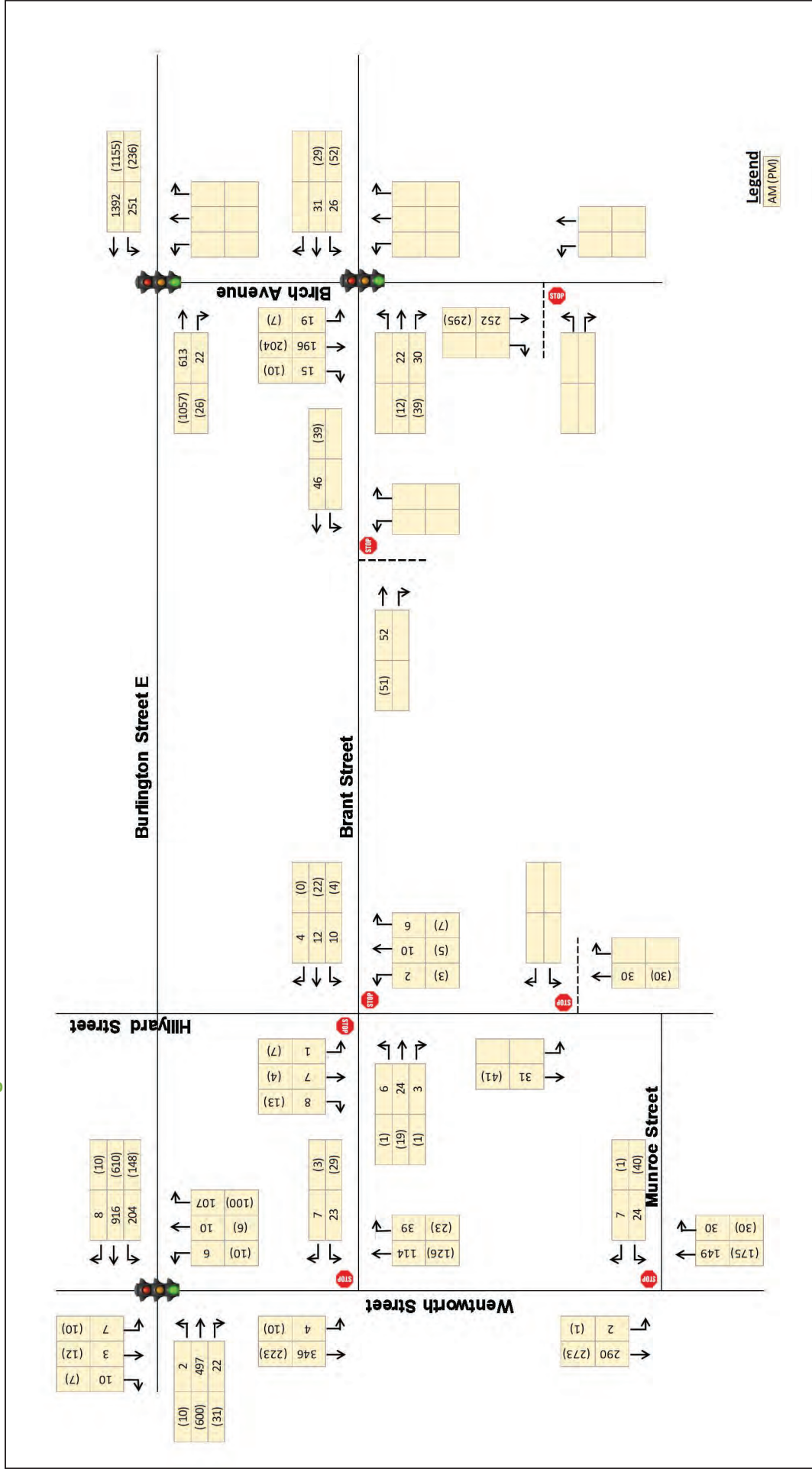
| HCM LOS | CONTROL DELAY PER VEHICLE (S) | |
|------------|-------------------------------|--------------|
| | Signalized | Unsignalized |
| A | ≤10 | ≤10 |
| B | >10 and ≤20 | >10 and ≤15 |
| C | >20 and ≤35 | >15 and ≤25 |
| D | >35 and ≤55 | >25 and ≤35 |
| E | >55 and ≤80 | >35 and ≤50 |
| F | >80 | >50 |

Default parameter values listed in the City of Hamilton TIS guidelines were used. This includes an ideal saturation rate of 1900 vehicles per hour, peak hour factor of 0.92, lane width of roads of 3.3m.

Operational concerns or deficiencies noted in the studied horizon years are identified and addressed through recommendations and potential mitigation measures and/or operational improvements.

For existing traffic operations, a summary of the analysis for the AM and PM peaks is found in Exhibit 3-4 with full Synchro outputs provided in Appendix B. Based on the results, all intersections in the study area currently operate well, with the signalized intersections operating at LOS B or better. No intersections, signalized or unsignalized, experience any critical movements in either peak periods, indicating stable and free-flow traffic conditions.

Exhibit 3-3: 2019 Existing Conditions Traffic Volumes



Note: Not to scale

Exhibit 3-4: Existing Traffic Analysis (All Movements) Summary

| Intersection Name | Control Type | Int LOS | All Movements | | | | |
|--------------------------------|--------------|---------|---------------|-----|-----------|-----------|---------------------------------------|
| | | | Mvmt | LOS | Delay (s) | V/C Ratio | 95 th Percentile Queue (m) |
| AM PEAK | | | | | | | |
| Wentworth St N/Burlington St E | Signalized | B | EBL | C | 20 | 0.01 | 2 |
| | | | EBTR | C | 26 | 0.53 | 56 |
| | | | WBL | B | 15 | 0.47 | 30 |
| | | | WBTR | B | 16 | 0.60 | 77 |
| | | | NBTLR | C | 21 | 0.07 | 7 |
| | | | SBTLR | C | 21 | 0.03 | 6 |
| Burlington St E/ Birch Avenue | Signalized | A | EBTR | B | 17 | 0.24 | 42 |
| | | | WBL | C | 28 | 0.33 | 30 |
| | | | WBT | A | 0 | 0.33 | - |
| Birch Avenue/ Brant St | Signalized | A | EBTR | B | 14 | 0.06 | 9 |
| | | | WBTL | B | 15 | 0.09 | 13 |
| | | | SBTLR | A | 8 | 0.13 | 21 |
| Wentworth St N/ Brant St | Unsignalized | - | WBLR | B | 11 | 0.05 | 1 |
| | | | SBTL | A | 0 | 0.00 | 0 |
| | | | NBTR | A | 0 | 0.00 | 0 |
| Brant St/ Hillyard St | Unsignalized | - | EBTLR | A | 1 | 0.00 | 0 |
| | | | WBTLR | A | 3 | 0.01 | 0 |
| | | | NBTLR | A | 9 | 0.02 | 1 |
| | | | SBTLR | A | 9 | 0.02 | 1 |
| Munroe St/ Wentworth St N* | Unsignalized | - | WBLR | B | 11 | 0.05 | 1 |
| | | | SBTL | A | 0 | 0.00 | 0 |
| | | | NBTR | A | 0 | 0.00 | 0 |
| PM PEAK | | | | | | | |
| Wentworth St N/Burlington St E | Signalized | B | EBL | B | 19 | 0.05 | 5 |
| | | | EBTR | C | 27 | 0.62 | 69 |
| | | | WBL | B | 15 | 0.41 | 22 |
| | | | WBTR | B | 13 | 0.39 | 45 |
| | | | NBTLR | C | 21 | 0.07 | 7 |
| | | | SBTLR | C | 21 | 0.06 | 9 |
| Burlington St E/ Birch Avenue | Signalized | A | EBTR | B | 14 | 0.38 | 59 |
| | | | WBL | C | 30 | 0.35 | 30 |
| | | | WBT | A | 0 | 0.29 | - |
| Birch Avenue/ Brant St | Signalized | A | EBTR | B | 12 | 0.04 | 7 |
| | | | WBTL | B | 13 | 0.13 | 16 |
| | | | SBTLR | A | 6 | 0.13 | 13 |
| Wentworth St N/ Brant St | Unsignalized | - | WBLR | B | 11 | 0.05 | 1 |
| | | | SBTL | A | 1 | 0.01 | 0 |
| | | | NBTR | A | 0 | 0.00 | 0 |
| Brant St/ Hillyard St | Unsignalized | - | EBTLR | A | 0 | 0.00 | 0 |
| | | | WBTLR | A | 1 | 0.00 | 0 |
| | | | NBTLR | A | 9 | 0.02 | 0 |
| | | | SBTLR | A | 9 | 0.03 | 1 |
| Munroe St/ Wentworth St N* | Unsignalized | - | WBLR | B | 11 | 0.07 | 2 |
| | | | SBTL | A | 0 | 0.00 | 0 |
| | | | NBTR | A | 0 | 0.00 | 0 |

*Currently operates with an intersection pedestrian signal (IPS) – City staff reported that the signal is to be removed in 2019 – modelled as ‘stop control’

4 Site Traffic

For this section, site peak periods refers to when site traffic is expected to be highest. Background peak periods represents when general background traffic for the road network is busiest. Analysis for passenger and bus fleet vehicle trips are limited to background peak periods only (7-9 AM & 4-6 PM).

4.1 Trip Generation

4.1.1 Passenger Vehicle Trips

Because the planned operation of the facility is known, staff numbers and types of operation provide a basis for trip generation that is better represented than ITE rates which depends on square footage and other metrics. Projected employee numbers were provided from the final Space Program (version 7). Staffing numbers are summarized below in Exhibit 4-1.

Exhibit 4-1: Staff Operations for MSF (Final Space Program v7)

| DESCRIPTION | STAFF NUMBERS |
|--------------------------|---------------|
| Bus Operators | 640 |
| Operations Support Staff | 27 |
| Transit Support Services | 4 |
| Fleet Maintenance | 140 |
| Stores | 3 |
| Facility Operations | 3 |
| Total | 817 |

Considering the use of the facility, the traffic forecasts were separated into 4 categories; staff positions where 1 shift was expected, staff positions where 2 shifts were expected, bus operators and bus fleet vehicles. The fourth category (i.e. bus fleet vehicles) is discussed separately in Section 4.1.2. It was assumed that each employee would use a vehicle due to the fact that many of the employees cannot utilize transit as they are the transit operators and thus would need to arrive outside transit operation times (off-peak / night periods). This may be conservative as all the employees are bus operators and some may have to take transit or use alternative modes of travel. The following assumptions were used to convert employees to trips during the two peak hours.

- Administration and support staffs (single shifts) were assumed to enter during the AM peak and leave during the PM peak (assumed 30 vph);
- Positions where two shifts were required such as transit maintenance, it was assumed that the first shift would arrive during the AM peak hour and leave during the PM peak hour, while a second shift would enter during the PM peak hour and leave outside the peak hours later that evening (assumed 45 vph). A small portion (10 vph) is assumed to enter during off peak and leave in the AM peak. This is consistent with the mechanic/maintenance shift times provided by the City (7 AM-3 PM, 3 PM-11 PM and 11 PM-7 AM);
- Majority of bus operators are expected to arrive and leave during the off peak. During the background peak periods, passenger vehicles from the bus operators

are expected to be the reverse of the bus fleet vehicles entering and leaving the site – refer to Section 4.1.2.

From the above assumptions, a summary of the total traffic generated by the site for passenger vehicles is provided in Exhibit 4-2. Total passenger vehicle two-way trip is approximately 94 in the AM peak hour and 134 in the PM peak hour. Majority of site generated traffic is expected outside of typical peak hours and will marginally impact the local road network.

Exhibit 4-2: Site Generated Trips (Passenger Vehicle Trips during Background Peak hour)

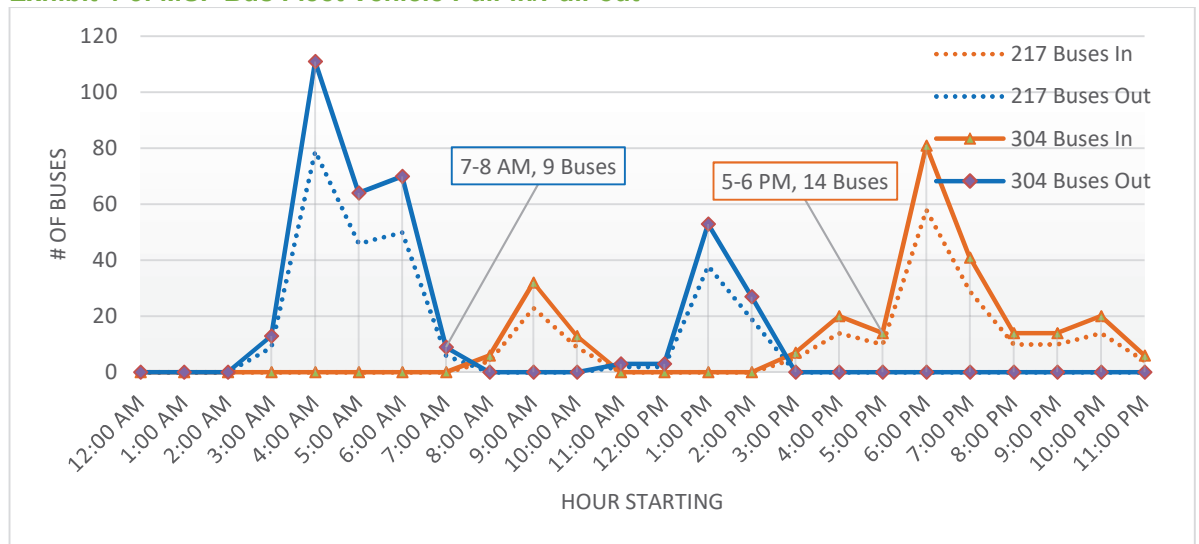
| TRAFFIC VOLUME GENERATOR | AM ENTER | AM EXIT | PM ENTER | PM EXIT |
|---------------------------------|-----------|-----------|-----------|-----------|
| Single Shift Positions | 30 | | | 30 |
| Two Shift Positions | 45 | 10 | 45 | 45 |
| Bus Operators* | 9 | | | 14 |
| Total Passenger Vehicles | 84 | 10 | 45 | 89 |

*Bus fleet vehicle trips (refer to Section 4.1.2)

4.1.2 Bus Fleet Trips

Based on existing bus routes, the Hamilton Street Railway (HSR) provided a service chart which represents required number of buses needed on the City’s roadways during a typical weekday. The highest number required is 217 buses and occurs prior to the afternoon background traffic PM peak at 3:01 PM. To estimate generated bus fleet vehicle trips during background peak, the difference in bus levels reflects the buses pulling in or out of the facility. The values corresponding to the background peak periods were used and scaled up by a factor of 1.401 (304 / 217) to represent a future build-out condition of 304 single bus equivalence (SBE) capacity (value considering the future expansion of 100 additional buses). A summary of the results is presented in Exhibit 4-3.

Exhibit 4-3: MSF Bus Fleet Vehicle Pull-in/Pull-out



From the above, a total of 9 and 14 bus vehicles are expected to be generated during AM and PM background peak hours (7-8 AM & 5-6 PM) respectively. This low volume is anticipated as majority of bus fleet vehicles are in operation around the City during this time. These vehicles are expected to minimally affect the local road network during peak times.

4.2 Trip Distribution

As shown in the site plans, three accesses are proposed; the main bus access on Birch Avenue, the secondary bus access on Brant Street, and the passenger vehicle access on Hillyard Street. Trips are distributed through the road network based on shortest path method.

Directional percentage split of site passenger vehicle traffic was developed through manual review of routes to and from nearby arterials and access to the rest of the city. Passenger vehicles are assumed to access/egress the site at the Hillyard Street (parkade) entrance only. This distribution also considers HSR employees using Birch Avenue to access the facility in an effort to reduce cut through traffic to surrounding neighborhoods (discussed in Section 6.2). The general direction of origin and destination for inbound and outbound trips was assumed to be that indicated in Exhibit 4-4. The resultant passenger vehicle site traffic distribution for the AM and PM peak hours is illustrated in Exhibit 4-6. The assigned trips is illustrated in Exhibit 4-7.

Exhibit 4-4: Passenger Vehicle Traffic Splits Per Direction

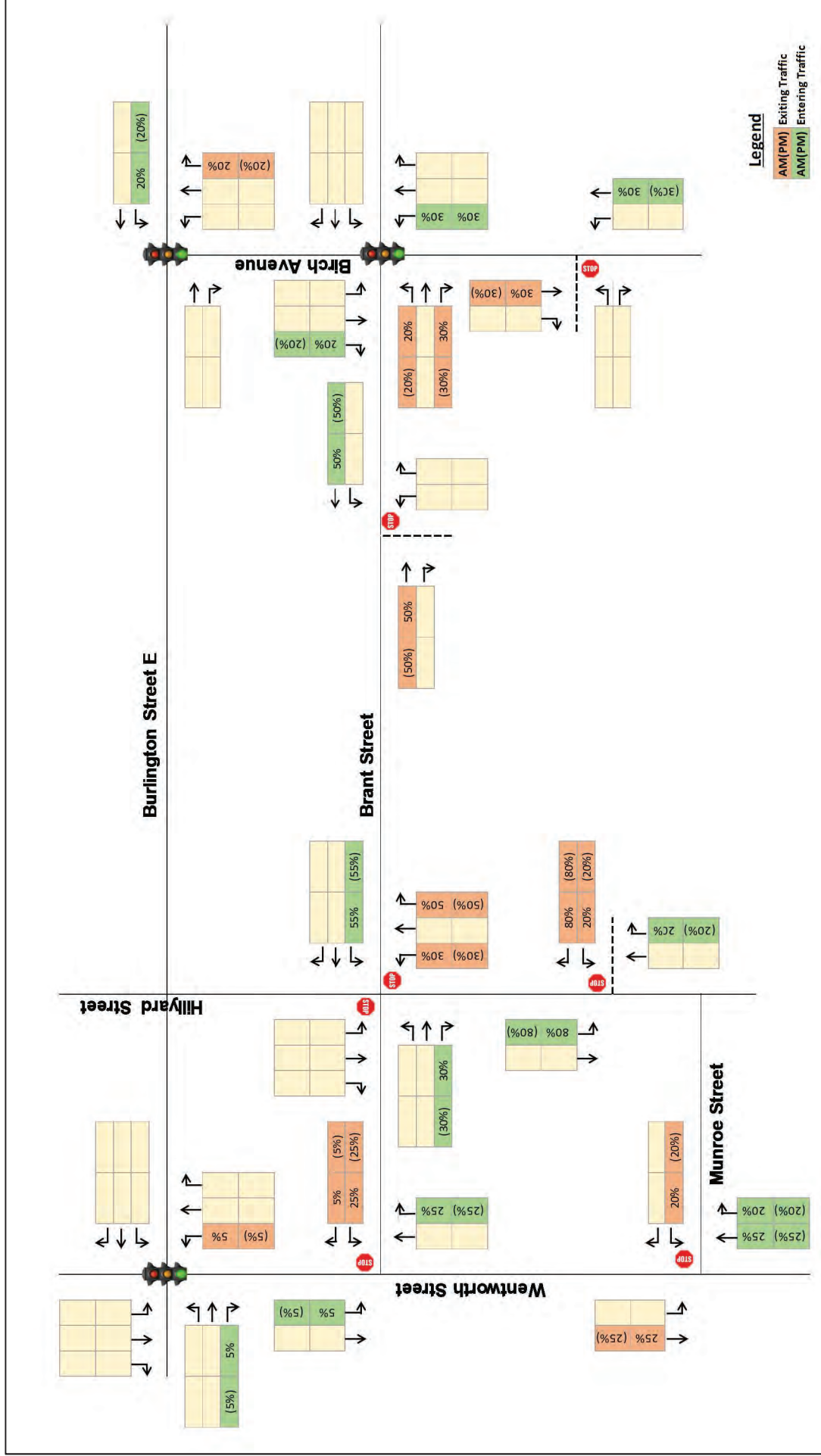
| ORIGIN/DESTINATION | PERCENT DISTRIBUTION | | | |
|---|----------------------|-----|--------------|-----|
| | AM Peak Hour | | PM Peak Hour | |
| To / From the North | In | Out | In | Out |
| via Birch Avenue / Burlington Street East | 10% | 10% | 10% | 10% |
| To / From the South | In | Out | In | Out |
| via Wentworth Street North | 45% | 45% | 45% | 45% |
| via Birch Avenue | 30% | 30% | 30% | 30% |
| To / From the East | In | Out | In | Out |
| via Birch Avenue / Burlington Street East | 10% | 10% | 10% | 10% |
| To / From the West | In | Out | In | Out |
| via Wentworth Street North / Burlington Street East | 5% | 5% | 5% | 5% |

Bus traffic splits follows a different distribution due to the fact that buses operate on fixed routes. Given that the ultimate operations of the facility will be up to HSR, buses would likely utilize the access that has the least number of left-turn movements and be oriented towards collector/arterial roads (i.e. Birch Avenue). These distributions also assumes that Birch Avenue will be converted to two-way. Lastly, it is assumed that majority of the bus trips are concentrated towards the main bus access (i.e. Birch Avenue). With input from City’s planning department, the general direction of trips originating from and destining to the site are that indicated in Exhibit 4-5. The resultant bus trip distribution for AM and PM is illustrated in Exhibit 4-8. The assigned trips is illustrated in Exhibit 4-9.

Exhibit 4-5: Bus Percentage Splits Per Direction

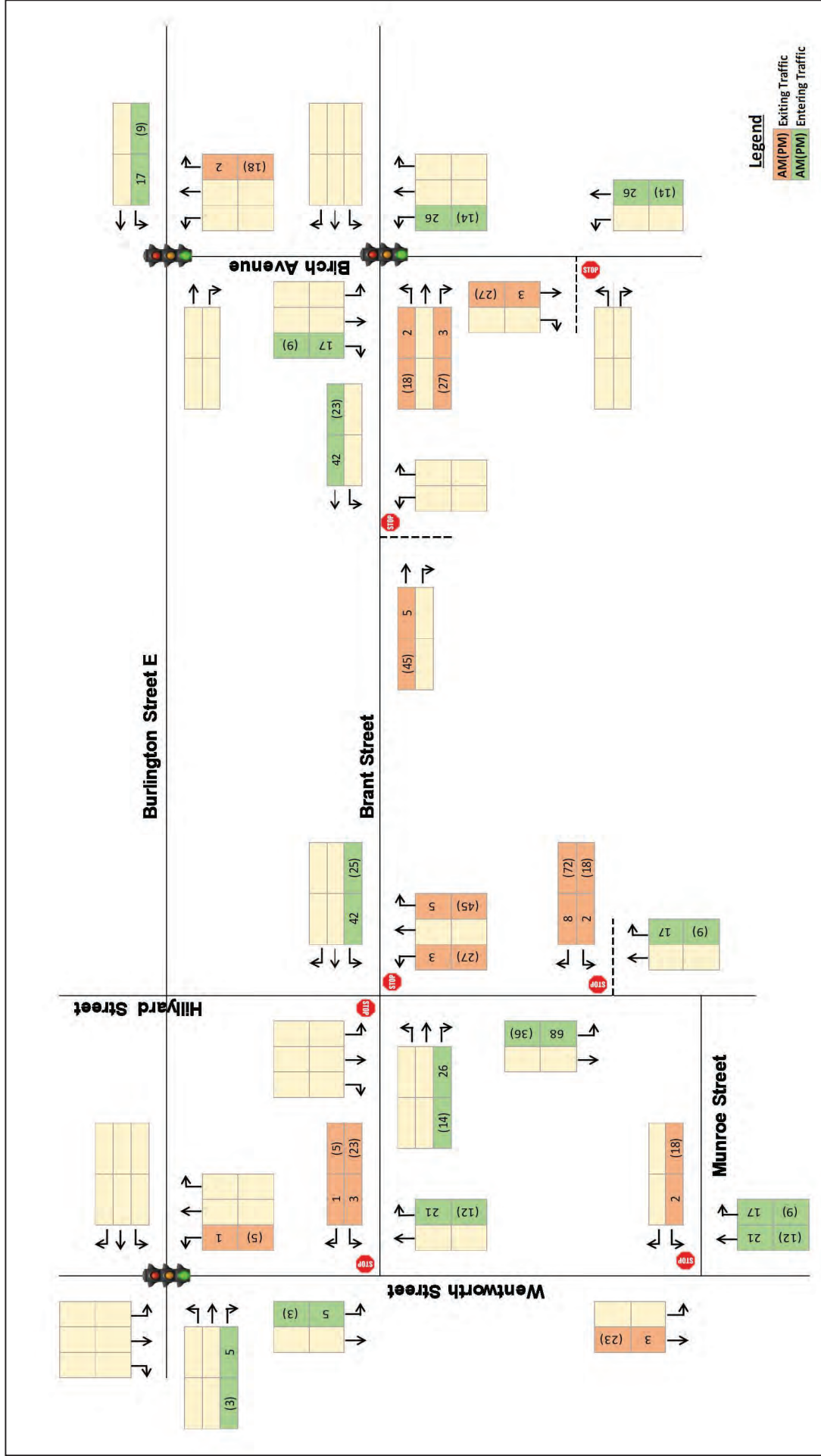
| ORIGIN/DESTINATION | PERCENT DISTRIBUTION | | | |
|---------------------|----------------------|-----|--------------|-----|
| | AM Peak Hour | | PM Peak Hour | |
| To / From the North | In | Out | In | Out |
| via Birch Avenue | 14% | 14% | 13% | 13% |
| To / From the South | In | Out | In | Out |
| via Birch Avenue | 86% | 86% | 87% | 87% |

Exhibit 4-6: Passenger Vehicle Site Distribution



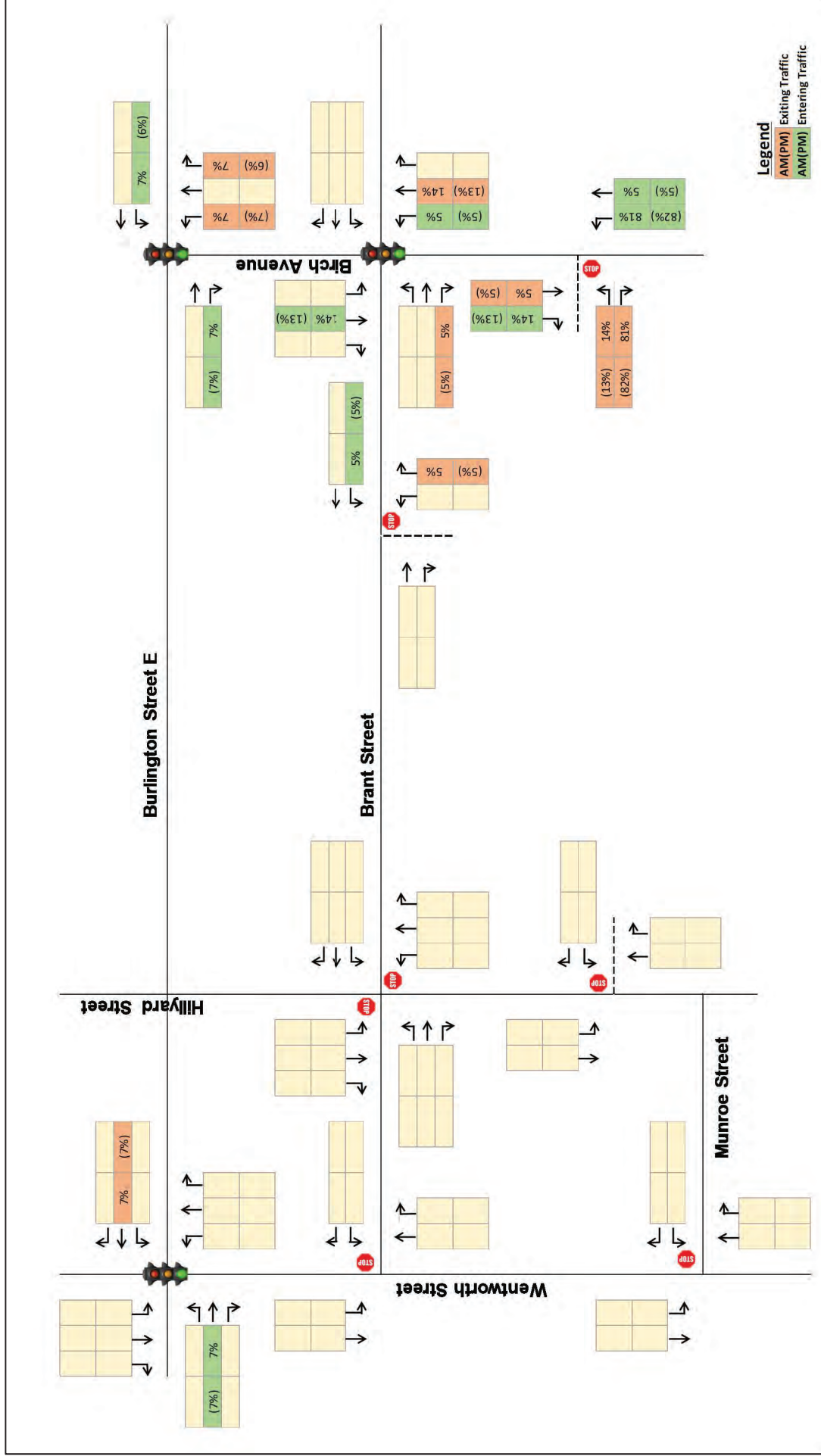
Note: Not to scale, rounded values

Exhibit 4-7: Passenger Vehicle Site Generated Trips



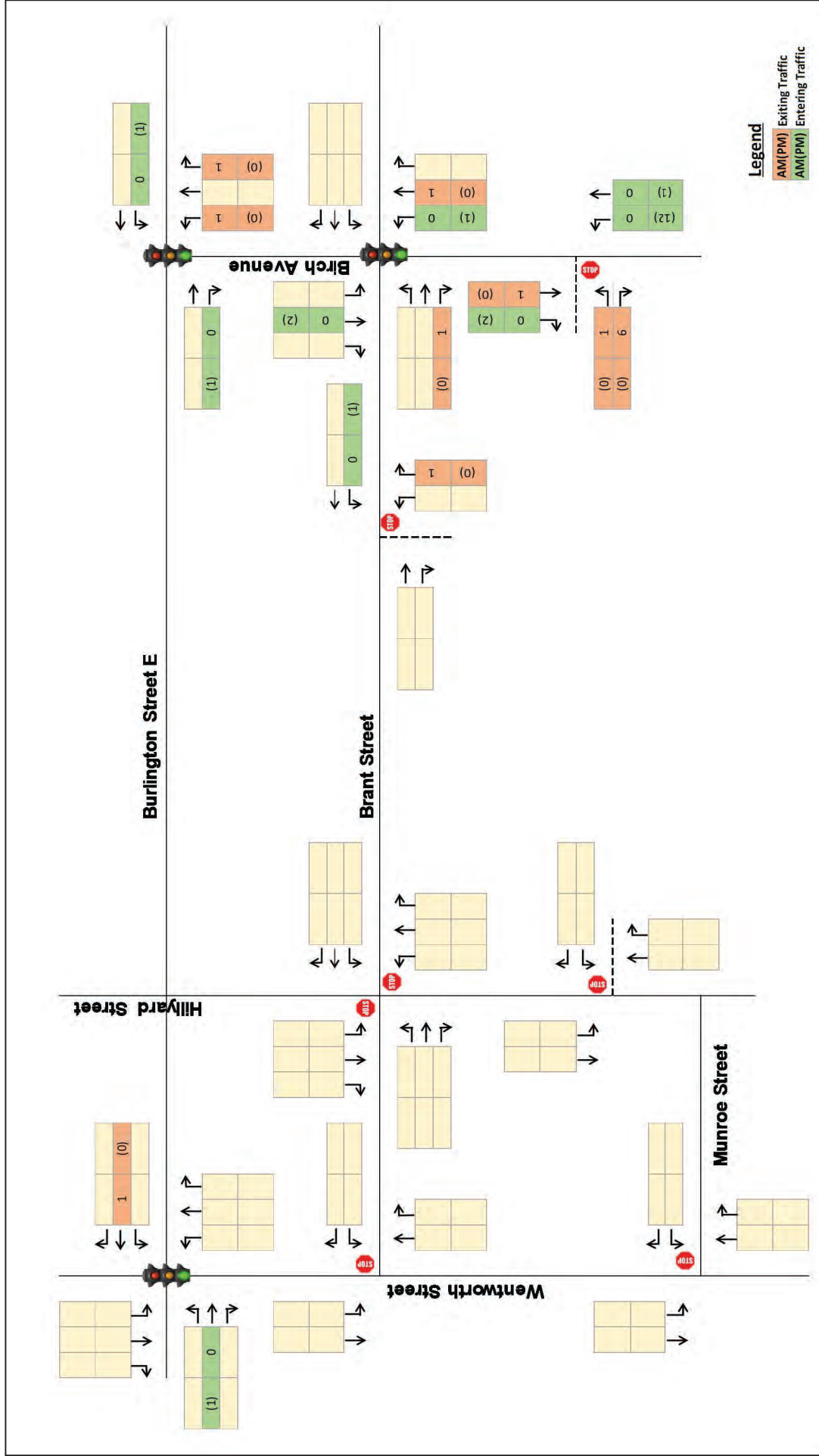
Note: Not to scale, rounded values

Exhibit 4-8: Bus Fleet Vehicle Site Distribution



Note: Not to scale, rounded values

Exhibit 4-9: Bus Fleet Vehicle Site Generated Trips



Note: Not to scale, rounded values

5 Future Conditions

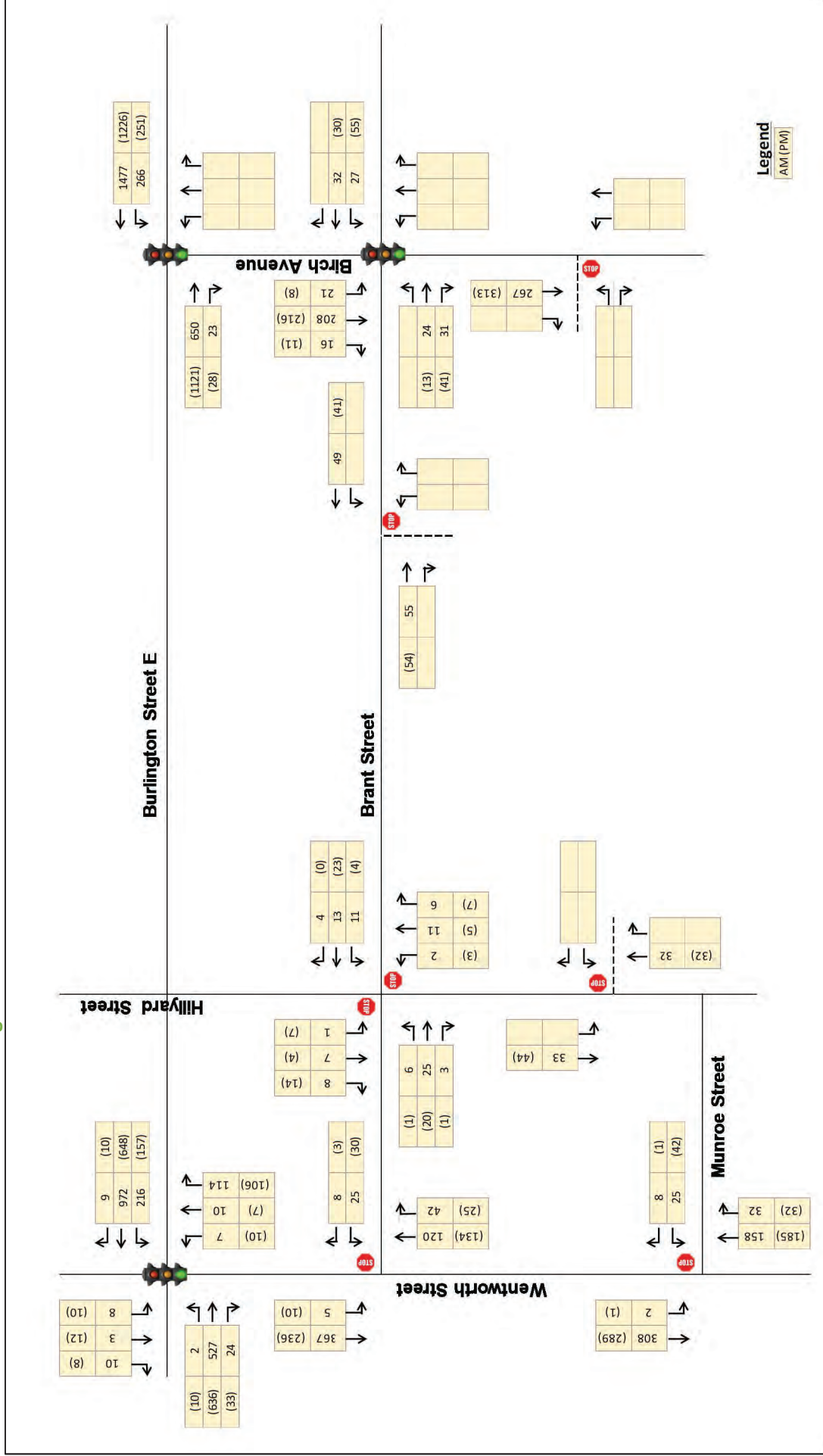
This section discusses the growth rate and future traffic operations under the 2022 and 2027 horizon years.

5.1 Traffic Volumes

To estimate future background growth (i.e. growth not accounted from the development), a 2% was applied to existing volumes. This equates to a growth of approximately 6% and 17% to years 2022 and 2027, respectively.

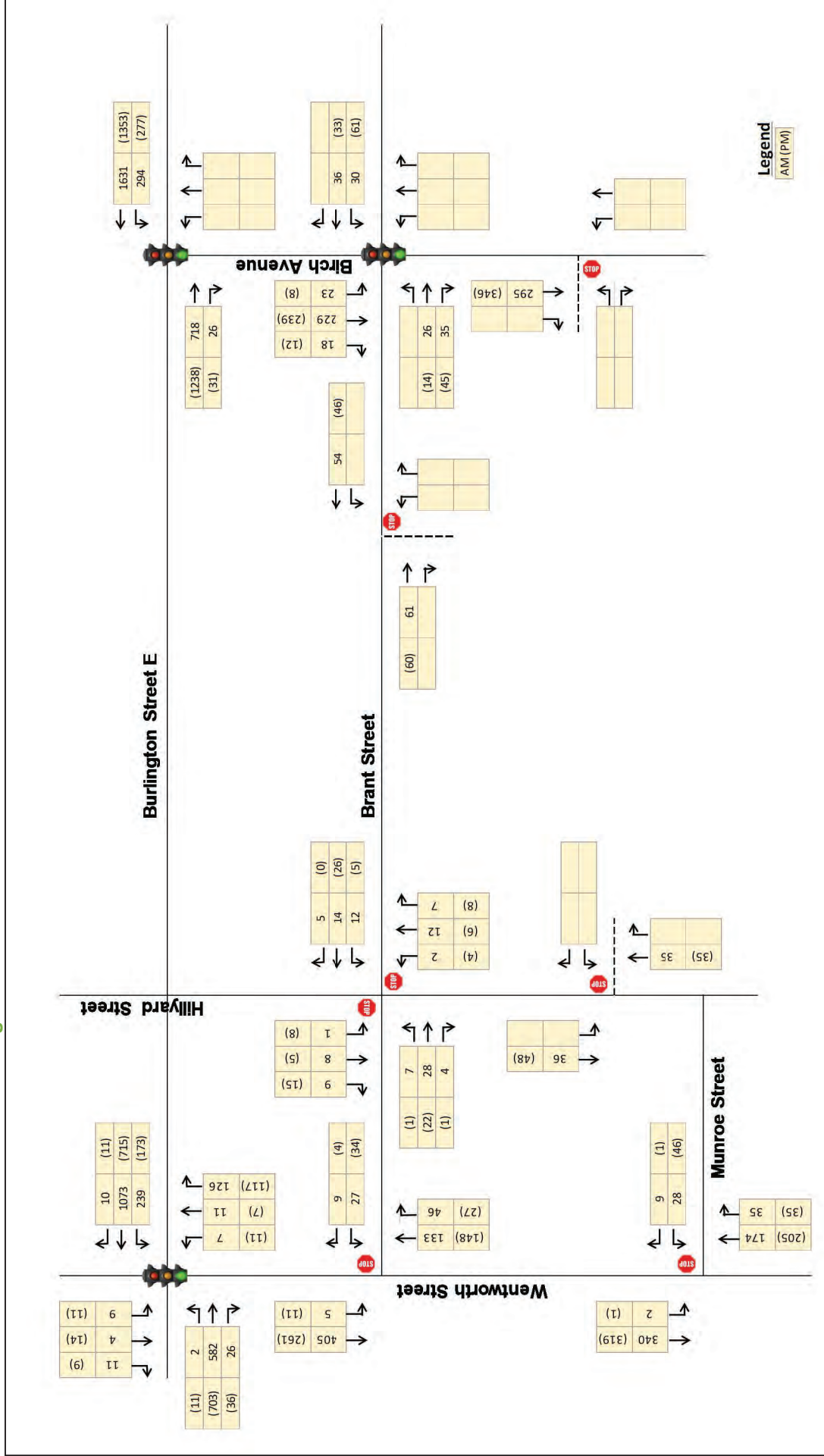
The rate was determined through a background review of historical counts with results indicating low growth. However, given the uncertainty with the conversion of Birch Avenue to two-way operation with potential traffic diversion through the study area, a 2% was considered appropriate. This rate is also endorsed in the City's *Traffic Impact Study Guideline (2009)* and was therefore carried forward for analyses. The traffic volumes for background conditions are shown in Exhibit 5-1 and Exhibit 5-2. With the added site development traffic (discussed in Section 4), the future total volumes are shown in Exhibit 5-3 and Exhibit 5-4.

Exhibit 5-1: 2022 Future Background Traffic Volumes



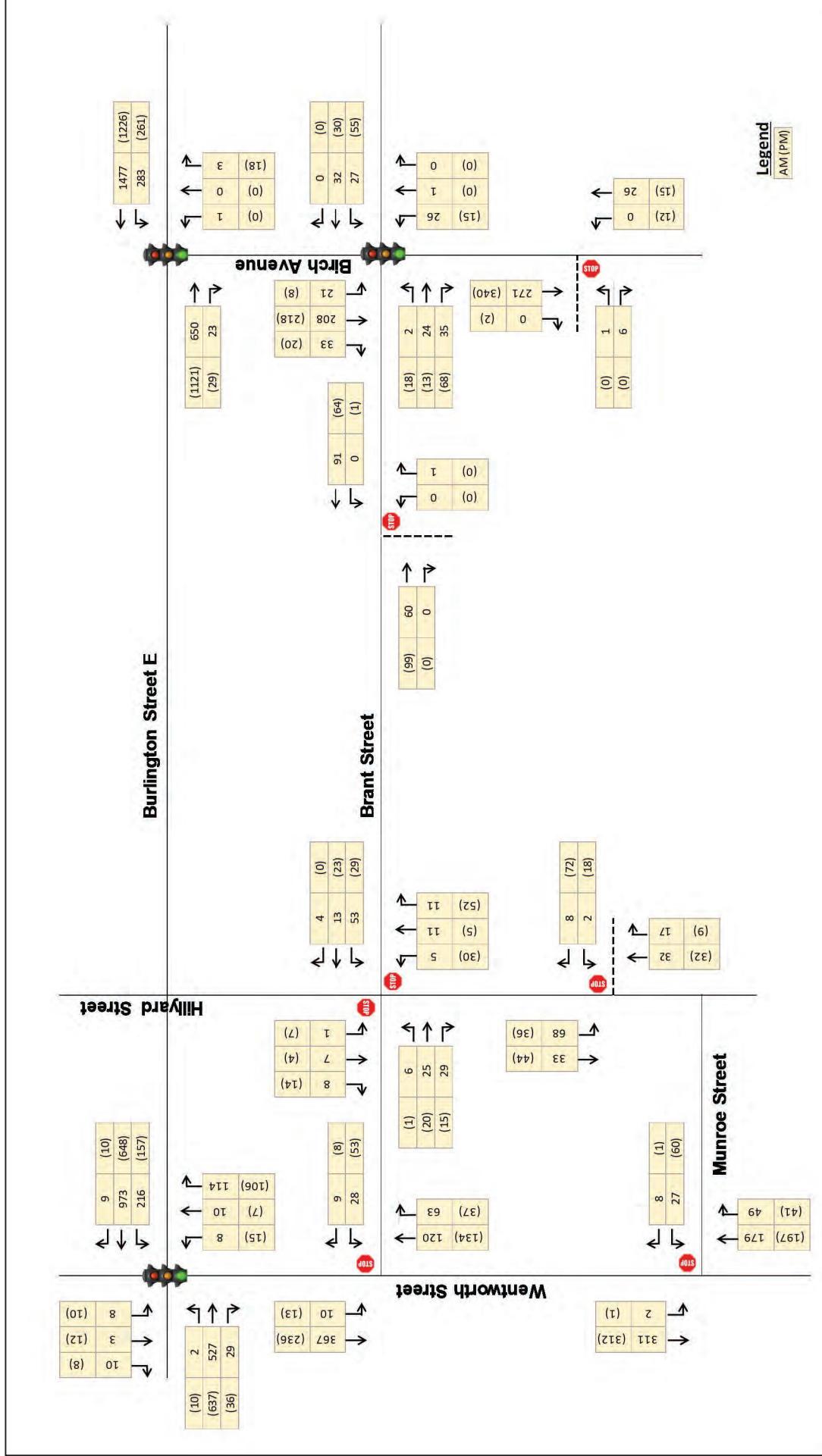
Note: Not to scale, rounded values

Exhibit 5-2: 2027 Future Background Traffic Volumes



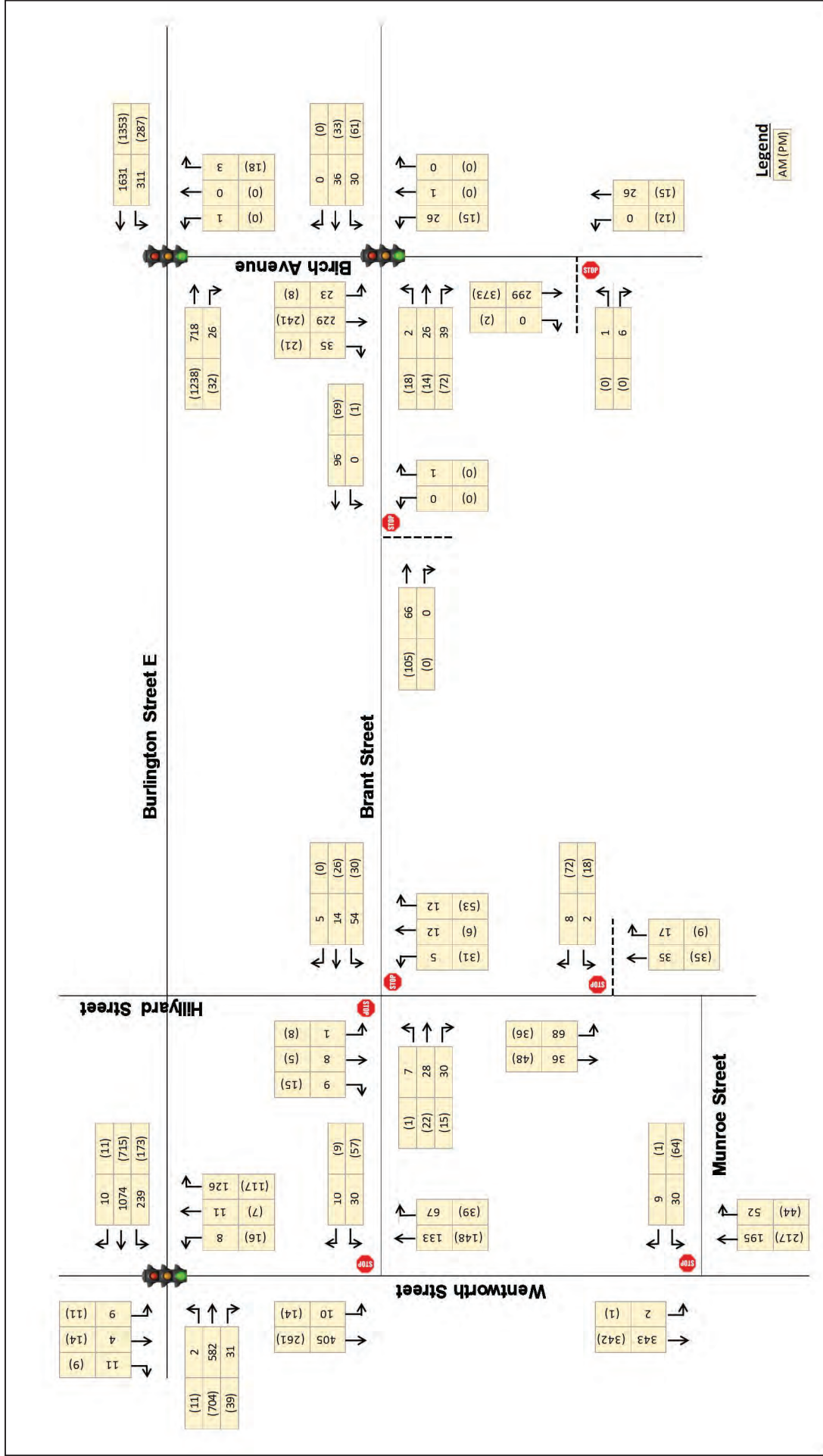
Note: Not to scale, rounded values

Exhibit 5-3: 2022 Future Total Traffic Volumes



Note: Not to scale, rounded values

Exhibit 5-4: 2027 Future Total Traffic Volumes



Note: Not to scale, rounded values

5.2 2022 Future Background

Exhibit 5-5 summarize the signalized and unsignalized operations, respectively during the AM and PM peaks. Future background Synchro reports are provided in Appendix C.

With the increased traffic volumes from background growth, the study area intersections are expected to operate with sufficient residual capacity during the AM and PM peak periods at LOS B or better. When compared to existing traffic conditions, delay increases by 1-2 seconds for individual/shared movements and are expected to marginal. The intersection of Birch Avenue and Brant Street deteriorates slightly in the AM peak and now operates at LOS B.

Overall, all study intersections continue to operate well with no critical movements or capacity concerns.

5.3 2022 Future Total

Exhibit 5-6 summarize the signalized and unsignalized intersection operations, respectively during the AM and PM peaks. Future total Synchro reports are provided in Appendix D.

As stated in Section 2.5, the study intersections on Birch Avenue have been analyzed as existing (southbound direction only). Although not modelled, northbound site traffic on Birch Avenue is expected to minimally affect the three intersections as they all have sufficient capacity and is currently operating well at LOS B or better. It is also noted that the generated site, particularly along Birch Avenue are low during the busy periods of the local road network.

To evaluate the impact of the proposed development has on the remainder of the study area network, the 2022 future total operations are compared to the 2022 future background operations. The following changes have been observed:

- In general, traffic operations marginally deteriorate due to additional trips generated by the MSF. During the PM peak, the Wentworth Street & Burlington Street intersection deteriorates slightly from LOS B to LOS C, however, sufficient capacity is provided. Delay at all other intersections increases slightly by 1-2 seconds; and
- For the site accesses, all individual/shared movements operate well with LOS B or better. During both peak periods, a few exiting movements now operate at LOS B with delays up to 10 seconds. This indicates good operation.

In summary, all study intersections continue to operate well with no critical movements or capacity concerns.

5.4 2027 Future Background

Exhibit 5-7 summarize the signalized and unsignalized intersection operations, respectively during the AM and PM peaks. Future background Synchro reports are provided in Appendix E.

With the increased traffic volumes from background growth, the study area intersections are expected to operate with sufficient reserve capacity during the AM and PM peak periods operating at LOS C or better. When compared to existing traffic conditions, delay increases by about 1-3 seconds for individual/shared movements and are expected to marginal.

Overall, all study intersections continue to operate well with no critical movements or capacity concerns.

5.5 2027 Future Total

Exhibit 5-8 summarize the signalized and unsignalized intersection operations, respectively during the AM and PM peaks. Future total Synchro reports are provided in Appendix F.

As stated in Section 2.5, the study intersections on Birch Avenue have been analyzed as existing (southbound direction only). Although not modelled, northbound site traffic on Birch Avenue is expected to minimally affect the three intersections as they all have sufficient capacity and is currently operating well at LOS B or better. It is also noted that the generated site, particularly along Birch Avenue are low during the busy periods of the local road network.

To evaluate the impact of the proposed development has on the remainder of the study area network, the 2027 future total operations are compared to the 2027 future background operations. The following changes have been observed:

- In general, traffic operations marginally deteriorate due to additional trips generated by the MSF development. Delays for individual/shared movements for the study intersections increase by 1-2 seconds; and
- For the site accesses, all individual/shared movements operate well with LOS B or better. During both peak periods, a few exiting movements now operate at LOS B with delays up to 10 seconds. This indicates good operation.

In summary, all study intersections continue to operate well with no critical movements or capacity concerns.

Exhibit 5-5: 2022 Future Background Traffic Analysis Summary

| Intersection Name | Control Type | Int LOS | All Movements | | | | |
|--------------------------------|--------------|---------|--|----------------------------|----------------------------------|--|---------------------------------------|
| | | | Mvmt | LOS | Delay (s) | V/C Ratio | 95 th Percentile Queue (m) |
| AM PEAK | | | | | | | |
| Wentworth St N/Burlington St E | Signalized | B | EBL EBTR WBL WBTR NBTLR SBTLR | C C B B C C | 20 27 16 16 21 21 | 0.01 0.56 0.52 0.64 0.07 0.04 | 2 60 32 84 7 7 |
| Burlington St E/ Birch Avenue | Signalized | A | EBTR WBL WBT | B C A | 18 29 0 | 0.26 0.35 0.35 | 45 32 - |
| Birch Avenue/ Brant St | Signalized | B | EBTR WBTL SBTLR | B B A | 14 15 8 | 0.07 0.09 0.13 | 9 13 22 |
| Wentworth St N/ Brant St | Unsignalized | - | WBLR SBTL NBTR | B A A | 11 0 0 | 0.05 0.00 0.00 | 1 0 0 |
| Brant St/ Hillyard St | Unsignalized | - | EBTLR WBTLR NBTLR SBTLR | A A A A | 1 3 9 9 | 0.00 0.01 0.02 0.02 | 0 0 1 1 |
| Munroe St/ Wentworth St N | Unsignalized | - | WBLR SBTL NBTR | B A A | 11 0 0 | 0.05 0.00 0.00 | 1 0 0 |
| Birch Ave & Site Access #1 | Unsignalized | - | EBL EBR SBTR | A A A | 0 0 0 | 0.00 0.00 0.00 | 0 0 0 |
| Brant Street & Site Access #2 | Unsignalized | - | NBL NBR EBTR WBTL | A A A A | 0 0 0 0 | 0.00 0.00 0.00 0.00 | 0 0 0 0 |
| Hillyard St & Site Access #3 | Unsignalized | - | WBLR SBTL NBTR | A A A | 0 0 0 | 0.00 0.00 0.00 | 0 0 0 |
| PM PEAK | | | | | | | |
| Wentworth St N/Burlington St E | Signalized | B | EBL EBTR WBL WBTR NBTLR SBTLR | B C B B C C | 19 28 16 13 21 21 | 0.05 0.66 0.45 0.42 0.07 0.06 | 5 74 23 49 7 9 |
| Burlington St E/ Birch Avenue | Signalized | A | EBTR WBL WBT | B C A | 14 31 0 | 0.40 0.38 0.30 | 64 32 - |
| Birch Avenue/ Brant St | Signalized | A | EBTR WBTL SBTLR | B B A | 12 13 6 | 0.05 0.13 0.14 | 7 17 13 |
| Wentworth St N/ Brant St | Unsignalized | - | WBLR SBTL NBTR | B A A | 11 1 0 | 0.05 0.01 0.00 | 1 0 0 |
| Brant St/ Hillyard St | Unsignalized | - | EBTLR WBTLR NBTLR SBTLR | A A A A | 0 1 9 9 | 0.00 0.00 0.02 0.03 | 0 0 0 1 |
| Munroe St/ Wentworth St N | Unsignalized | - | WBLR SBTL NBTR | B A A | 11 0 0 | 0.07 0.00 0.00 | 2 0 0 |
| Birch Ave & Site Access #1 | Unsignalized | - | EBL EBR SBTR | A A A | 0 0 0 | 0.00 0.00 0.00 | 0 0 0 |
| Brant Street & Site Access #2 | Unsignalized | - | NBL NBR EBTR WBTL | A A A A | 0 0 0 0 | 0.00 0.00 0.00 0.00 | 0 0 0 0 |
| Hillyard St & Site Access #3 | Unsignalized | - | WBLR SBTL NBTR | A A A | 0 0 0 | 0.00 0.00 0.00 | 0 0 0 |

Exhibit 5-6: 2022 Future Total Traffic Analysis Summary

| Intersection Name | Control Type | Int LOS | All Movements | | | | |
|--------------------------------|--------------|---------|---------------|-----|-----------|-----------|---------------------------------------|
| | | | Mvmt | LOS | Delay (s) | V/C Ratio | 95 th Percentile Queue (m) |
| AM PEAK | | | | | | | |
| Wentworth St N/Burlington St E | Signalized | B | EBL | C | 20 | 0.01 | 2 |
| | | | EBTR | C | 27 | 0.57 | 61 |
| | | | WBL | B | 16 | 0.52 | 32 |
| | | | WBTR | B | 16 | 0.64 | 84 |
| | | | NBTLR | C | 21 | 0.07 | 8 |
| Burlington St E/ Birch Avenue | Signalized | A | SBTLR | C | 21 | 0.04 | 7 |
| | | | EBTR | B | 18 | 0.26 | 45 |
| | | | WBL | C | 29 | 0.38 | 34 |
| Birch Avenue/ Brant St | Signalized | B | WBT | A | 0 | 0.35 | - |
| | | | EBTR | B | 14 | 0.07 | 9 |
| | | | WBTL | B | 15 | 0.10 | 13 |
| Wentworth St N/ Brant St | Unsignalized | - | SBTLR | A | 8 | 0.14 | 23 |
| | | | WBLR | B | 11 | 0.07 | 2 |
| | | | SBTL | A | 1 | 0.01 | 0 |
| Brant St/ Hillyard St | Unsignalized | - | NBTR | A | 0 | 0.00 | 0 |
| | | | EBTLR | A | 1 | 0.00 | 0 |
| | | | WBTLR | A | 6 | 0.04 | 1 |
| Munroe St/ Wentworth St N* | Unsignalized | - | NBTLR | A | 10 | 0.04 | 1 |
| | | | SBTLR | A | 10 | 0.02 | 1 |
| | | | WBLR | B | 11 | 0.06 | 2 |
| Birch Ave & Site Access #1 | Unsignalized | - | SBTL | A | 0 | 0.00 | 0 |
| | | | EBL | A | 0 | 0.00 | 0 |
| | | | EBR | B | 10 | 0.01 | 0 |
| Brant Street & Site Access #2 | Unsignalized | - | SBTR | A | 0 | 0.00 | 0 |
| | | | NBL | A | 0 | 0.00 | 0 |
| | | | NBR | A | 10 | 0.00 | 0 |
| Hillyard St & Site Access #3 | Unsignalized | - | EBTR | A | 0 | 0.00 | 0 |
| | | | WBTL | A | 0 | 0.00 | 0 |
| | | | WBLR | A | 9 | 0.02 | 0 |
| PM PEAK | | | | | | | |
| Wentworth St N/Burlington St E | Signalized | C | SBTL | A | 6 | 0.06 | 2 |
| | | | EBL | A | 0 | 0.00 | 0 |
| | | | EBTR | B | 19 | 0.05 | 5 |
| | | | EBTR | C | 28 | 0.67 | 74 |
| | | | WBL | B | 16 | 0.45 | 23 |
| Burlington St E/ Birch Avenue | Signalized | A | WBTR | B | 13 | 0.42 | 49 |
| | | | NBTLR | C | 21 | 0.08 | 8 |
| | | | SBTLR | C | 21 | 0.06 | 9 |
| Birch Avenue/ Brant St | Signalized | A | WBT | A | 0 | 0.30 | - |
| | | | EBTR | B | 12 | 0.07 | 8 |
| | | | WBTL | B | 13 | 0.13 | 17 |
| Wentworth St N/ Brant St | Unsignalized | - | SBTLR | A | 6 | 0.14 | 12 |
| | | | WBLR | B | 11 | 0.10 | 3 |
| | | | SBTL | A | 1 | 0.01 | 0 |
| Brant St/ Hillyard St | Unsignalized | - | NBTR | A | 0 | 0.00 | 0 |
| | | | EBTLR | A | 0 | 0.00 | 0 |
| | | | WBTLR | A | 4 | 0.02 | 1 |
| Munroe St/ Wentworth St N* | Unsignalized | - | NBTLR | A | 9 | 0.10 | 3 |
| | | | SBTLR | A | 9 | 0.03 | 1 |
| | | | WBLR | B | 12 | 0.13 | 3 |
| Birch Ave & Site Access #1 | Unsignalized | - | SBTL | A | 0 | 0.00 | 0 |
| | | | EBL | A | 0 | 0.00 | 0 |
| | | | EBR | B | 10 | 0.01 | 0 |
| Brant Street & Site Access #2 | Unsignalized | - | SBTR | A | 0 | 0.00 | 0 |
| | | | NBL | A | 0 | 0.00 | 0 |
| | | | NBR | A | 0 | 0.00 | 0 |
| Hillyard St & Site Access #3 | Unsignalized | - | EBTR | A | 0 | 0.00 | 0 |
| | | | WBTL | A | 0 | 0.00 | 0 |
| | | | WBLR | A | 9 | 0.13 | 4 |
| | Unsignalized | - | SBTL | A | 4 | 0.03 | 1 |
| | | | SBTL | A | 0 | 0.00 | 0 |
| | | | NBTR | A | 0 | 0.00 | 0 |

Exhibit 5-7: 2027 Future Background Traffic Analysis Summary

| Intersection Name | Control Type | Int LOS | All Movements | | | | |
|--------------------------------|--------------|---------|--|----------------------------|----------------------------------|--|---------------------------------------|
| | | | Mvmt | LOS | Delay (s) | V/C Ratio | 95 th Percentile Queue (m) |
| AM PEAK | | | | | | | |
| Wentworth St N/Burlington St E | Signalized | C | EBL EBTR WBL WBTR NBTLR SBTLR | C C B B C C | 20 28 18 18 21 21 | 0.01 0.62 0.60 0.70 0.08 0.04 | 2 67 35 98 8 7 |
| Burlington St E/ Birch Avenue | Signalized | A | EBTR WBL WBT | B C A | 19 29 1 | 0.28 0.39 0.39 | 51 35 - |
| Birch Avenue/ Brant St | Signalized | B | EBTR WBTL SBTLR | B B A | 14 15 8 | 0.07 0.11 0.15 | 10 15 24 |
| Wentworth St N/ Brant St | Unsignalized | - | WBLR SBTL NBTR | B A A | 11 0 0 | 0.06 0.00 0.00 | 2 0 0 |
| Brant St/ Hillyard St | Unsignalized | - | EBTLR WBTLR NBTLR SBTLR | A A A A | 1 3 9 9 | 0.00 0.01 0.03 0.02 | 0 0 1 1 |
| Munroe St/ Wentworth St N | Unsignalized | - | WBLR SBTL NBTR | B A A | 11 0 0 | 0.07 0.00 0.00 | 2 0 0 |
| Birch Ave & Site Access #1 | Unsignalized | - | EBL EBR SBTR | A A A | 0 0 0 | 0.00 0.00 0.00 | 0 0 0 |
| Brant Street & Site Access #2 | Unsignalized | - | NBL NBR EBTR WBTL | A A A A | 0 0 0 0 | 0.00 0.00 0.00 0.00 | 0 0 0 0 |
| Hillyard St & Site Access #3 | Unsignalized | - | WBLR SBTL NBTR | A A A | 0 0 0 | 0.00 0.00 0.00 | 0 0 0 |
| PM PEAK | | | | | | | |
| Wentworth St N/Burlington St E | Signalized | C | EBL EBTR WBL WBTR NBTLR SBTLR | B C B B C C | 20 30 18 13 21 22 | 0.06 0.73 0.53 0.46 0.08 0.07 | 5 84 26 55 8 10 |
| Burlington St E/ Birch Avenue | Signalized | A | EBTR WBL WBT | B C A | 15 31 0 | 0.45 0.41 0.34 | 72 35 - |
| Birch Avenue/ Brant St | Signalized | A | EBTR WBTL SBTLR | B B A | 12 13 7 | 0.05 0.15 0.15 | 7 18 13 |
| Wentworth St N/ Brant St | Unsignalized | - | WBLR SBTL NBTR | B A A | 11 1 0 | 0.06 0.01 0.00 | 2 0 0 |
| Brant St/ Hillyard St | Unsignalized | - | EBTLR WBTLR NBTLR SBTLR | A A A A | 0 1 9 9 | 0.00 0.00 0.02 0.03 | 0 0 1 1 |
| Munroe St/ Wentworth St N | Unsignalized | - | WBLR SBTL NBTR | B A A | 12 0 0 | 0.09 0.00 0.00 | 2 0 0 |
| Birch Ave & Site Access #1 | Unsignalized | - | EBL EBR SBTR | A A A | 0 0 0 | 0.00 0.00 0.00 | 0 0 0 |
| Brant Street & Site Access #2 | Unsignalized | - | NBL NBR EBTR WBTL | A A A A | 0 0 0 0 | 0.00 0.00 0.00 0.00 | 0 0 0 0 |
| Hillyard St & Site Access #3 | Unsignalized | - | WBLR SBTL NBTR | A A A | 0 0 0 | 0.00 0.00 0.00 | 0 0 0 |

Exhibit 5-8: 2027 Future Total Traffic Analysis Summary

| Intersection Name | Control Type | Int LOS | All Movements | | | | |
|--------------------------------|--------------|---------|--|----------------------------|----------------------------------|--|---------------------------------------|
| | | | Mvmt | LOS | Delay (s) | V/C Ratio | 95 th Percentile Queue (m) |
| AM PEAK | | | | | | | |
| Wentworth St N/Burlington St E | Signalized | C | EBL EBTR WBL WBTR NBTLR SBTLR | C C B B C C | 20 28 19 18 21 21 | 0.01 0.63 0.61 0.70 0.08 0.04 | 2 68 35 98 8 7 |
| Burlington St E/ Birch Avenue | Signalized | A | EBTR WBL WBT | B C A | 19 30 1 | 0.28 0.41 0.39 | 51 37 - |
| Birch Avenue/ Brant St | Signalized | B | EBTR WBTL SBTLR | B B A | 14 15 8 | 0.07 0.11 0.15 | 10 15 25 |
| Wentworth St N/ Brant St | Unsignalized | - | WBLR SBTL NBTR | B A A | 12 1 0 | 0.08 0.01 0.00 | 2 0 0 |
| Brant St/ Hillyard St | Unsignalized | - | EBTLR WBTLR NBTLR SBTLR | A A A A | 1 6 10 10 | 0.00 0.04 0.04 0.03 | 0 1 1 1 |
| Munroe St/ Wentworth St N* | Unsignalized | - | WBLR SBTL NBTR | B A A | 12 0 0 | 0.08 0.00 0.00 | 2 0 0 |
| Birch Ave & Site Access #1 | Unsignalized | - | EBL EBR SBTR | A B A | 0 10 0 | 0.00 0.01 0.00 | 0 0 0 |
| Brant Street & Site Access #2 | Unsignalized | - | NBL NBR EBTR WBTL | A A A A | 0 10 0 0 | 0.00 0.00 0.00 0.00 | 0 0 0 0 |
| Hillyard St & Site Access #3 | Unsignalized | - | WBLR SBTL NBTR | A A A | 9 6 0 | 0.02 0.06 0.00 | 0 2 0 |
| PM PEAK | | | | | | | |
| Wentworth St N/Burlington St E | Signalized | C | EBL EBTR WBL WBTR NBTLR SBTLR | B C B B C C | 20 30 18 13 21 22 | 0.06 0.73 0.53 0.46 0.08 0.07 | 5 84 26 55 8 10 |
| Burlington St E/ Birch Avenue | Signalized | B | EBTR WBL WBT | B C A | 15 31 0 | 0.45 0.43 0.34 | 72 36 - |
| Birch Avenue/ Brant St | Signalized | A | EBTR WBTL SBTLR | B B A | 12 13 6 | 0.07 0.15 0.16 | 9 18 13 |
| Wentworth St N/ Brant St | Unsignalized | - | WBLR SBTL NBTR | B A A | 12 1 0 | 0.11 0.01 0.00 | 3 0 0 |
| Brant St/ Hillyard St | Unsignalized | - | EBTLR WBTLR NBTLR SBTLR | A A A A | 0 4 10 9 | 0.00 0.02 0.11 0.03 | 0 1 3 1 |
| Munroe St/ Wentworth St N* | Unsignalized | - | WBLR SBTL NBTR | B A A | 13 0 0 | 0.13 0.00 0.00 | 4 0 0 |
| Birch Ave & Site Access #1 | Unsignalized | - | EBL EBR SBTR | A B A | 0 10 0 | 0.00 0.01 0.00 | 0 0 0 |
| Brant Street & Site Access #2 | Unsignalized | - | NBL NBR EBTR WBTL | A A A A | 0 0 0 0 | 0.00 0.00 0.00 0.00 | 0 0 0 0 |
| Hillyard St & Site Access #3 | Unsignalized | - | WBLR SBTL NBTR | A A A | 9 4 0 | 0.13 0.03 0.00 | 4 1 0 |

6 Improvement Measures

6.1 Hillyard Street and Brant Street Intersection

In consultation with the City, staff requested analysis of converting the existing intersection from two-way stop control (TWSC) to all-way stop control (AWSC). The AWSC warrant was performed using OTM Book 5 (Regulatory Signs) guidelines. The results is summarized in Exhibit 6-1.

Exhibit 6-1: All-way Stop Control Warrant for Hillyard Street & Brant Street Intersection

| Justification | Description | Minimum Requirement | Compliance | | Justification Met? |
|---------------------------|--|---------------------|------------|-----|--------------------|
| | | | Section | | |
| | | | Numerical | % | |
| 1. Minimum Volume Warrant | A. Vehicle volume on all approaches, for the highest hour recorded | 350 | 172 | 49% | NO |
| | B. Vehicle volume split | 65 / 35 | 53 / 47 | Met | YES |

Review of traffic volumes shows that AWSC is appropriate at this location as the intersection has moderate and relatively balanced volume levels (53 / 47 split) for each approach. Although the volume criteria is not met, from a traffic safety perspective, AWSC can reduce right-angle and turning collisions. It provides a more orderly movement at an intersection, reducing through and turning speeds on Brant Street, and minimizing safety concerns of any sight distance restrictions that may be present. At the intersection and on the east quadrants, as illustrated in Exhibit 6-2, there currently exists a safety hazard where sightlines are obstructed from trees/utility poles (left image) and the meat plant / warehouse (right image). This concern can be addressed with the proposed improvement.

Exhibit 6-2: Sightline Obstruction at Hillyard Street and Brant Street Intersection



The AWSC improvement measure was compared using 2027 future total volumes, with results presented in Exhibit 6-3.

Exhibit 6-3: Analysis of Improvements to Hillyard Street and Brant Street Intersection

| Control Type | AM PEAK | | | | | PM PEAK | | | | |
|--|------------|--------------------|-----|-------|-----------|------------|--------------------|-----|-------|-----------|
| | Avg. Delay | Critical Movements | | | | Avg. Delay | Critical Movements | | | |
| | | Mvmnt | LOS | Delay | 95% Queue | | Mvmnt | LOS | Delay | 95% Queue |
| Two-Way Stop Control (TWSC) – Existing | 5 | EBTLR | A | 1 | 0 | 6 | EBTLR | A | 0 | 0 |
| | | WBTLR | A | 6 | 1 | | WBTLR | A | 4 | 1 |
| | | NBTLR | A | 10 | 1 | | NBTLR | A | 10 | 3 |
| | | SBTLR | A | 10 | 1 | | SBTLR | A | 9 | 1 |
| All-way Stop Control (AWSC) | 8 | EBTLR | A | 7 | - | 8 | EBTLR | A | 7 | - |
| | | WBTLR | A | 8 | - | | WBTLR | A | 8 | - |
| | | NBTLR | A | 7 | - | | NBTLR | A | 8 | - |
| | | SBTLR | A | 7 | - | | SBTLR | A | 7 | - |

Traffic operations for both alternatives are similar, with average intersection delays higher by 2-3 seconds for the AWSC. With added signage and pavement markings, the Hillyard Street (minor) approaches will decrease in delay, however at the expense of the Brant Street (major) approaches which was originally operating as free-flow. Overall, both alternatives are expected to operate well with average intersection delays of 8 seconds or lower.

Given that traffic operations for the two options is comparable and with safety / sight line improvements, converting TWSC to AWSC is recommended for this location.

6.2 Traffic Calming / Management

With the introduction of MSF, some site generated traffic are expected to travel through residential streets (i.e. Brant Street, Niagara Street, and Munroe Street). These volumes are expected to be low, however most of these trips (discussed in Section 4) are made during off peak periods (4:30-6:30 AM, 1-3:30 PM) which may cause noise concerns to local residents.

In this situation, traffic calming / management measures may be appropriate to safely reduce vehicular speeds and attempt to reroute traffic. It is recommended that HSR develop an appropriate access route plan for employees entering and exiting the site. HSR staff should be instructed to travel via Birch Avenue in order to reduce impacts to surrounding neighbourhoods. This is align with the City's Traffic Calming / Management Policy (updated 2013) that considers management plans to be preferable than street-by-street traffic calming measures which may inadvertently shift problems to adjacent roadways.

If cut-through traffic or speeding are a concern following the construction of MSF, implementation of speed humps may be appropriate on Niagara Street and Munroe Street. This is subject to further review to confirm the observed need (e.g. spot speed study) and with consultation with Ward 3 Councillor.

6.3 Left Turn Assessment

To assess potential left-turn lane requirements associated with the proposed development, the MTO Design Supplement for the Transportation Association of Canada (TAC) Geometric Design Guide for Canadian Roads (June 2017) was reviewed. For the primary bus access off of Birch Avenue, the MTO supplement's Exhibit EA-7-1 (left turn storage at a two lane undivided highway, unsignalized) was used.

Given that traffic forecasts for the two-way conversion of Birch Avenue have not yet been established, a preliminary assessment was conducted using the following assumptions:

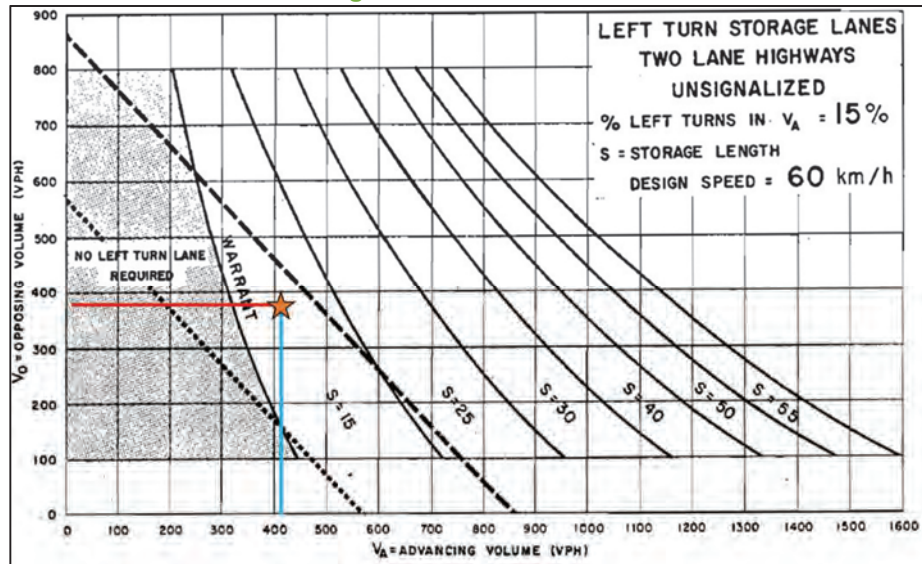
- Two lane undivided roadway (one-northbound, one southbound);
- 60 km/h design speed (50 km/h posted speed limit);
- Advancing and opposing volume of 375 vph. Opposing volume is represented by the full southbound approach volume for 2027 future total condition. Due to lack of northbound volumes, the advancing volume was assumed to be that of the opposing volume;
- Left turning traffic volume – 42 vph. This value is the amount of bus fleet vehicles generated during the PM peak (12 vph) multiplied by a 3.5 factor. This factor is to represent impact from an equivalent passenger vehicle (3.5 passenger car equivalence for buses).

The preliminary assessment shows that northbound left-turn storage lanes is warranted at the Birch Avenue entrance. The MTO Design supplement suggests a minimum storage of 15

metres, as illustrated in Exhibit 6-4. To accommodate for vehicle type, it is recommended that the length be extended to 20 metres to safely store at least one (18 metre) articulated bus.

It is recommended that future analysis and review be conducted once traffic forecasts are confirmed under the Birch Avenue EA.

Exhibit 6-4: Left Turn Storage Lane Assessment: Birch Avenue Access



7 Sight Distance

Following TAC Geometric Design Guide for Canadian Roads, departure sight triangles for each accesses were reviewed. These sight triangles are defined as areas free of obstruction at each quadrant of each intersection approach controlled by either a stop or yield sign. It allows drivers of the vehicles on the major road to see any vehicles stopped on the minor road approach and to be prepared to slow or stop, if needed.

At both the Birch Avenue and Brant Street accesses, the adjacent roadways are relatively straight and flat (< 3% grade). No sightline deficiencies exists at either intersections. Drivers on both approaches will have appropriate sightlines, minimum stopping sight distance, to respond to critical intersection decision points at the unsignalized accesses. The site plans show 5-metre sight triangles in keeping with site plan guidelines.

At the Hillyard Street access, due to the curvature of the alignment, existing vegetation and the utility pole to the south, sightlines are currently obstructed for traffic entering/exiting the parkade. This concern is mitigated by the existing all-way stop control at Munroe Street and Hillyard Street intersection (20-25 metres south of the entrance) that will provide appropriate visibility for incoming traffic. To/from the north leg, the site plans also show a 5-metre sight triangle.

Overall, it is concluded that all three accesses would either have sufficient sightline or have appropriate traffic control to function safely.

8 Parking Demand & Supply

To aid in the design of the multi-storey parking garage, a separate memorandum was developed and titled “Parking Demand & Supply Review” (attached as Appendix G). The memorandum includes a parking survey, staffing comparison, and adjustment factors to determine minimum number of parking spaces required for the MSF. This is aligned with Hamilton’s TDM for Development guideline to avoid an oversupply of parking and to encourage employees and visitors to choose alternative modes of transportation. It recommends that the new MSF to have a minimum parking provision of 402 parking spaces, 10 of which are barrier-free. These values were taken into consideration in determining the final parking design.

9 Parkade Access Analysis

The multi-storey parking structure is required to have a secure perimeter with card reading technology at the entry point combined with an overhead door. The site layout proposes a pedestal card reader and video intercom at the vehicle entrance along with roll up doors. With this control type, it is important to ensure that entry driveway controls do not cause vehicles to back onto public streets where they can potentially disrupt local traffic and causing safety hazards.

There are a number of factors that have been considered and are as follows:

- One access provided for a parking structure of approximately 400-420 parking spaces;
- From background review, this control type typically has a service rate of 250-300 vph. Typical planning practices assumes a vehicle generation rate of 0.5 vehicles per parking space, resulting in approximately 200-210 vph (near threshold);
- Expected site generated traffic of 84 vph and 45 vph (discussed in Section 4.1.1) entering the parking structure during the AM and PM peak periods respectively. It is noted that most of entering traffic falls outside of background peak periods (operators required off peak);
- Transit operators and fleet maintenance staff have shift changes which will contribute to sharper peak flows (peak hour factor); and
- Future expansion and storage of 100 additional buses (with associated transit operators).

With the above considerations, two-lane entry and one-lane exit is recommended for the site access on Hillyard Street.

Under this configuration, the access is anticipated to operate well at LOS A for all movements. During the busier AM peak for entering traffic, the shared southbound through/left and northbound through/right lanes has delays of 6-9 seconds with 95th percentile queues of 2 metres. This is within an acceptable range for the parking facility and will minimally affect local traffic. It is also recommended that the card reader gate be stationed far enough into the site (10 m) to store at least two cars if the site plan allows for it.

10 Active Transportation

The proposed site layout will provide good pedestrian/cycling facilities and is summarized below.

Short-term bike storage will be provided at a rate of 2.5% of all peak visitors, and long term bike storage will be provided at a rate of 5% of all occupants. The designated area for bike storage is on the first level of the parking garage – P1 and is roughly 200 metres from the planned bike network. Showers will also be provided onsite. The present Hamilton bicycle network does not connect to the subject lands (closest being on Victoria Avenue North), however, the City of Hamilton Transportation Master Plan (updated 2018) shows planned bike routes running along Burlington Street and Birch Avenue, which will significantly improve connectivity to the larger bike network. Bike lanes on Birch Avenue will require further review under the EA. It is also noted that City staff are currently pursuing a multi-use path through the hydro corridor in place of bike lanes on Birch Avenue. The existing and planned bike routes are shown in Exhibit 10-1 below.

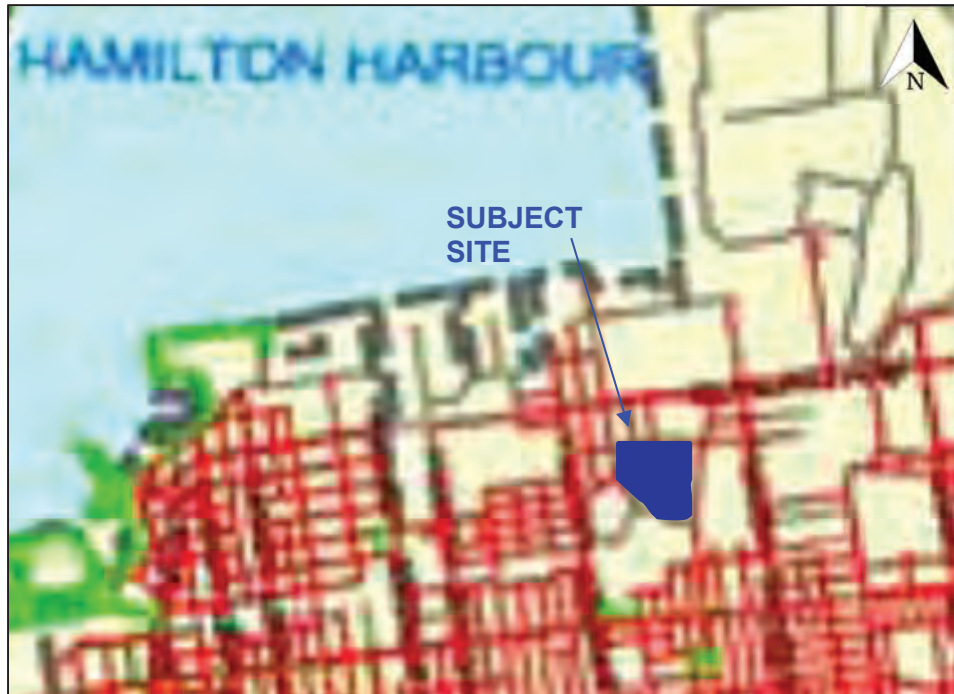
Currently, there is a well-established sidewalk network surrounding the site and it is easily accessible by pedestrians. The City of Hamilton’s Pedestrian Mobility Plan (refer to Exhibit 10-2) identifies the area surrounding MSF as “Industrial” and require sidewalks of minimum 1.8 m clear zone width. Hillyard Street, Brant Street and Birch Avenue currently have pedestrian sidewalks on either side of the road with illumination. All other smaller adjacent roadways have sidewalks which prioritizes pedestrian movements to adjacent neighbourhoods (west and northwest of site). Latest site plans also show sidewalks and curbs on the south side of Brant Street, north of the property limits. If the multi-use path in the hydro corridor is constructed, a sidewalk path will be included for connection.

These projects are currently underway and should provide safe, connected, and protected cycling and pedestrian network surrounding the subject lands.

Exhibit 10-1: Existing and Planned Bicycle Facilities



Exhibit 10-2: Existing Pedestrian Network (shown in red)



Source: City of Hamilton Pedestrian Mobility Plan (higher resolution image not available online)

11 Summary and Conclusions

The proposed 430,000 ft² Maintenance Storage Facility is a key component of the City's planned transit system expansion. The facility will house a total of approximately 200 regular and articulated buses with a planned future expansion that will store an additional 100 buses. The study provides a transportation impact assessment of the facility including traffic and bus-related operations up to the 2027 horizon.

For the future road network, Birch Avenue has been identified for conversion from one-way to two-way. The City has agreed that Birch Avenue intersections will not be analyzed in detail given limited information.

Vehicular access to the site will be provided by three locations; Birch Avenue which is the primary bus access, Brant street which is the secondary bus access, and Hillyard Street which serve passenger vehicles only. In addition, a multi-level parking structure is planned on the northwest corner of the study area.

The conclusions of the study is summarized below.

- Overall, under 2019 existing conditions, the study area intersections all operate well with sufficient reserve capacity during both peak periods. There are no intersections or movements that are considered to be operating at critical levels.
- Under both 2022 and 2027 future background conditions, traffic operations are maintained with sufficient capacity for all movements and intersections. Each study intersections is expected to operate at levels-of-service C or better.

- The subject site is expected to generate a total of 94 and 134 two-way trips for passenger vehicles in the AM and PM peak hour respectively. Separately, for bus fleet vehicles, a total of 9 and 14 vehicle trips are expected to be generated during the two peak periods. Majority of site generated traffic are generated outside of background peak periods and will minimally impact the road network.
- Under both the 2022 and 2027 future total conditions, traffic operations are expected to be similar to future background conditions. There are no intersections or movements in the study area that are anticipated to operate at critical levels. The site accesses are expected to operate well, either operating at levels-of-service B or better.
- During development of improvement measures, converting Hillyard Street & Brant Street intersection to all-way stop control (AWSC) is recommended. Although OTM warrant is not met, traffic operations for AWSC is comparable to the existing two-way stop control. This improvement will also provide for a more orderly movement at the intersection, reduce through and turning speeds on Brant Street, and improve sightline concerns at the east-leg.
- Traffic calming was considered for the study area, in addition to the above bullet, it is recommended that HSR develop an appropriate access route plan for staff travel to minimize impacts to surrounding neighbourhoods. HSR should instruct employees to access the facility via Birch Avenue. If cut-through or speeding are a concern following the construction of the development, speed humps may be appropriate on Niagara Street and Munroe Street. This is subject to further review / study (e.g. consultation with Ward 3 Councillor) at that time.
- A preliminary left turn assessment was conducted for the Birch Avenue access. A single northbound left-turn lane is recommended with a minimum storage length of 20 metres. Once traffic forecast are developed under the Birch Avenue EA, further analysis is recommended to confirm findings.
- Following TAC geometric guidelines, all three accesses have sufficient sightline or have appropriate traffic control to function safely.
- Parking Demand & Supply review was completed in a separate study (attached). A minimum parking provision of 402 parking spaces with 10 being barrier-free was recommended. This is aligned with Hamilton's TDM for Development guideline to avoid an oversupply of parking and to encourage employees and visitors to choose alternative modes of transportation.
- The parkade access analysis shows that two-lane entry and one-lane exit is the recommended configuration for the Hillyard Street access. If site plans allow, it is also recommended that the card reader gate be stationed 10 metres into the site. This is to ensure that vehicles do not back onto public streets causing blockages to local traffic.
- The site will have good connection to the active transportation network. It would be accessible via existing sidewalks on Hillyard Street, Brant Street and Birch Avenue and the planned bike routes along Burlington Street and Birch Avenue. Latest site plans show sidewalks and curbs on the south side of Brant Street, north of the property limits. City staff are currently pursuing a multi-use path through the hydro corridor in place of bike lanes on Birch Avenue. Short term bike storage at a rate of 2.5% of peak visitors, long term bike storage at a 5% rate of all occupants is recommended and is located at the first level of the parking garage (P1). Showers will also be provided.

APPENDIX A

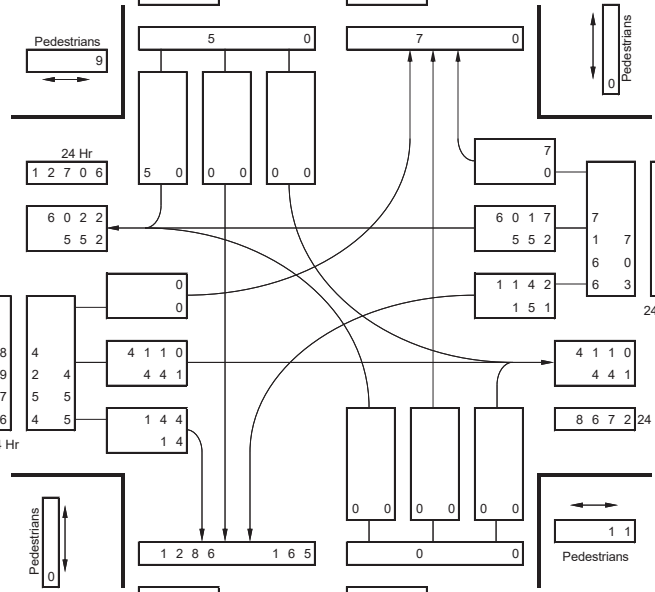
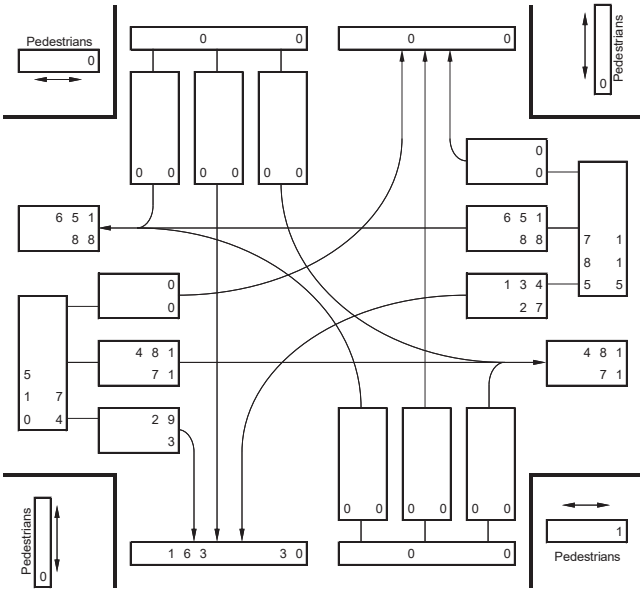
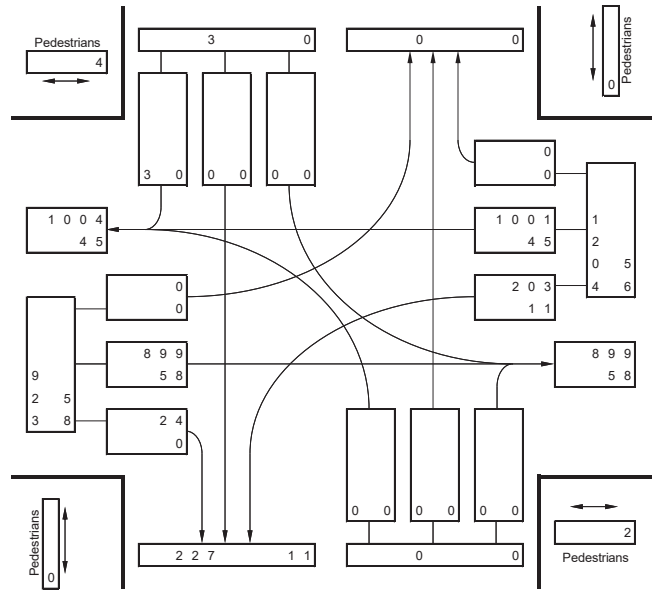
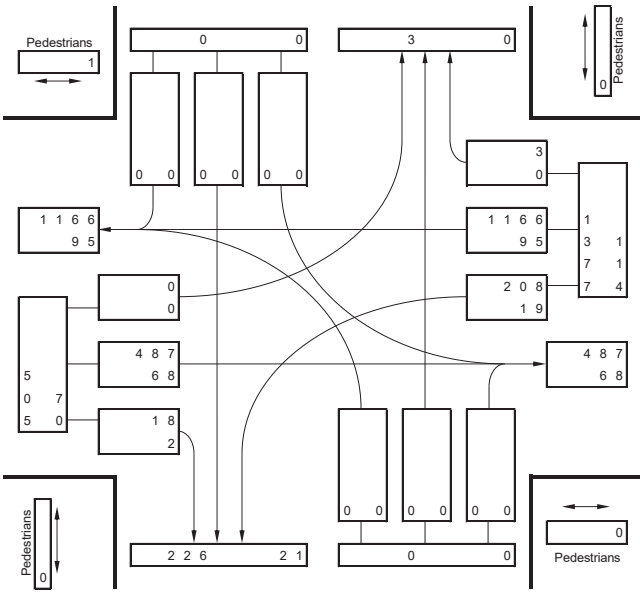
Turning Movement Counts (TMCs)

Intersection: **Burlington St**
 Direction: (East/West)
 Road Condition: Dry
 Comments:

at **Birch Ave**
 (North/South)
 Weather: Cloudy

Total Vehicles: 11,425
 M.V.E./Year: 8,196
 AWDT Factor: 2.11

Date: Monday
 Dec 8, 2014
 Period: 7 hours



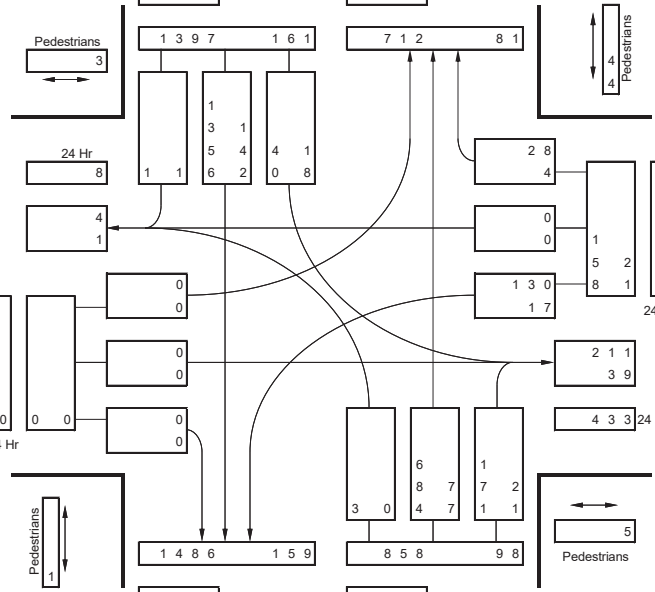
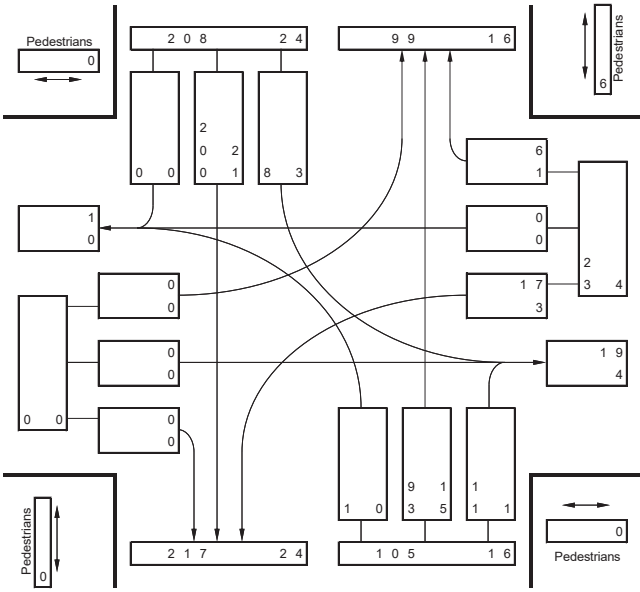
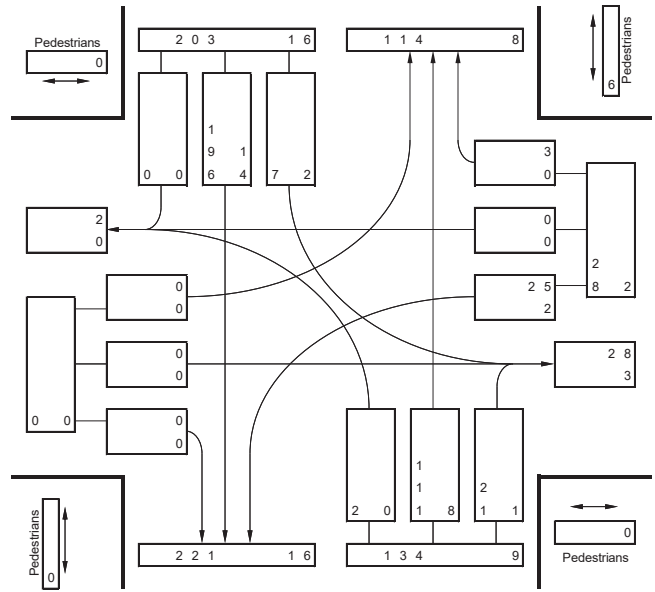
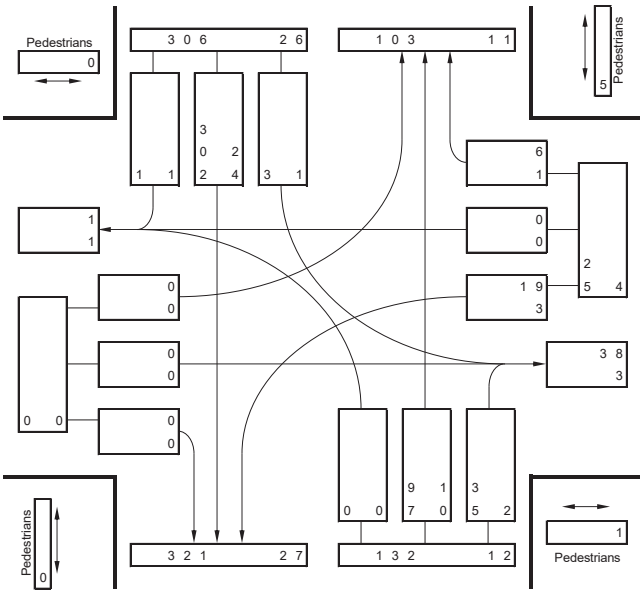
7 Hr & 24 Hr TOTAL VOLUMES

Intersection: Brant St at Wentworth St
Direction: (East/West)
Road Condition: Dry
Comments:

Weather: Clear

Total Vehicles: 2,413
M.V.E./Year: 1.682
AWDT Factor: 2.05

Date: Tuesday
Sep 20, 2016
Period: 7 hours

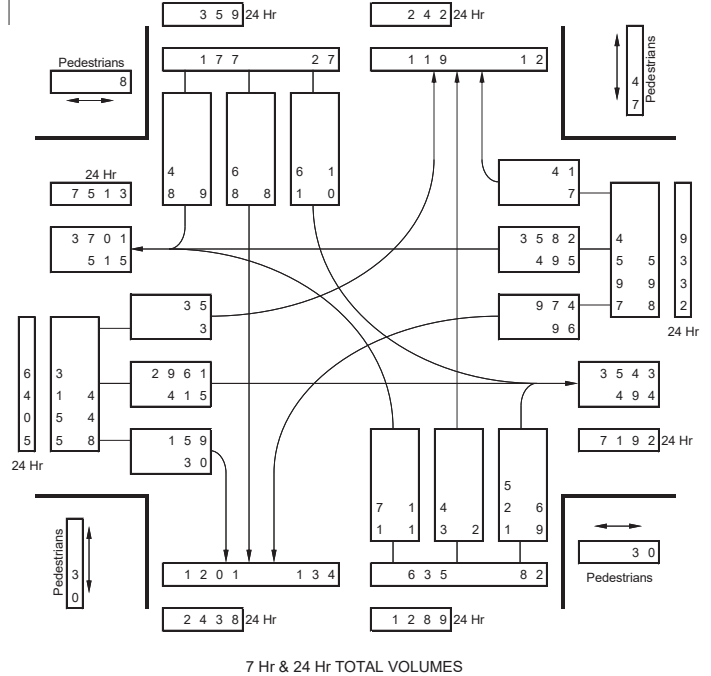
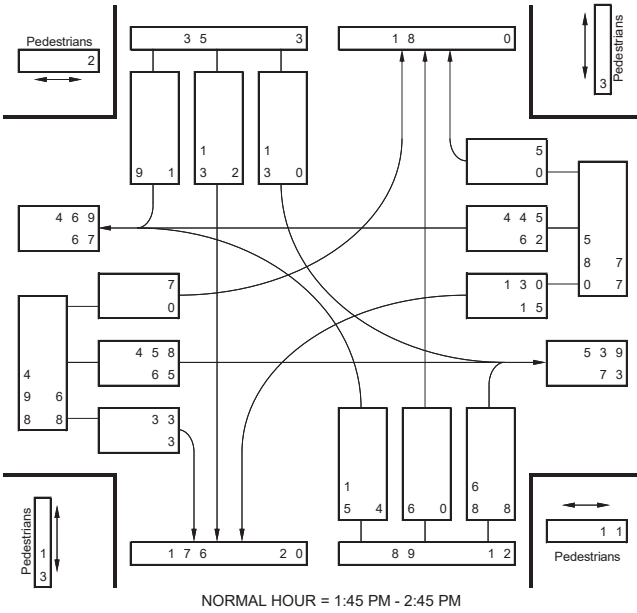
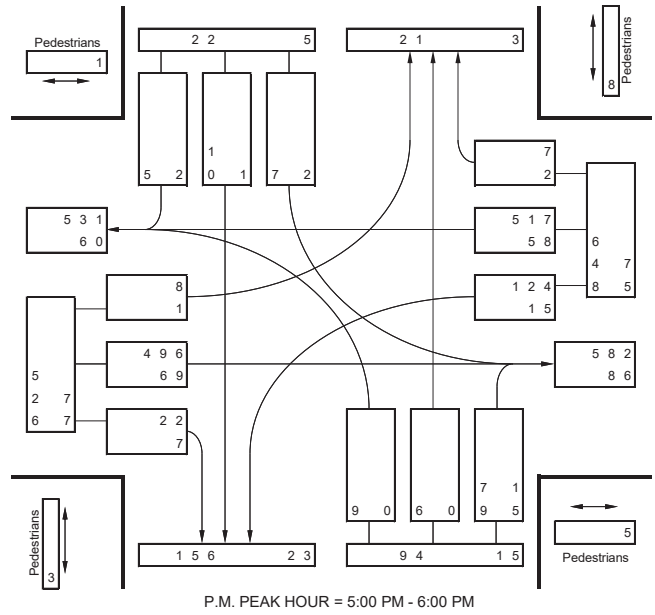
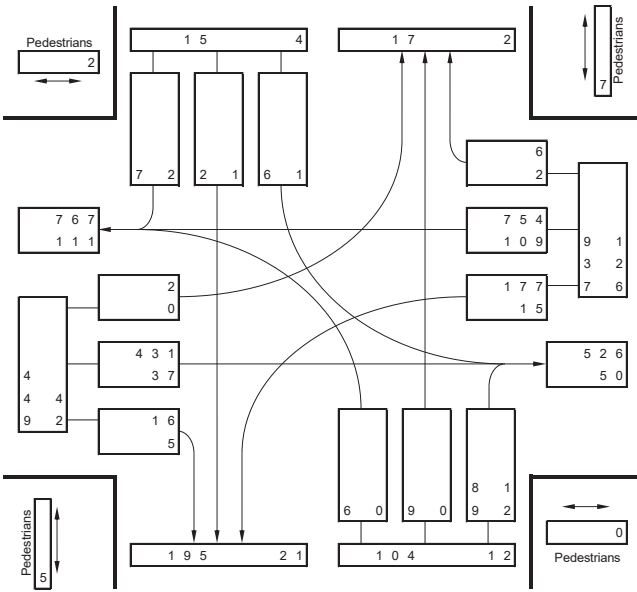


Intersection: Burlington St at Wentworth St
Direction: (East/West)
Road Condition: Dry
Comments:

Weather: Clear

Total Vehicles: 8,564
M.V.E./Year: 5,911
AWDT Factor: 2.03

Date: Wednesday
Sep 21, 2016
Period: 7 hours

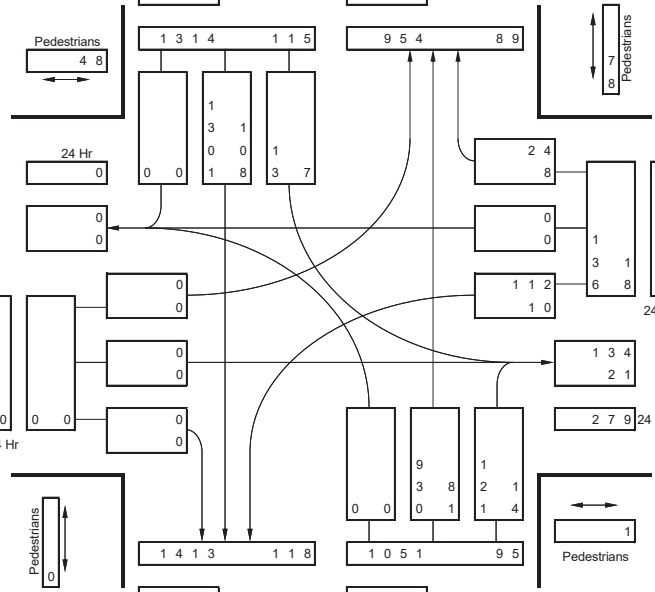
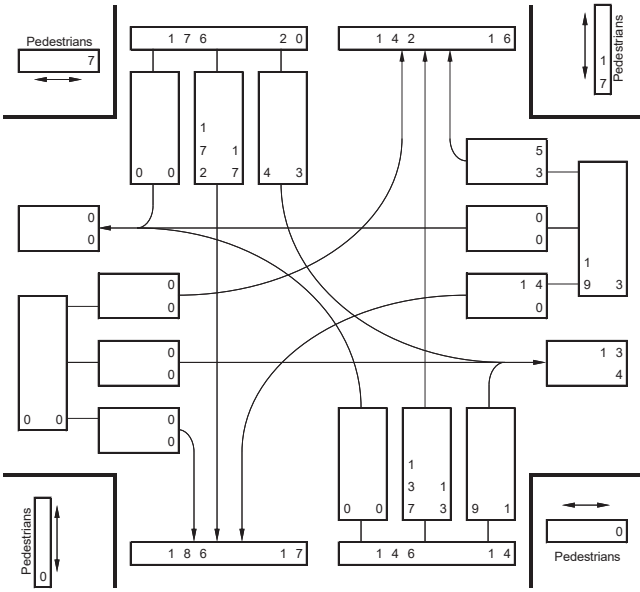
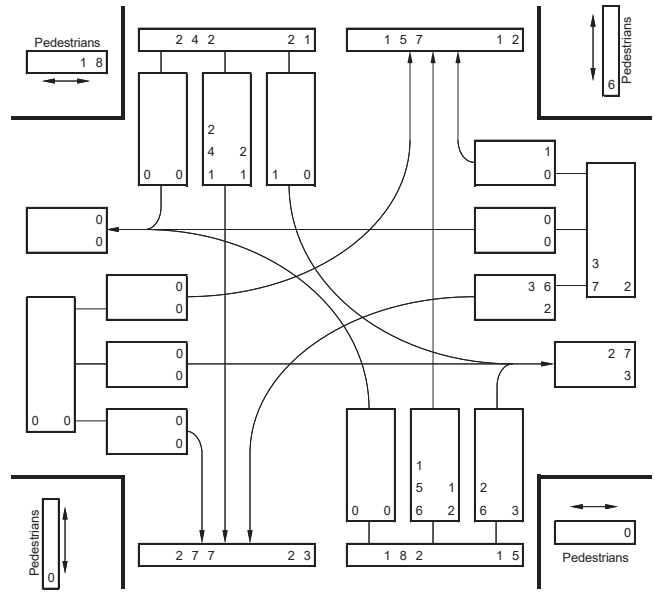
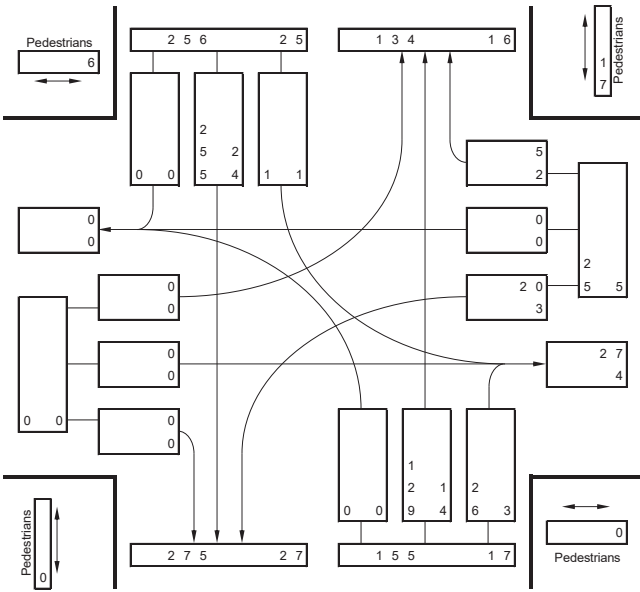


Intersection: **Monroe St** at **Wentworth St N**
 Direction: (East/West)
 Road Condition: Dry
 Comments:

Weather: Cloudy

Total Vehicles: 2,501
 M.V.E./Year: 1.769
 AWDT Factor: 2.08

Date: Monday
 Sep 18, 2017
 Period: 7 hours

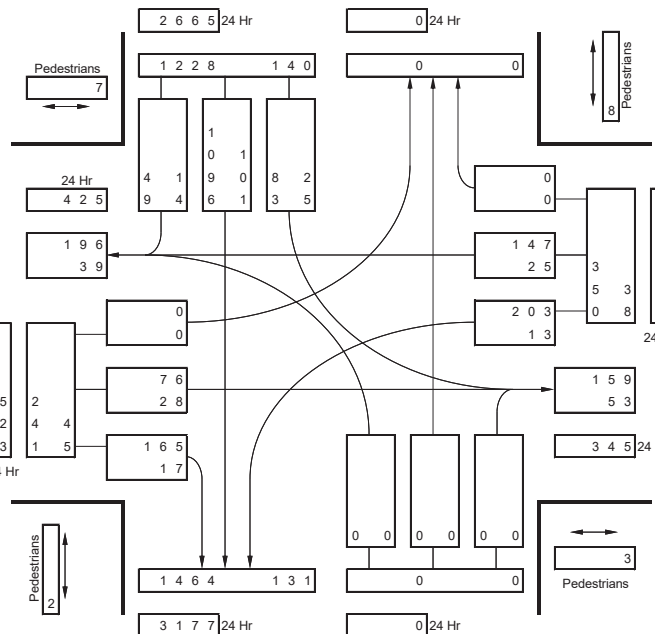
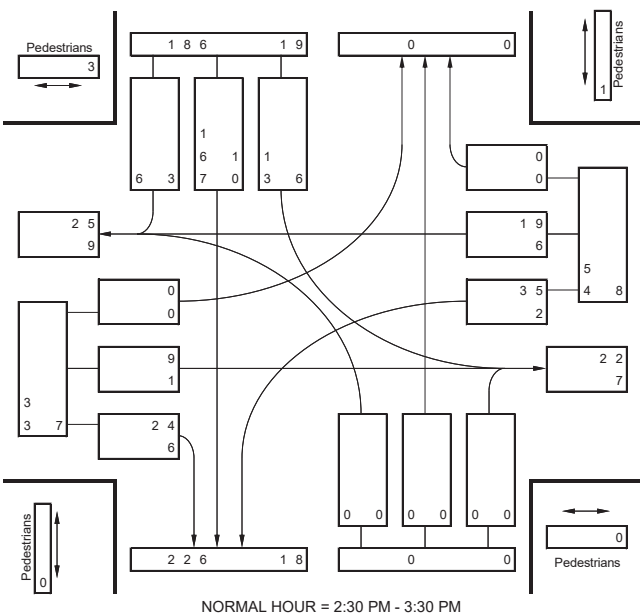
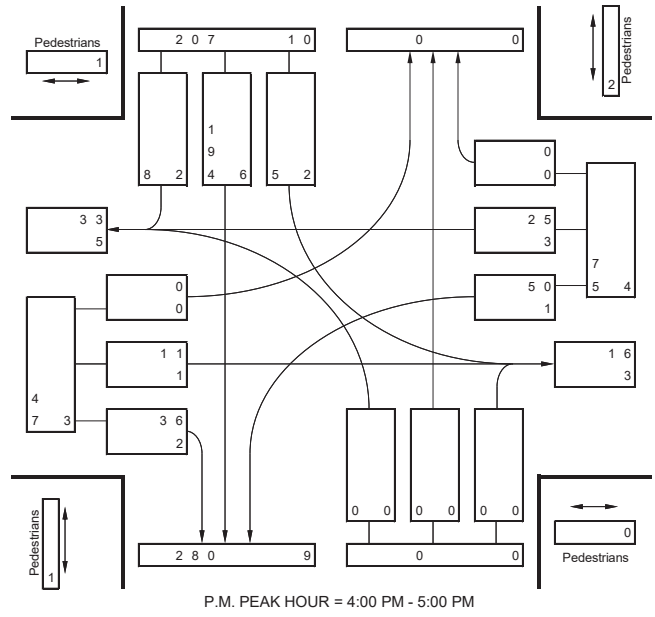
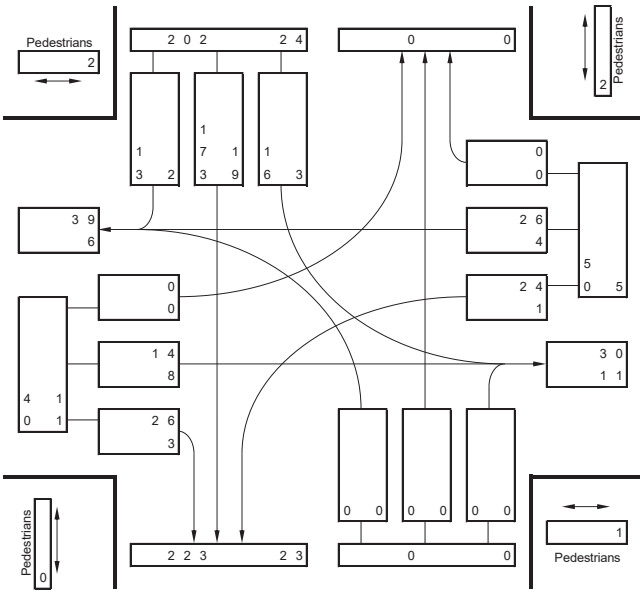


Intersection: Birch Ave at Brant St
Direction: (North/South)
Road Condition: Wet
Comments:

Weather: Rain

Total Vehicles: 1,819
M.V.E./Year: 1.342
AWDT Factor: 2.17

Date: Wednesday
Apr 25, 2018
Period: 7 hours

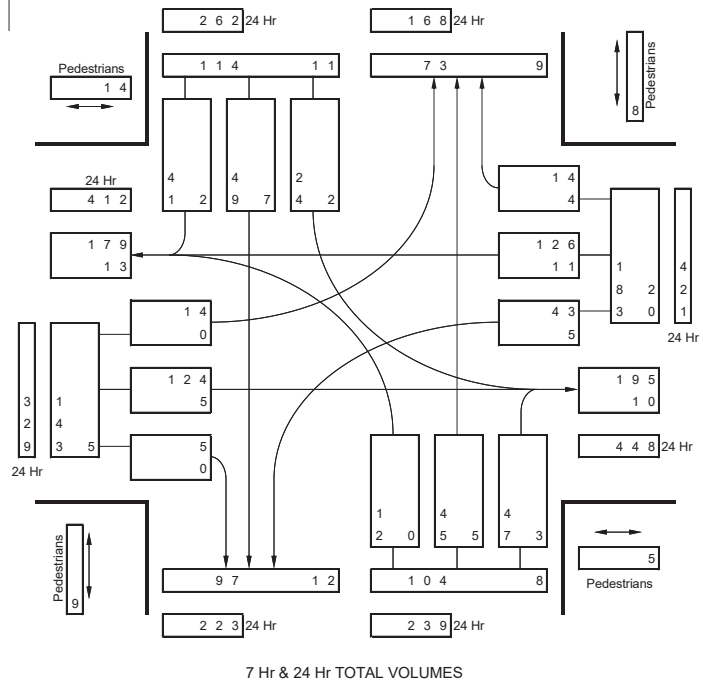
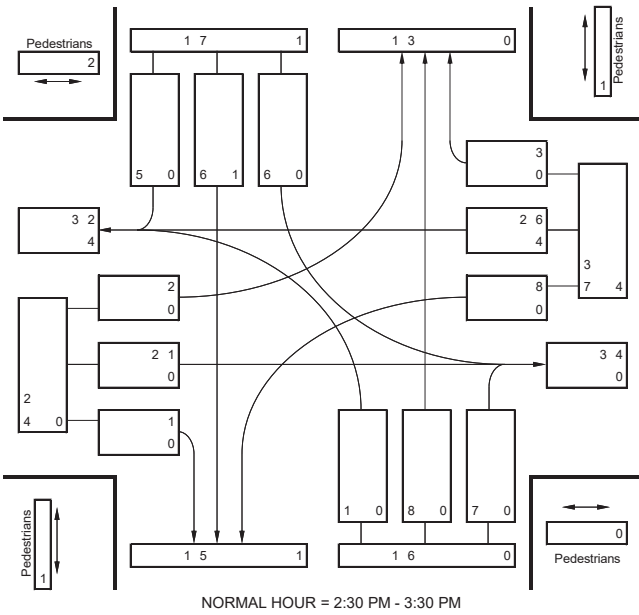
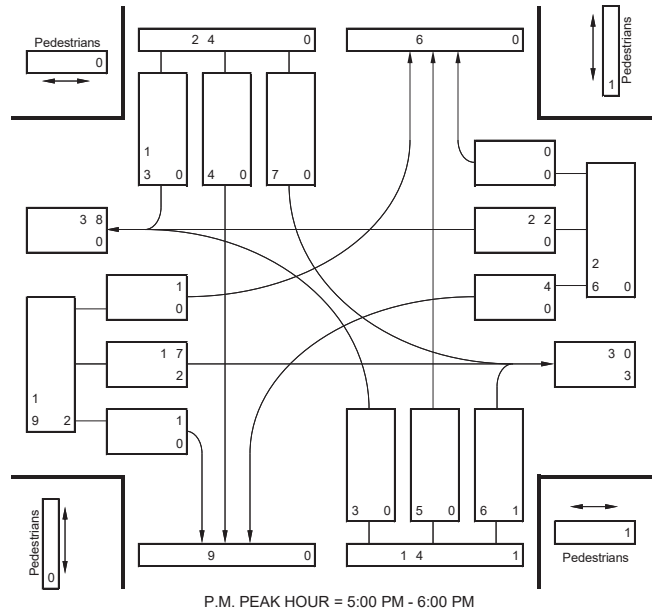
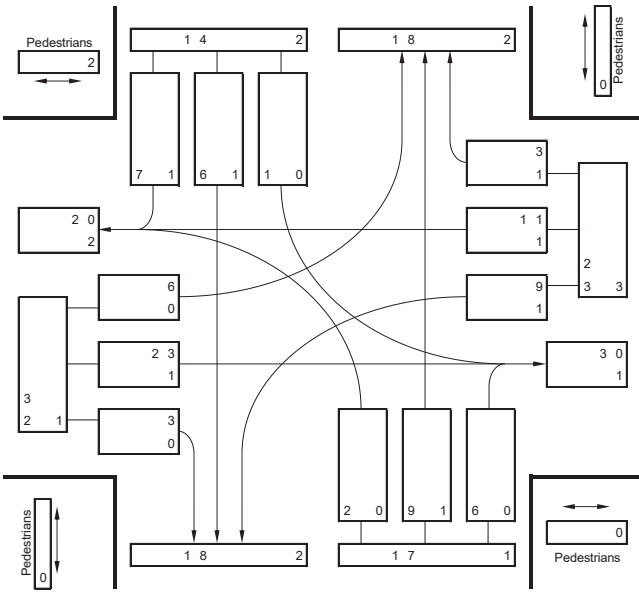


Intersection: **Brant St**
 Direction: (East/West)
 Road Condition: Dry
 Comments:

at **Hillyard St**
 (North/South)
 Weather: Cloudy

Total Vehicles: 544
 M.V.E./Year: .425
 AWDT Factor: 2.3

Date: Wednesday
 Mar 6, 2019
 Period: 7 hours



APPENDIX B

Synchro Outputs – Existing Conditions

Lanes, Volumes, Timings

AM Peak Period

3: Wentworth Street N & Burlington St E/Burlington Street E

05-16-2019



| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|----------------------------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|
| Lane Configurations | | | | | | | | | | | | |
| Traffic Volume (vph) | 2 | 497 | 22 | 204 | 916 | 8 | 6 | 10 | 107 | 7 | 3 | 10 |
| Future Volume (vph) | 2 | 497 | 22 | 204 | 916 | 8 | 6 | 10 | 107 | 7 | 3 | 10 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (m) | 60.0 | | 0.0 | 50.0 | | 0.0 | 0.0 | | 0.0 | 0.0 | | 0.0 |
| Storage Lanes | 1 | | 0 | 1 | | 0 | 0 | | 0 | 0 | | 0 |
| Taper Length (m) | 15.0 | | | 15.0 | | | 15.0 | | | 15.0 | | |
| Lane Util. Factor | 1.00 | 0.95 | 0.95 | 1.00 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 1.00 | 1.00 | 1.00 |
| Frt | | 0.994 | | | 0.999 | | | 0.870 | | | 0.932 | |
| Flt Protected | 0.950 | | | 0.950 | | | | 0.997 | | | 0.982 | |
| Satd. Flow (prot) | 1745 | 3192 | 0 | 1616 | 3082 | 0 | 0 | 2742 | 0 | 0 | 1394 | 0 |
| Flt Permitted | 0.285 | | | 0.312 | | | | 0.946 | | | 0.915 | |
| Satd. Flow (perm) | 523 | 3192 | 0 | 531 | 3082 | 0 | 0 | 2602 | 0 | 0 | 1299 | 0 |
| Right Turn on Red | | | Yes | | | Yes | | | Yes | | | Yes |
| Satd. Flow (RTOR) | | 5 | | | 2 | | | 116 | | | 11 | |
| Link Speed (k/h) | | 50 | | | 50 | | | 50 | | | 50 | |
| Link Distance (m) | | 125.0 | | | 716.7 | | | 215.5 | | | 91.4 | |
| Travel Time (s) | | 9.0 | | | 51.6 | | | 15.5 | | | 6.6 | |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles (%) | 0% | 8% | 24% | 8% | 13% | 25% | 0% | 0% | 12% | 14% | 33% | 22% |
| Adj. Flow (vph) | 2 | 540 | 24 | 222 | 996 | 9 | 7 | 11 | 116 | 8 | 3 | 11 |
| Shared Lane Traffic (%) | | | | | | | | | | | | |
| Lane Group Flow (vph) | 2 | 564 | 0 | 222 | 1005 | 0 | 0 | 134 | 0 | 0 | 22 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(m) | | 3.3 | | | 3.3 | | | 0.0 | | | 0.0 | |
| Link Offset(m) | | 0.0 | | | 0.0 | | | 0.0 | | | 0.0 | |
| Crosswalk Width(m) | | 4.9 | | | 4.9 | | | 4.9 | | | 4.9 | |
| Two way Left Turn Lane | | | | | Yes | | | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | 24 | | 14 | 24 | | 14 | 24 | | 14 | 24 | | 14 |
| Turn Type | Perm | NA | | pm+pt | NA | | Perm | NA | | Perm | NA | |
| Protected Phases | | 2 | | 1 | 6 | | | 4 | | | 8 | |
| Permitted Phases | 2 | | | 6 | | | 4 | | | 8 | | |
| Minimum Split (s) | 29.0 | 29.0 | | 9.5 | 37.2 | | 35.0 | 35.0 | | 35.0 | 35.0 | |
| Total Split (s) | 36.0 | 36.0 | | 19.0 | 55.0 | | 35.0 | 35.0 | | 35.0 | 35.0 | |
| Total Split (%) | 40.0% | 40.0% | | 21.1% | 61.1% | | 38.9% | 38.9% | | 38.9% | 38.9% | |
| Maximum Green (s) | 30.0 | 30.0 | | 15.0 | 48.8 | | 29.0 | 29.0 | | 29.0 | 29.0 | |
| Yellow Time (s) | 3.3 | 3.3 | | 3.0 | 3.3 | | 3.3 | 3.3 | | 3.3 | 3.3 | |
| All-Red Time (s) | 2.7 | 2.7 | | 1.0 | 2.9 | | 2.7 | 2.7 | | 2.7 | 2.7 | |
| Lost Time Adjust (s) | 0.0 | 0.0 | | 0.0 | 0.0 | | 0.0 | 0.0 | | 0.0 | 0.0 | |
| Total Lost Time (s) | 6.0 | 6.0 | | 4.0 | 6.2 | | 6.0 | 6.0 | | 6.0 | 6.0 | |
| Lead/Lag | Lag | Lag | | Lead | | | | | | | | |
| Lead-Lag Optimize? | Yes | Yes | | Yes | | | | | | | | |
| Walk Time (s) | 7.0 | 7.0 | | | 15.0 | | 11.0 | 11.0 | | 11.0 | 11.0 | |
| Flash Dont Walk (s) | 16.0 | 16.0 | | | 16.0 | | 18.0 | 18.0 | | 18.0 | 18.0 | |
| Pedestrian Calls (#/hr) | 0 | 0 | | | 0 | | 0 | 0 | | 0 | 0 | |
| Act Effct Green (s) | 30.0 | 30.0 | | 51.0 | 48.8 | | 29.0 | 29.0 | | 29.0 | 29.0 | |
| Actuated g/C Ratio | 0.33 | 0.33 | | 0.57 | 0.54 | | 0.32 | 0.32 | | 0.32 | 0.32 | |

Lanes, Volumes, Timings
 3: Wentworth Street N & Burlington St E/Burlington Street E

AM Peak Period
 05-16-2019



| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|----------------|------|------|-----|------|------|-----|-----|------|-----|-----|-----|------|
| v/c Ratio | 0.01 | 0.53 | | 0.46 | 0.60 | | | 0.15 | | | | 0.05 |
| Control Delay | 20.5 | 26.2 | | 13.1 | 15.8 | | | 6.3 | | | | 14.9 |
| Queue Delay | 0.0 | 0.0 | | 0.0 | 0.0 | | | 0.0 | | | | 0.0 |
| Total Delay | 20.5 | 26.2 | | 13.1 | 15.8 | | | 6.3 | | | | 14.9 |
| LOS | C | C | | B | B | | | A | | | | B |
| Approach Delay | | 26.2 | | | 15.3 | | | 6.3 | | | | 14.9 |
| Approach LOS | | C | | | B | | | A | | | | B |

Intersection Summary

| | |
|-----------------------------------|--|
| Area Type: | Other |
| Cycle Length: | 90 |
| Actuated Cycle Length: | 90 |
| Offset: | 0 (0%), Referenced to phase 2:EBTL, Start of Green |
| Natural Cycle: | 75 |
| Control Type: | Pretimed |
| Maximum v/c Ratio: | 0.60 |
| Intersection Signal Delay: | 17.9 |
| Intersection LOS: | B |
| Intersection Capacity Utilization | 65.7% |
| ICU Level of Service | C |
| Analysis Period (min) | 15 |

Splits and Phases: 3: Wentworth Street N & Burlington St E/Burlington Street E



Lanes, Volumes, Timings
9: Birch Avenue & Burlington Street E

AM Peak Period
05-16-2019



| Lane Group | EBT | EBR | WBL | WBT | NBL | NBR |
|----------------------------|-------|-------|-------|-------|-------|-------|
| Lane Configurations | ↑↑↑ | | ↵↵ | ↑↑↑ | | |
| Traffic Volume (vph) | 613 | 22 | 251 | 1392 | 0 | 0 |
| Future Volume (vph) | 613 | 22 | 251 | 1392 | 0 | 0 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (m) | | 0.0 | 125.0 | | 0.0 | 0.0 |
| Storage Lanes | | 0 | 2 | | 0 | 0 |
| Taper Length (m) | | | 15.0 | | 15.0 | |
| Lane Util. Factor | 0.91 | 0.91 | 0.97 | 0.91 | 1.00 | 1.00 |
| Frt | 0.995 | | | | | |
| Flt Protected | | | 0.950 | | | |
| Satd. Flow (prot) | 4457 | 0 | 3224 | 4821 | 0 | 0 |
| Flt Permitted | | | 0.950 | | | |
| Satd. Flow (perm) | 4457 | 0 | 3224 | 4821 | 0 | 0 |
| Right Turn on Red | | Yes | | | | Yes |
| Satd. Flow (RTOR) | 11 | | | | | |
| Link Speed (k/h) | 50 | | | 50 | 50 | |
| Link Distance (m) | 716.7 | | | 130.8 | 242.7 | |
| Travel Time (s) | 51.6 | | | 9.4 | 17.5 | |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles (%) | 12% | 10% | 5% | 4% | 0% | 0% |
| Adj. Flow (vph) | 666 | 24 | 273 | 1513 | 0 | 0 |
| Shared Lane Traffic (%) | | | | | | |
| Lane Group Flow (vph) | 690 | 0 | 273 | 1513 | 0 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No |
| Lane Alignment | Left | Right | Left | Left | Left | Right |
| Median Width(m) | 6.6 | | | 6.6 | 0.0 | |
| Link Offset(m) | 0.0 | | | 0.0 | 0.0 | |
| Crosswalk Width(m) | 1.6 | | | 1.6 | 1.6 | |
| Two way Left Turn Lane | Yes | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | | 14 | 24 | | 24 | 14 |
| Turn Type | NA | | Prot | NA | | |
| Protected Phases | 2 | | 4 | 2 4 | | |
| Permitted Phases | | | | | | |
| Minimum Split (s) | 47.0 | | 15.1 | | | |
| Total Split (s) | 62.0 | | 28.0 | | | |
| Total Split (%) | 68.9% | | 31.1% | | | |
| Maximum Green (s) | 57.0 | | 22.9 | | | |
| Yellow Time (s) | 3.7 | | 3.7 | | | |
| All-Red Time (s) | 1.3 | | 1.4 | | | |
| Lost Time Adjust (s) | 0.0 | | 0.0 | | | |
| Total Lost Time (s) | 5.0 | | 5.1 | | | |
| Lead/Lag | | | | | | |
| Lead-Lag Optimize? | | | | | | |
| Walk Time (s) | 30.0 | | | | | |
| Flash Dont Walk (s) | 12.0 | | | | | |
| Pedestrian Calls (#/hr) | 0 | | | | | |
| Act Effct Green (s) | 57.0 | | 22.9 | 90.0 | | |
| Actuated g/C Ratio | 0.63 | | 0.25 | 1.00 | | |

Lanes, Volumes, Timings
 9: Birch Avenue & Burlington Street E

AM Peak Period
 05-16-2019



| Lane Group | EBT | EBR | WBL | WBT | NBL | NBR |
|----------------|------|-----|------|------|-----|-----|
| v/c Ratio | 0.24 | | 0.33 | 0.31 | | |
| Control Delay | 16.9 | | 28.7 | 0.2 | | |
| Queue Delay | 0.0 | | 0.0 | 0.0 | | |
| Total Delay | 16.9 | | 28.7 | 0.2 | | |
| LOS | B | | C | A | | |
| Approach Delay | 16.9 | | | 4.5 | | |
| Approach LOS | B | | | A | | |

Intersection Summary

| | |
|-----------------------------------|---|
| Area Type: | Other |
| Cycle Length: | 90 |
| Actuated Cycle Length: | 90 |
| Offset: | 0 (0%), Referenced to phase 2:EBWB and 6:, Start of Green |
| Natural Cycle: | 65 |
| Control Type: | Pretimed |
| Maximum v/c Ratio: | 0.33 |
| Intersection Signal Delay: | 8.0 |
| Intersection LOS: | A |
| Intersection Capacity Utilization | 31.1% |
| ICU Level of Service | A |
| Analysis Period (min) | 15 |

Splits and Phases: 9: Birch Avenue & Burlington Street E



Lanes, Volumes, Timings
13: Wentworth Street N & Brant Street

AM Peak Period
05-16-2019



| Lane Group | WBL | WBR | NBT | NBR | SBL | SBT |
|----------------------------|-------|-------|-------|-------|------|-------|
| Lane Configurations | | | | | | |
| Traffic Volume (vph) | 23 | 7 | 114 | 39 | 4 | 346 |
| Future Volume (vph) | 23 | 7 | 114 | 39 | 4 | 346 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Util. Factor | 1.00 | 1.00 | 0.95 | 0.95 | 0.95 | 0.95 |
| Frt | 0.967 | | 0.962 | | | |
| Flt Protected | 0.964 | | | | | 0.999 |
| Satd. Flow (prot) | 1502 | 0 | 3109 | 0 | 0 | 3252 |
| Flt Permitted | 0.964 | | | | | 0.999 |
| Satd. Flow (perm) | 1502 | 0 | 3109 | 0 | 0 | 3252 |
| Link Speed (k/h) | 48 | | 50 | | | 50 |
| Link Distance (m) | 104.2 | | 236.6 | | | 215.5 |
| Travel Time (s) | 7.8 | | 17.0 | | | 15.5 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles (%) | 14% | 14% | 9% | 5% | 25% | 7% |
| Adj. Flow (vph) | 25 | 8 | 124 | 42 | 4 | 376 |
| Shared Lane Traffic (%) | | | | | | |
| Lane Group Flow (vph) | 33 | 0 | 166 | 0 | 0 | 380 |
| Enter Blocked Intersection | No | No | No | No | No | No |
| Lane Alignment | Left | Right | Left | Right | Left | Left |
| Median Width(m) | 3.3 | | 0.0 | | | 0.0 |
| Link Offset(m) | 0.0 | | 0.0 | | | 0.0 |
| Crosswalk Width(m) | 1.6 | | 1.6 | | | 1.6 |
| Two way Left Turn Lane | | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | 24 | 14 | | 14 | 24 | |
| Sign Control | Stop | | Free | | | Free |

Intersection Summary

| | |
|-----------------------------------|--------------|
| Area Type: | Other |
| Control Type: | Unsignalized |
| Intersection Capacity Utilization | 22.4% |
| ICU Level of Service | A |
| Analysis Period (min) | 15 |

Lanes, Volumes, Timings
15: Brant Street & Birch Avenue

AM Peak Period
05-16-2019



| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|----------------------------|------|-------|-------|-------|-------|-------|------|-------|-------|-------|-------|-------|
| Lane Configurations | | | | | | | | | | | | |
| Traffic Volume (vph) | 0 | 22 | 30 | 26 | 31 | 0 | 0 | 0 | 0 | 19 | 196 | 15 |
| Future Volume (vph) | 0 | 22 | 30 | 26 | 31 | 0 | 0 | 0 | 0 | 19 | 196 | 15 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (m) | 0.0 | | 0.0 | 0.0 | | 0.0 | 0.0 | | 0.0 | 0.0 | | 150.0 |
| Storage Lanes | 0 | | 0 | 0 | | 0 | 0 | | 0 | 0 | | 0 |
| Taper Length (m) | 15.0 | | | 15.0 | | | 15.0 | | | 15.0 | | |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.91 | 0.91 | 0.91 |
| Frt | | 0.922 | | | | | | | | | | 0.990 |
| Flt Protected | | | | | 0.978 | | | | | | | 0.996 |
| Satd. Flow (prot) | 0 | 1400 | 0 | 0 | 1649 | 0 | 0 | 0 | 0 | 0 | 4466 | 0 |
| Flt Permitted | | | | | 0.891 | | | | | | | 0.996 |
| Satd. Flow (perm) | 0 | 1400 | 0 | 0 | 1502 | 0 | 0 | 0 | 0 | 0 | 4466 | 0 |
| Right Turn on Red | | | Yes | | | Yes | | | Yes | | | Yes |
| Satd. Flow (RTOR) | | 33 | | | | | | | | | | 15 |
| Link Speed (k/h) | | 50 | | | 50 | | | 50 | | | | 50 |
| Link Distance (m) | | 157.8 | | | 106.5 | | | 391.5 | | | | 242.7 |
| Travel Time (s) | | 11.4 | | | 7.7 | | | 28.2 | | | | 17.5 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles (%) | 0% | 36% | 10% | 4% | 13% | 0% | 0% | 0% | 0% | 16% | 10% | 13% |
| Adj. Flow (vph) | 0 | 24 | 33 | 28 | 34 | 0 | 0 | 0 | 0 | 21 | 213 | 16 |
| Shared Lane Traffic (%) | | | | | | | | | | | | |
| Lane Group Flow (vph) | 0 | 57 | 0 | 0 | 62 | 0 | 0 | 0 | 0 | 0 | 250 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(m) | | 0.0 | | | 0.0 | | | 0.0 | | | | 0.0 |
| Link Offset(m) | | 0.0 | | | 0.0 | | | 0.0 | | | | 0.0 |
| Crosswalk Width(m) | | 1.6 | | | 1.6 | | | 1.6 | | | | 1.6 |
| Two way Left Turn Lane | | | | | | | | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | 24 | | 14 | 24 | | 14 | 24 | | 14 | 24 | | 14 |
| Turn Type | | NA | | Perm | NA | | | | | | Perm | NA |
| Protected Phases | | 4 | | | 8 | | | | | | | 2 |
| Permitted Phases | | | | 8 | | | | | | 2 | | |
| Minimum Split (s) | | 32.5 | | 32.5 | 32.5 | | | | | 23.4 | 23.4 | |
| Total Split (s) | | 46.0 | | 46.0 | 46.0 | | | | | 44.0 | 44.0 | |
| Total Split (%) | | 51.1% | | 51.1% | 51.1% | | | | | 48.9% | 48.9% | |
| Maximum Green (s) | | 40.5 | | 40.5 | 40.5 | | | | | 38.6 | 38.6 | |
| Yellow Time (s) | | 3.3 | | 3.3 | 3.3 | | | | | 3.3 | 3.3 | |
| All-Red Time (s) | | 2.2 | | 2.2 | 2.2 | | | | | 2.1 | 2.1 | |
| Lost Time Adjust (s) | | 0.0 | | | 0.0 | | | | | | | 0.0 |
| Total Lost Time (s) | | 5.5 | | | 5.5 | | | | | | | 5.4 |
| Lead/Lag | | | | | | | | | | | | |
| Lead-Lag Optimize? | | | | | | | | | | | | |
| Walk Time (s) | | 17.0 | | 17.0 | 17.0 | | | | | 7.0 | 7.0 | |
| Flash Dont Walk (s) | | 10.0 | | 10.0 | 10.0 | | | | | 11.0 | 11.0 | |
| Pedestrian Calls (#/hr) | | 0 | | 0 | 0 | | | | | 0 | 0 | |
| Act Effct Green (s) | | 40.5 | | | 40.5 | | | | | | | 38.6 |
| Actuated g/C Ratio | | 0.45 | | | 0.45 | | | | | | | 0.43 |

Lanes, Volumes, Timings
15: Brant Street & Birch Avenue

AM Peak Period
05-16-2019

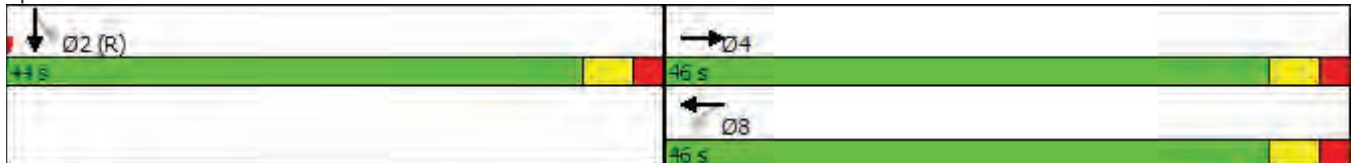


| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|----------------|-----|------|-----|-----|------|-----|-----|-----|-----|-----|-----|------|
| v/c Ratio | | 0.09 | | | 0.09 | | | | | | | 0.13 |
| Control Delay | | 8.1 | | | 14.8 | | | | | | | 7.7 |
| Queue Delay | | 0.0 | | | 0.0 | | | | | | | 0.0 |
| Total Delay | | 8.1 | | | 14.8 | | | | | | | 7.7 |
| LOS | | A | | | B | | | | | | | A |
| Approach Delay | | 8.1 | | | 14.8 | | | | | | | 7.7 |
| Approach LOS | | A | | | B | | | | | | | A |

Intersection Summary

| | |
|-----------------------------------|---|
| Area Type: | Other |
| Cycle Length: | 90 |
| Actuated Cycle Length: | 90 |
| Offset: | 0 (0%), Referenced to phase 2:SBTL and 6:, Start of Green |
| Natural Cycle: | 60 |
| Control Type: | Pretimed |
| Maximum v/c Ratio: | 0.13 |
| Intersection Signal Delay: | 8.9 |
| Intersection LOS: | A |
| Intersection Capacity Utilization | 27.2% |
| ICU Level of Service | A |
| Analysis Period (min) | 15 |

Splits and Phases: 15: Brant Street & Birch Avenue



Lanes, Volumes, Timings
25: Hillyard Street & Brant Street

AM Peak Period
05-16-2019



| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|----------------------------|------|-------|-------|------|-------|-------|------|-------|-------|------|-------|-------|
| Lane Configurations | | ↕ | | | ↕ | | | ↕ | | | ↕ | |
| Traffic Volume (vph) | 6 | 24 | 3 | 10 | 12 | 4 | 2 | 10 | 6 | 1 | 7 | 8 |
| Future Volume (vph) | 6 | 24 | 3 | 10 | 12 | 4 | 2 | 10 | 6 | 1 | 7 | 8 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt | | 0.989 | | | 0.981 | | | 0.953 | | | 0.932 | |
| Flt Protected | | 0.990 | | | 0.981 | | | 0.995 | | | 0.997 | |
| Satd. Flow (prot) | 0 | 1748 | 0 | 0 | 1589 | 0 | 0 | 1651 | 0 | 0 | 1514 | 0 |
| Flt Permitted | | 0.990 | | | 0.981 | | | 0.995 | | | 0.997 | |
| Satd. Flow (perm) | 0 | 1748 | 0 | 0 | 1589 | 0 | 0 | 1651 | 0 | 0 | 1514 | 0 |
| Link Speed (k/h) | | 48 | | | 48 | | | 48 | | | 48 | |
| Link Distance (m) | | 103.4 | | | 96.5 | | | 137.3 | | | 105.1 | |
| Travel Time (s) | | 7.8 | | | 7.2 | | | 10.3 | | | 7.9 | |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles (%) | 0% | 4% | 0% | 10% | 8% | 25% | 0% | 10% | 0% | 0% | 14% | 13% |
| Adj. Flow (vph) | 7 | 26 | 3 | 11 | 13 | 4 | 2 | 11 | 7 | 1 | 8 | 9 |
| Shared Lane Traffic (%) | | | | | | | | | | | | |
| Lane Group Flow (vph) | 0 | 36 | 0 | 0 | 28 | 0 | 0 | 20 | 0 | 0 | 18 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(m) | | 0.0 | | | 0.0 | | | 0.0 | | | 0.0 | |
| Link Offset(m) | | 0.0 | | | 0.0 | | | 0.0 | | | 0.0 | |
| Crosswalk Width(m) | | 1.6 | | | 1.6 | | | 1.6 | | | 1.6 | |
| Two way Left Turn Lane | | | | | | | | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | 24 | | 14 | 24 | | 14 | 24 | | 14 | 24 | | 14 |
| Sign Control | | Free | | | Free | | | Stop | | | Stop | |

Intersection Summary

| | |
|-----------------------------------|--------------|
| Area Type: | Other |
| Control Type: | Unsignalized |
| Intersection Capacity Utilization | 13.3% |
| ICU Level of Service | A |
| Analysis Period (min) | 15 |

Lanes, Volumes, Timings
31: Wentworth Street N & Munroe Street

AM Peak Period
05-16-2019



| Lane Group | WBL | WBR | NBT | NBR | SBL | SBT |
|----------------------------|-------|-------|-------|-------|------|-------|
| Lane Configurations | | | | | | |
| Traffic Volume (vph) | 24 | 7 | 149 | 30 | 2 | 290 |
| Future Volume (vph) | 24 | 7 | 149 | 30 | 2 | 290 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Util. Factor | 1.00 | 1.00 | 0.95 | 0.95 | 0.95 | 0.95 |
| Frt | 0.968 | | 0.975 | | | |
| Flt Protected | 0.963 | | | | | |
| Satd. Flow (prot) | 1466 | 0 | 3093 | 0 | 0 | 3194 |
| Flt Permitted | 0.963 | | | | | |
| Satd. Flow (perm) | 1466 | 0 | 3093 | 0 | 0 | 3194 |
| Link Speed (k/h) | 48 | | 50 | | | 50 |
| Link Distance (m) | 118.1 | | 172.4 | | | 236.6 |
| Travel Time (s) | 8.9 | | 12.4 | | | 17.0 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles (%) | 13% | 29% | 10% | 10% | 50% | 9% |
| Adj. Flow (vph) | 26 | 8 | 162 | 33 | 2 | 315 |
| Shared Lane Traffic (%) | | | | | | |
| Lane Group Flow (vph) | 34 | 0 | 195 | 0 | 0 | 317 |
| Enter Blocked Intersection | No | No | No | No | No | No |
| Lane Alignment | Left | Right | Left | Right | Left | Left |
| Median Width(m) | 3.3 | | 0.0 | | | 0.0 |
| Link Offset(m) | 0.0 | | 0.0 | | | 0.0 |
| Crosswalk Width(m) | 1.6 | | 1.6 | | | 1.6 |
| Two way Left Turn Lane | | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | 24 | 14 | | 14 | 24 | |
| Sign Control | Stop | | Free | | | Free |

Intersection Summary

| | |
|-----------------------------------|------------------------|
| Area Type: | Other |
| Control Type: | Unsignalized |
| Intersection Capacity Utilization | 19.4% |
| Analysis Period (min) | 15 |
| | ICU Level of Service A |

Lanes, Volumes, Timings
3: Wentworth Street N & Burlington Street E

PM Peak Period
05-16-2019



| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|----------------------------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|
| Lane Configurations | | | | | | | | | | | | |
| Traffic Volume (vph) | 10 | 600 | 31 | 148 | 610 | 10 | 10 | 6 | 100 | 10 | 12 | 7 |
| Future Volume (vph) | 10 | 600 | 31 | 148 | 610 | 10 | 10 | 6 | 100 | 10 | 12 | 7 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (m) | 60.0 | | 0.0 | 50.0 | | 0.0 | 0.0 | | 0.0 | 0.0 | | 0.0 |
| Storage Lanes | 1 | | 0 | 1 | | 0 | 0 | | 0 | 0 | | 0 |
| Taper Length (m) | 15.0 | | | 15.0 | | | 15.0 | | | 15.0 | | |
| Lane Util. Factor | 1.00 | 0.95 | 0.95 | 1.00 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 1.00 | 1.00 | 1.00 |
| Frt | | 0.993 | | | 0.998 | | | 0.871 | | | 0.966 | |
| Flt Protected | 0.950 | | | 0.950 | | | | 0.996 | | | 0.983 | |
| Satd. Flow (prot) | 1572 | 3078 | 0 | 1572 | 3160 | 0 | 0 | 2662 | 0 | 0 | 1429 | 0 |
| Flt Permitted | 0.395 | | | 0.254 | | | | 0.938 | | | 0.912 | |
| Satd. Flow (perm) | 654 | 3078 | 0 | 420 | 3160 | 0 | 0 | 2507 | 0 | 0 | 1326 | 0 |
| Right Turn on Red | | | Yes | | | Yes | | | Yes | | | Yes |
| Satd. Flow (RTOR) | | 6 | | | 3 | | | 109 | | | 8 | |
| Link Speed (k/h) | | 50 | | | 50 | | | 50 | | | 50 | |
| Link Distance (m) | | 125.0 | | | 716.7 | | | 215.5 | | | 91.4 | |
| Travel Time (s) | | 9.0 | | | 51.6 | | | 15.5 | | | 6.6 | |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles (%) | 11% | 12% | 24% | 11% | 10% | 22% | 0% | 0% | 16% | 22% | 11% | 40% |
| Adj. Flow (vph) | 11 | 652 | 34 | 161 | 663 | 11 | 11 | 7 | 109 | 11 | 13 | 8 |
| Shared Lane Traffic (%) | | | | | | | | | | | | |
| Lane Group Flow (vph) | 11 | 686 | 0 | 161 | 674 | 0 | 0 | 127 | 0 | 0 | 32 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(m) | | 3.3 | | | 3.3 | | | 0.0 | | | 0.0 | |
| Link Offset(m) | | 0.0 | | | 0.0 | | | 0.0 | | | 0.0 | |
| Crosswalk Width(m) | | 4.9 | | | 4.9 | | | 4.9 | | | 4.9 | |
| Two way Left Turn Lane | | | | | Yes | | | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | 24 | | 14 | 24 | | 14 | 24 | | 14 | 24 | | 14 |
| Turn Type | Perm | NA | | pm+pt | NA | | Perm | NA | | Perm | NA | |
| Protected Phases | | 2 | | 1 | 6 | | | 4 | | | 8 | |
| Permitted Phases | 2 | | | 6 | | | 4 | | | 8 | | |
| Minimum Split (s) | 29.0 | 29.0 | | 9.5 | 37.2 | | 35.0 | 35.0 | | 35.0 | 35.0 | |
| Total Split (s) | 38.0 | 38.0 | | 17.0 | 55.0 | | 35.0 | 35.0 | | 35.0 | 35.0 | |
| Total Split (%) | 42.2% | 42.2% | | 18.9% | 61.1% | | 38.9% | 38.9% | | 38.9% | 38.9% | |
| Maximum Green (s) | 32.0 | 32.0 | | 13.0 | 48.8 | | 29.0 | 29.0 | | 29.0 | 29.0 | |
| Yellow Time (s) | 3.3 | 3.3 | | 3.0 | 3.3 | | 3.3 | 3.3 | | 3.3 | 3.3 | |
| All-Red Time (s) | 2.7 | 2.7 | | 1.0 | 2.9 | | 2.7 | 2.7 | | 2.7 | 2.7 | |
| Lost Time Adjust (s) | 0.0 | 0.0 | | 0.0 | 0.0 | | 0.0 | 0.0 | | 0.0 | 0.0 | |
| Total Lost Time (s) | 6.0 | 6.0 | | 4.0 | 6.2 | | 6.0 | 6.0 | | 6.0 | 6.0 | |
| Lead/Lag | Lag | Lag | | Lead | | | | | | | | |
| Lead-Lag Optimize? | Yes | Yes | | Yes | | | | | | | | |
| Walk Time (s) | 7.0 | 7.0 | | | 15.0 | | 11.0 | 11.0 | | 11.0 | 11.0 | |
| Flash Dont Walk (s) | 16.0 | 16.0 | | | 16.0 | | 18.0 | 18.0 | | 18.0 | 18.0 | |
| Pedestrian Calls (#/hr) | 0 | 0 | | | 0 | | 0 | 0 | | 0 | 0 | |
| Act Effct Green (s) | 32.0 | 32.0 | | 51.0 | 48.8 | | 29.0 | 29.0 | | 29.0 | 29.0 | |
| Actuated g/C Ratio | 0.36 | 0.36 | | 0.57 | 0.54 | | 0.32 | 0.32 | | 0.32 | 0.32 | |

Lanes, Volumes, Timings
3: Wentworth Street N & Burlington Street E

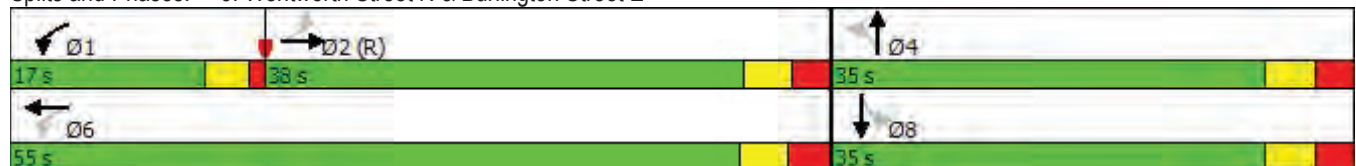
PM Peak Period
05-16-2019



| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|----------------|------|------|-----|------|------|-----|-----|------|-----|-----|-----|------|
| v/c Ratio | 0.05 | 0.62 | | 0.40 | 0.39 | | | 0.14 | | | | 0.07 |
| Control Delay | 19.9 | 26.9 | | 12.5 | 12.8 | | | 6.5 | | | | 17.9 |
| Queue Delay | 0.0 | 0.0 | | 0.0 | 0.0 | | | 0.0 | | | | 0.0 |
| Total Delay | 19.9 | 26.9 | | 12.5 | 12.8 | | | 6.5 | | | | 17.9 |
| LOS | B | C | | B | B | | | A | | | | B |
| Approach Delay | | 26.7 | | | 12.7 | | | 6.5 | | | | 17.9 |
| Approach LOS | | C | | | B | | | A | | | | B |

| Intersection Summary | |
|-----------------------------------|--|
| Area Type: | Other |
| Cycle Length: | 90 |
| Actuated Cycle Length: | 90 |
| Offset: | 0 (0%), Referenced to phase 2:EBTL, Start of Green |
| Natural Cycle: | 75 |
| Control Type: | Pretimed |
| Maximum v/c Ratio: | 0.62 |
| Intersection Signal Delay: | 18.1 |
| Intersection LOS: | B |
| Intersection Capacity Utilization | 57.3% |
| ICU Level of Service | B |
| Analysis Period (min) | 15 |

Splits and Phases: 3: Wentworth Street N & Burlington Street E



Lanes, Volumes, Timings
9: Birch Avenue & Burlington Street E

PM Peak Period
05-16-2019



| Lane Group | EBT | EBR | WBL | WBT | NBL | NBR |
|----------------------------|-------|-------|-------|-------|-------|-------|
| Lane Configurations | ↑↑↑ | | ↔ | ↑↑↑ | | |
| Traffic Volume (vph) | 1057 | 26 | 236 | 1155 | 0 | 0 |
| Future Volume (vph) | 1057 | 26 | 236 | 1155 | 0 | 0 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (m) | | 0.0 | 125.0 | | 0.0 | 0.0 |
| Storage Lanes | | 0 | 2 | | 0 | 0 |
| Taper Length (m) | | | 15.0 | | 15.0 | |
| Lane Util. Factor | 0.91 | 0.91 | 0.97 | 0.91 | 1.00 | 1.00 |
| Frt | 0.996 | | | | | |
| Flt Protected | | | 0.950 | | | |
| Satd. Flow (prot) | 4718 | 0 | 3134 | 4643 | 0 | 0 |
| Flt Permitted | | | 0.950 | | | |
| Satd. Flow (perm) | 4718 | 0 | 3134 | 4643 | 0 | 0 |
| Right Turn on Red | | Yes | | | | Yes |
| Satd. Flow (RTOR) | 8 | | | | | |
| Link Speed (k/h) | 50 | | | 50 | 50 | |
| Link Distance (m) | 716.7 | | | 130.8 | 242.7 | |
| Travel Time (s) | 51.6 | | | 9.4 | 17.5 | |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles (%) | 6% | 0% | 8% | 8% | 0% | 0% |
| Adj. Flow (vph) | 1149 | 28 | 257 | 1255 | 0 | 0 |
| Shared Lane Traffic (%) | | | | | | |
| Lane Group Flow (vph) | 1177 | 0 | 257 | 1255 | 0 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No |
| Lane Alignment | Left | Right | Left | Left | Left | Right |
| Median Width(m) | 6.6 | | | 6.6 | 0.0 | |
| Link Offset(m) | 0.0 | | | 0.0 | 0.0 | |
| Crosswalk Width(m) | 1.6 | | | 1.6 | 1.6 | |
| Two way Left Turn Lane | Yes | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | | 14 | 24 | | 24 | 14 |
| Turn Type | NA | | Prot | NA | | |
| Protected Phases | 2 | | 4 | 2 4 | | |
| Permitted Phases | | | | | | |
| Minimum Split (s) | 47.0 | | 15.1 | | | |
| Total Split (s) | 64.0 | | 26.0 | | | |
| Total Split (%) | 71.1% | | 28.9% | | | |
| Maximum Green (s) | 59.0 | | 20.9 | | | |
| Yellow Time (s) | 3.7 | | 3.7 | | | |
| All-Red Time (s) | 1.3 | | 1.4 | | | |
| Lost Time Adjust (s) | 0.0 | | 0.0 | | | |
| Total Lost Time (s) | 5.0 | | 5.1 | | | |
| Lead/Lag | | | | | | |
| Lead-Lag Optimize? | | | | | | |
| Walk Time (s) | 30.0 | | | | | |
| Flash Dont Walk (s) | 12.0 | | | | | |
| Pedestrian Calls (#/hr) | 0 | | | | | |
| Act Effct Green (s) | 59.0 | | 20.9 | 90.0 | | |
| Actuated g/C Ratio | 0.66 | | 0.23 | 1.00 | | |

Lanes, Volumes, Timings
 9: Birch Avenue & Burlington Street E

PM Peak Period
 05-16-2019



| Lane Group | EBT | EBR | WBL | WBT | NBL | NBR |
|----------------|------|-----|------|------|-----|-----|
| v/c Ratio | 0.38 | | 0.35 | 0.27 | | |
| Control Delay | 13.7 | | 30.6 | 0.1 | | |
| Queue Delay | 0.0 | | 0.0 | 0.0 | | |
| Total Delay | 13.7 | | 30.6 | 0.1 | | |
| LOS | B | | C | A | | |
| Approach Delay | 13.7 | | | 5.3 | | |
| Approach LOS | B | | | A | | |

Intersection Summary

| | |
|-----------------------------------|---|
| Area Type: | Other |
| Cycle Length: | 90 |
| Actuated Cycle Length: | 90 |
| Offset: | 0 (0%), Referenced to phase 2:EBWB and 6:, Start of Green |
| Natural Cycle: | 65 |
| Control Type: | Pretimed |
| Maximum v/c Ratio: | 0.38 |
| Intersection Signal Delay: | 9.0 |
| Intersection Capacity Utilization | 37.8% |
| Analysis Period (min) | 15 |
| Intersection LOS: | A |
| ICU Level of Service | A |

Splits and Phases: 9: Birch Avenue & Burlington Street E



Lanes, Volumes, Timings
 13: Brant Street & Wentworth Street N

PM Peak Period
 05-16-2019



| Lane Group | WBL | WBR | NBT | NBR | SBL | SBT |
|----------------------------|-------|-------|-------|-------|------|-------|
| Lane Configurations | | | | | | |
| Traffic Volume (vph) | 29 | 3 | 126 | 23 | 10 | 223 |
| Future Volume (vph) | 29 | 3 | 126 | 23 | 10 | 223 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Util. Factor | 1.00 | 1.00 | 0.95 | 0.95 | 0.95 | 0.95 |
| Frt | 0.988 | | 0.977 | | | |
| Flt Protected | 0.956 | | | | | 0.998 |
| Satd. Flow (prot) | 1630 | 0 | 3196 | 0 | 0 | 3235 |
| Flt Permitted | 0.956 | | | | | 0.998 |
| Satd. Flow (perm) | 1630 | 0 | 3196 | 0 | 0 | 3235 |
| Link Speed (k/h) | 48 | | 48 | | | 48 |
| Link Distance (m) | 104.2 | | 236.6 | | | 215.5 |
| Travel Time (s) | 7.8 | | 17.7 | | | 16.2 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles (%) | 7% | 0% | 7% | 5% | 22% | 7% |
| Adj. Flow (vph) | 32 | 3 | 137 | 25 | 11 | 242 |
| Shared Lane Traffic (%) | | | | | | |
| Lane Group Flow (vph) | 35 | 0 | 162 | 0 | 0 | 253 |
| Enter Blocked Intersection | No | No | No | No | No | No |
| Lane Alignment | Left | Right | Left | Right | Left | Left |
| Median Width(m) | 3.3 | | 0.0 | | | 0.0 |
| Link Offset(m) | 0.0 | | 0.0 | | | 0.0 |
| Crosswalk Width(m) | 1.6 | | 1.6 | | | 1.6 |
| Two way Left Turn Lane | | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | 24 | 14 | | 14 | 24 | |
| Sign Control | Stop | | Free | | | Free |

Intersection Summary

| | |
|-----------------------------------|------------------------|
| Area Type: | Other |
| Control Type: | Unsignalized |
| Intersection Capacity Utilization | 23.5% |
| Analysis Period (min) | 15 |
| | ICU Level of Service A |

Lanes, Volumes, Timings
15: Brant Street & Birch Avenue

PM Peak Period
05-16-2019



| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|----------------------------|------|-------|-------|-------|-------|-------|------|-------|-------|-------|-------|-------|
| Lane Configurations | | | | | | | | | | | | |
| Traffic Volume (vph) | 0 | 12 | 39 | 52 | 29 | 0 | 0 | 0 | 0 | 7 | 204 | 10 |
| Future Volume (vph) | 0 | 12 | 39 | 52 | 29 | 0 | 0 | 0 | 0 | 7 | 204 | 10 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (m) | 0.0 | | 0.0 | 0.0 | | 0.0 | 0.0 | | 0.0 | 0.0 | | 150.0 |
| Storage Lanes | 0 | | 0 | 0 | | 0 | 0 | | 0 | 0 | | 0 |
| Taper Length (m) | 15.0 | | | 15.0 | | | 15.0 | | | 15.0 | | |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.91 | 0.91 | 0.91 |
| Frt | | 0.897 | | | | | | | | | | 0.993 |
| Flt Protected | | | | | 0.969 | | | | | | | 0.998 |
| Satd. Flow (prot) | 0 | 1559 | 0 | 0 | 1691 | 0 | 0 | 0 | 0 | 0 | 4749 | 0 |
| Flt Permitted | | | | | 0.822 | | | | | | | 0.998 |
| Satd. Flow (perm) | 0 | 1559 | 0 | 0 | 1435 | 0 | 0 | 0 | 0 | 0 | 4749 | 0 |
| Right Turn on Red | | | Yes | | | Yes | | | Yes | | | Yes |
| Satd. Flow (RTOR) | | 42 | | | | | | | | | | 9 |
| Link Speed (k/h) | | 50 | | | 50 | | | 50 | | | | 50 |
| Link Distance (m) | | 157.8 | | | 106.5 | | | 391.5 | | | | 242.7 |
| Travel Time (s) | | 11.4 | | | 7.7 | | | 28.2 | | | | 17.5 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles (%) | 0% | 8% | 5% | 2% | 11% | 0% | 0% | 0% | 0% | 29% | 3% | 20% |
| Adj. Flow (vph) | 0 | 13 | 42 | 57 | 32 | 0 | 0 | 0 | 0 | 8 | 222 | 11 |
| Shared Lane Traffic (%) | | | | | | | | | | | | |
| Lane Group Flow (vph) | 0 | 55 | 0 | 0 | 89 | 0 | 0 | 0 | 0 | 0 | 241 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(m) | | 0.0 | | | 0.0 | | | 0.0 | | | | 0.0 |
| Link Offset(m) | | 0.0 | | | 0.0 | | | 0.0 | | | | 0.0 |
| Crosswalk Width(m) | | 1.6 | | | 1.6 | | | 1.6 | | | | 1.6 |
| Two way Left Turn Lane | | | | | | | | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | 24 | | 14 | 24 | | 14 | 24 | | 14 | 24 | | 14 |
| Turn Type | | NA | | Perm | NA | | | | | | Perm | NA |
| Protected Phases | | 4 | | | 8 | | | | | | | 2 |
| Permitted Phases | | | | 8 | | | | | | 2 | | |
| Minimum Split (s) | | 32.5 | | 32.5 | 32.5 | | | | | 23.4 | 23.4 | |
| Total Split (s) | | 50.0 | | 50.0 | 50.0 | | | | | 40.0 | 40.0 | |
| Total Split (%) | | 55.6% | | 55.6% | 55.6% | | | | | 44.4% | 44.4% | |
| Maximum Green (s) | | 44.5 | | 44.5 | 44.5 | | | | | 34.6 | 34.6 | |
| Yellow Time (s) | | 3.3 | | 3.3 | 3.3 | | | | | 3.3 | 3.3 | |
| All-Red Time (s) | | 2.2 | | 2.2 | 2.2 | | | | | 2.1 | 2.1 | |
| Lost Time Adjust (s) | | 0.0 | | | 0.0 | | | | | | | 0.0 |
| Total Lost Time (s) | | 5.5 | | | 5.5 | | | | | | | 5.4 |
| Lead/Lag | | | | | | | | | | | | |
| Lead-Lag Optimize? | | | | | | | | | | | | |
| Walk Time (s) | | 17.0 | | 17.0 | 17.0 | | | | | 7.0 | 7.0 | |
| Flash Dont Walk (s) | | 10.0 | | 10.0 | 10.0 | | | | | 11.0 | 11.0 | |
| Pedestrian Calls (#/hr) | | 0 | | 0 | 0 | | | | | 0 | 0 | |
| Act Effct Green (s) | | 44.5 | | | 44.5 | | | | | | | 34.6 |
| Actuated g/C Ratio | | 0.49 | | | 0.49 | | | | | | | 0.38 |

Lanes, Volumes, Timings
 15: Brant Street & Birch Avenue

PM Peak Period
 05-16-2019

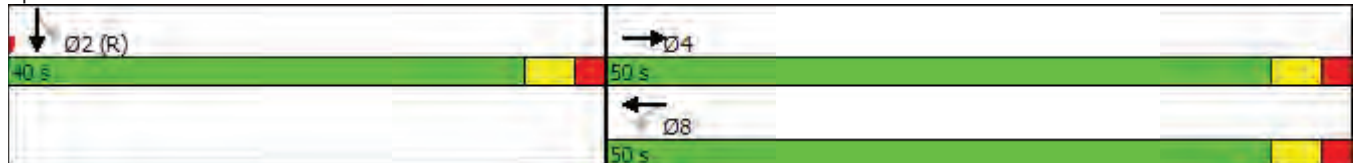


| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|----------------|-----|------|-----|-----|------|-----|-----|-----|-----|-----|-----|------|
| v/c Ratio | | 0.07 | | | 0.13 | | | | | | | 0.13 |
| Control Delay | | 5.5 | | | 12.9 | | | | | | | 6.2 |
| Queue Delay | | 0.0 | | | 0.0 | | | | | | | 0.0 |
| Total Delay | | 5.5 | | | 12.9 | | | | | | | 6.2 |
| LOS | | A | | | B | | | | | | | A |
| Approach Delay | | 5.5 | | | 12.9 | | | | | | | 6.2 |
| Approach LOS | | A | | | B | | | | | | | A |

Intersection Summary

| | |
|-----------------------------------|---|
| Area Type: | Other |
| Cycle Length: | 90 |
| Actuated Cycle Length: | 90 |
| Offset: | 0 (0%), Referenced to phase 2:SBTL and 6:, Start of Green |
| Natural Cycle: | 60 |
| Control Type: | Pretimed |
| Maximum v/c Ratio: | 0.13 |
| Intersection Signal Delay: | 7.6 |
| Intersection LOS: | A |
| Intersection Capacity Utilization | 28.5% |
| ICU Level of Service | A |
| Analysis Period (min) | 15 |

Splits and Phases: 15: Brant Street & Birch Avenue



Lanes, Volumes, Timings
25: Hillyard Street & Brant Street

PM Peak Period
05-16-2019



| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|----------------------------|------|-------|-------|------|-------|-------|------|-------|-------|------|-------|-------|
| Lane Configurations | | ↕ | | | ↕ | | | ↕ | | | ↕ | |
| Traffic Volume (vph) | 1 | 19 | 1 | 4 | 22 | 0 | 3 | 5 | 7 | 7 | 4 | 13 |
| Future Volume (vph) | 1 | 19 | 1 | 4 | 22 | 0 | 3 | 5 | 7 | 7 | 4 | 13 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Fr _t | | 0.994 | | | | | | 0.932 | | | 0.927 | |
| Fl _t Protected | | 0.998 | | | 0.993 | | | 0.991 | | | 0.985 | |
| Satd. Flow (prot) | 0 | 1656 | 0 | 0 | 1824 | 0 | 0 | 1585 | 0 | 0 | 1659 | 0 |
| Fl _t Permitted | | 0.998 | | | 0.993 | | | 0.991 | | | 0.985 | |
| Satd. Flow (perm) | 0 | 1656 | 0 | 0 | 1824 | 0 | 0 | 1585 | 0 | 0 | 1659 | 0 |
| Link Speed (k/h) | | 48 | | | 48 | | | 48 | | | 48 | |
| Link Distance (m) | | 103.4 | | | 96.5 | | | 137.3 | | | 105.1 | |
| Travel Time (s) | | 7.8 | | | 7.2 | | | 10.3 | | | 7.9 | |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles (%) | 0% | 11% | 0% | 0% | 0% | 0% | 0% | 0% | 14% | 0% | 0% | 2% |
| Adj. Flow (vph) | 1 | 21 | 1 | 4 | 24 | 0 | 3 | 5 | 8 | 8 | 4 | 14 |
| Shared Lane Traffic (%) | | | | | | | | | | | | |
| Lane Group Flow (vph) | 0 | 23 | 0 | 0 | 28 | 0 | 0 | 16 | 0 | 0 | 26 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(m) | | 0.0 | | | 0.0 | | | 0.0 | | | 0.0 | |
| Link Offset(m) | | 0.0 | | | 0.0 | | | 0.0 | | | 0.0 | |
| Crosswalk Width(m) | | 1.6 | | | 1.6 | | | 1.6 | | | 1.6 | |
| Two way Left Turn Lane | | | | | | | | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | 24 | | 14 | 24 | | 14 | 24 | | 14 | 24 | | 14 |
| Sign Control | | Free | | | Free | | | Stop | | | Stop | |

Intersection Summary

| | |
|-----------------------------------|--------------|
| Area Type: | Other |
| Control Type: | Unsignalized |
| Intersection Capacity Utilization | 13.3% |
| ICU Level of Service | A |
| Analysis Period (min) | 15 |

Lanes, Volumes, Timings
 31: Wentworth Street N & Munroe Street

PM Peak Period
 05-16-2019



| Lane Group | WBL | WBR | NBT | NBR | SBL | SBT |
|----------------------------|-------|-------|-------|-------|------|-------|
| Lane Configurations | | | | | | |
| Traffic Volume (vph) | 40 | 1 | 175 | 30 | 1 | 273 |
| Future Volume (vph) | 40 | 1 | 175 | 30 | 1 | 273 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Util. Factor | 1.00 | 1.00 | 0.95 | 0.95 | 0.95 | 0.95 |
| Frt | 0.997 | | 0.978 | | | |
| Flt Protected | 0.953 | | | | | |
| Satd. Flow (prot) | 1664 | 0 | 3176 | 0 | 0 | 3232 |
| Flt Permitted | 0.953 | | | | | |
| Satd. Flow (perm) | 1664 | 0 | 3176 | 0 | 0 | 3232 |
| Link Speed (k/h) | 48 | | 48 | | | 48 |
| Link Distance (m) | 118.1 | | 172.4 | | | 236.6 |
| Travel Time (s) | 8.9 | | 12.9 | | | 17.7 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles (%) | 5% | 0% | 7% | 10% | 0% | 8% |
| Adj. Flow (vph) | 43 | 1 | 190 | 33 | 1 | 297 |
| Shared Lane Traffic (%) | | | | | | |
| Lane Group Flow (vph) | 44 | 0 | 223 | 0 | 0 | 298 |
| Enter Blocked Intersection | No | No | No | No | No | No |
| Lane Alignment | Left | Right | Left | Right | Left | Left |
| Median Width(m) | 3.3 | | 0.0 | | | 0.0 |
| Link Offset(m) | 0.0 | | 0.0 | | | 0.0 |
| Crosswalk Width(m) | 1.6 | | 1.6 | | | 1.6 |
| Two way Left Turn Lane | | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | 24 | 14 | | 14 | 24 | |
| Sign Control | Stop | | Free | | | Free |

Intersection Summary

| | |
|-----------------------------------|------------------------|
| Area Type: | Other |
| Control Type: | Unsignalized |
| Intersection Capacity Utilization | 18.2% |
| Analysis Period (min) | 15 |
| | ICU Level of Service A |

APPENDIX C

Synchro Outputs – Future Background 2022 Conditions

Lanes, Volumes, Timings
 3: Wentworth Street N & Burlington St E/Burlington Street E

AM Peak Period
 05-16-2019



| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|----------------------------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|
| Lane Configurations | | | | | | | | | | | | |
| Traffic Volume (vph) | 2 | 527 | 24 | 216 | 972 | 9 | 7 | 10 | 114 | 8 | 3 | 10 |
| Future Volume (vph) | 2 | 527 | 24 | 216 | 972 | 9 | 7 | 10 | 114 | 8 | 3 | 10 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (m) | 60.0 | | 0.0 | 50.0 | | 0.0 | 0.0 | | 0.0 | 0.0 | | 0.0 |
| Storage Lanes | 1 | | 0 | 1 | | 0 | 0 | | 0 | 0 | | 0 |
| Taper Length (m) | 15.0 | | | 15.0 | | | 15.0 | | | 15.0 | | |
| Lane Util. Factor | 1.00 | 0.95 | 0.95 | 1.00 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 1.00 | 1.00 | 1.00 |
| Frt | | 0.993 | | | 0.999 | | | 0.870 | | | 0.935 | |
| Flt Protected | 0.950 | | | 0.950 | | | | 0.997 | | | 0.981 | |
| Satd. Flow (prot) | 1745 | 3188 | 0 | 1616 | 3082 | 0 | 0 | 2742 | 0 | 0 | 1400 | 0 |
| Flt Permitted | 0.268 | | | 0.290 | | | | 0.945 | | | 0.905 | |
| Satd. Flow (perm) | 492 | 3188 | 0 | 493 | 3082 | 0 | 0 | 2599 | 0 | 0 | 1292 | 0 |
| Right Turn on Red | | | Yes | | | Yes | | | Yes | | | Yes |
| Satd. Flow (RTOR) | | 5 | | | 2 | | | 124 | | | 11 | |
| Link Speed (k/h) | | 50 | | | 50 | | | 50 | | | 50 | |
| Link Distance (m) | | 125.0 | | | 716.7 | | | 215.5 | | | 91.4 | |
| Travel Time (s) | | 9.0 | | | 51.6 | | | 15.5 | | | 6.6 | |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles (%) | 0% | 8% | 24% | 8% | 13% | 25% | 0% | 0% | 12% | 14% | 33% | 22% |
| Adj. Flow (vph) | 2 | 573 | 26 | 235 | 1057 | 10 | 8 | 11 | 124 | 9 | 3 | 11 |
| Shared Lane Traffic (%) | | | | | | | | | | | | |
| Lane Group Flow (vph) | 2 | 599 | 0 | 235 | 1067 | 0 | 0 | 143 | 0 | 0 | 23 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(m) | | 3.3 | | | 3.3 | | | 0.0 | | | 0.0 | |
| Link Offset(m) | | 0.0 | | | 0.0 | | | 0.0 | | | 0.0 | |
| Crosswalk Width(m) | | 4.9 | | | 4.9 | | | 4.9 | | | 4.9 | |
| Two way Left Turn Lane | | | | | Yes | | | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | 24 | | 14 | 24 | | 14 | 24 | | 14 | 24 | | 14 |
| Turn Type | Perm | NA | | pm+pt | NA | | Perm | NA | | Perm | NA | |
| Protected Phases | | 2 | | 1 | 6 | | | 4 | | | 8 | |
| Permitted Phases | 2 | | | 6 | | | 4 | | | 8 | | |
| Minimum Split (s) | 29.0 | 29.0 | | 9.5 | 37.2 | | 35.0 | 35.0 | | 35.0 | 35.0 | |
| Total Split (s) | 36.0 | 36.0 | | 19.0 | 55.0 | | 35.0 | 35.0 | | 35.0 | 35.0 | |
| Total Split (%) | 40.0% | 40.0% | | 21.1% | 61.1% | | 38.9% | 38.9% | | 38.9% | 38.9% | |
| Maximum Green (s) | 30.0 | 30.0 | | 15.0 | 48.8 | | 29.0 | 29.0 | | 29.0 | 29.0 | |
| Yellow Time (s) | 3.3 | 3.3 | | 3.0 | 3.3 | | 3.3 | 3.3 | | 3.3 | 3.3 | |
| All-Red Time (s) | 2.7 | 2.7 | | 1.0 | 2.9 | | 2.7 | 2.7 | | 2.7 | 2.7 | |
| Lost Time Adjust (s) | 0.0 | 0.0 | | 0.0 | 0.0 | | 0.0 | 0.0 | | 0.0 | 0.0 | |
| Total Lost Time (s) | 6.0 | 6.0 | | 4.0 | 6.2 | | 6.0 | 6.0 | | 6.0 | 6.0 | |
| Lead/Lag | Lag | Lag | | Lead | | | | | | | | |
| Lead-Lag Optimize? | Yes | Yes | | Yes | | | | | | | | |
| Walk Time (s) | 7.0 | 7.0 | | | 15.0 | | 11.0 | 11.0 | | 11.0 | 11.0 | |
| Flash Dont Walk (s) | 16.0 | 16.0 | | | 16.0 | | 18.0 | 18.0 | | 18.0 | 18.0 | |
| Pedestrian Calls (#/hr) | 0 | 0 | | | 0 | | 0 | 0 | | 0 | 0 | |
| Act Effct Green (s) | 30.0 | 30.0 | | 51.0 | 48.8 | | 29.0 | 29.0 | | 29.0 | 29.0 | |
| Actuated g/C Ratio | 0.33 | 0.33 | | 0.57 | 0.54 | | 0.32 | 0.32 | | 0.32 | 0.32 | |

Lanes, Volumes, Timings
 3: Wentworth Street N & Burlington St E/Burlington Street E

AM Peak Period
 05-16-2019

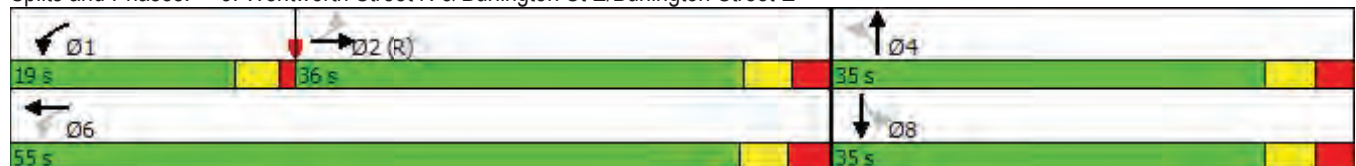


| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|----------------|------|------|-----|------|------|-----|-----|------|-----|-----|-----|------|
| v/c Ratio | 0.01 | 0.56 | | 0.50 | 0.64 | | | 0.16 | | | | 0.05 |
| Control Delay | 20.5 | 26.9 | | 13.8 | 16.5 | | | 6.2 | | | | 15.0 |
| Queue Delay | 0.0 | 0.0 | | 0.0 | 0.0 | | | 0.0 | | | | 0.0 |
| Total Delay | 20.5 | 26.9 | | 13.8 | 16.5 | | | 6.2 | | | | 15.0 |
| LOS | C | C | | B | B | | | A | | | | B |
| Approach Delay | | 26.8 | | | 16.0 | | | 6.2 | | | | 15.0 |
| Approach LOS | | C | | | B | | | A | | | | B |

Intersection Summary

| | |
|-----------------------------------|--|
| Area Type: | Other |
| Cycle Length: | 90 |
| Actuated Cycle Length: | 90 |
| Offset: | 0 (0%), Referenced to phase 2:EBTL, Start of Green |
| Natural Cycle: | 75 |
| Control Type: | Pretimed |
| Maximum v/c Ratio: | 0.64 |
| Intersection Signal Delay: | 18.5 |
| Intersection LOS: | B |
| Intersection Capacity Utilization | 67.4% |
| ICU Level of Service | C |
| Analysis Period (min) | 15 |

Splits and Phases: 3: Wentworth Street N & Burlington St E/Burlington Street E



Lanes, Volumes, Timings
9: Birch Avenue & Burlington Street E

AM Peak Period
05-16-2019



| Lane Group | EBT | EBR | WBL | WBT | NBL | NBR |
|----------------------------|-------|-------|-------|-------|-------|-------|
| Lane Configurations | ↑↑↑ | | ↔ | ↑↑↑ | | |
| Traffic Volume (vph) | 650 | 23 | 266 | 1477 | 0 | 0 |
| Future Volume (vph) | 650 | 23 | 266 | 1477 | 0 | 0 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (m) | | 0.0 | 125.0 | | 0.0 | 0.0 |
| Storage Lanes | | 0 | 2 | | 0 | 0 |
| Taper Length (m) | | | 15.0 | | 15.0 | |
| Lane Util. Factor | 0.91 | 0.91 | 0.97 | 0.91 | 1.00 | 1.00 |
| Frt | 0.995 | | | | | |
| Flt Protected | | | 0.950 | | | |
| Satd. Flow (prot) | 4457 | 0 | 3224 | 4821 | 0 | 0 |
| Flt Permitted | | | 0.950 | | | |
| Satd. Flow (perm) | 4457 | 0 | 3224 | 4821 | 0 | 0 |
| Right Turn on Red | | Yes | | | | Yes |
| Satd. Flow (RTOR) | 11 | | | | | |
| Link Speed (k/h) | 50 | | | 50 | 50 | |
| Link Distance (m) | 716.7 | | | 130.8 | 242.7 | |
| Travel Time (s) | 51.6 | | | 9.4 | 17.5 | |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles (%) | 12% | 10% | 5% | 4% | 0% | 0% |
| Adj. Flow (vph) | 707 | 25 | 289 | 1605 | 0 | 0 |
| Shared Lane Traffic (%) | | | | | | |
| Lane Group Flow (vph) | 732 | 0 | 289 | 1605 | 0 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No |
| Lane Alignment | Left | Right | Left | Left | Left | Right |
| Median Width(m) | 6.6 | | | 6.6 | 0.0 | |
| Link Offset(m) | 0.0 | | | 0.0 | 0.0 | |
| Crosswalk Width(m) | 1.6 | | | 1.6 | 1.6 | |
| Two way Left Turn Lane | Yes | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | | 14 | 24 | | 24 | 14 |
| Turn Type | NA | | Prot | NA | | |
| Protected Phases | 2 | | 4 | 2 4 | | |
| Permitted Phases | | | | | | |
| Minimum Split (s) | 47.0 | | 15.1 | | | |
| Total Split (s) | 62.0 | | 28.0 | | | |
| Total Split (%) | 68.9% | | 31.1% | | | |
| Maximum Green (s) | 57.0 | | 22.9 | | | |
| Yellow Time (s) | 3.7 | | 3.7 | | | |
| All-Red Time (s) | 1.3 | | 1.4 | | | |
| Lost Time Adjust (s) | 0.0 | | 0.0 | | | |
| Total Lost Time (s) | 5.0 | | 5.1 | | | |
| Lead/Lag | | | | | | |
| Lead-Lag Optimize? | | | | | | |
| Walk Time (s) | 30.0 | | | | | |
| Flash Dont Walk (s) | 12.0 | | | | | |
| Pedestrian Calls (#/hr) | 0 | | | | | |
| Act Effct Green (s) | 57.0 | | 22.9 | 90.0 | | |
| Actuated g/C Ratio | 0.63 | | 0.25 | 1.00 | | |

Lanes, Volumes, Timings
 9: Birch Avenue & Burlington Street E

AM Peak Period
 05-16-2019



| Lane Group | EBT | EBR | WBL | WBT | NBL | NBR |
|----------------|------|-----|------|------|-----|-----|
| v/c Ratio | 0.26 | | 0.35 | 0.33 | | |
| Control Delay | 17.6 | | 29.0 | 0.2 | | |
| Queue Delay | 0.0 | | 0.0 | 0.0 | | |
| Total Delay | 17.6 | | 29.0 | 0.2 | | |
| LOS | B | | C | A | | |
| Approach Delay | 17.6 | | | 4.6 | | |
| Approach LOS | B | | | A | | |

Intersection Summary

| | |
|-----------------------------------|---|
| Area Type: | Other |
| Cycle Length: | 90 |
| Actuated Cycle Length: | 90 |
| Offset: | 0 (0%), Referenced to phase 2:EBWB and 6:, Start of Green |
| Natural Cycle: | 65 |
| Control Type: | Pretimed |
| Maximum v/c Ratio: | 0.35 |
| Intersection Signal Delay: | 8.2 |
| Intersection LOS: | A |
| Intersection Capacity Utilization | 32.7% |
| ICU Level of Service | A |
| Analysis Period (min) | 15 |

Splits and Phases: 9: Birch Avenue & Burlington Street E



Lanes, Volumes, Timings
 13: Wentworth Street N & Brant Street

AM Peak Period
 05-16-2019



| Lane Group | WBL | WBR | NBT | NBR | SBL | SBT |
|----------------------------|-------|-------|-------|-------|------|-------|
| Lane Configurations | | | | | | |
| Traffic Volume (vph) | 25 | 8 | 120 | 42 | 5 | 367 |
| Future Volume (vph) | 25 | 8 | 120 | 42 | 5 | 367 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Util. Factor | 1.00 | 1.00 | 0.95 | 0.95 | 0.95 | 0.95 |
| Frt | 0.966 | | 0.961 | | | |
| Flt Protected | 0.964 | | | | | 0.999 |
| Satd. Flow (prot) | 1500 | 0 | 3106 | 0 | 0 | 3251 |
| Flt Permitted | 0.964 | | | | | 0.999 |
| Satd. Flow (perm) | 1500 | 0 | 3106 | 0 | 0 | 3251 |
| Link Speed (k/h) | 48 | | 50 | | | 50 |
| Link Distance (m) | 104.2 | | 236.6 | | | 215.5 |
| Travel Time (s) | 7.8 | | 17.0 | | | 15.5 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles (%) | 14% | 14% | 9% | 5% | 25% | 7% |
| Adj. Flow (vph) | 27 | 9 | 130 | 46 | 5 | 399 |
| Shared Lane Traffic (%) | | | | | | |
| Lane Group Flow (vph) | 36 | 0 | 176 | 0 | 0 | 404 |
| Enter Blocked Intersection | No | No | No | No | No | No |
| Lane Alignment | Left | Right | Left | Right | Left | Left |
| Median Width(m) | 3.3 | | 0.0 | | | 0.0 |
| Link Offset(m) | 0.0 | | 0.0 | | | 0.0 |
| Crosswalk Width(m) | 1.6 | | 1.6 | | | 1.6 |
| Two way Left Turn Lane | | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | 24 | 14 | | 14 | 24 | |
| Sign Control | Stop | | Free | | | Free |

Intersection Summary

| | |
|-----------------------------------|------------------------|
| Area Type: | Other |
| Control Type: | Unsignalized |
| Intersection Capacity Utilization | 23.7% |
| Analysis Period (min) | 15 |
| | ICU Level of Service A |

Lanes, Volumes, Timings
 14: Site Access #2 & Brant Street

AM Peak Period
 05-16-2019



| Lane Group | EBT | EBR | WBL | WBT | NBL | NBR |
|----------------------------|------|-------|------|-------|------|-------|
| Lane Configurations | | | | | | |
| Traffic Volume (vph) | 55 | 0 | 0 | 49 | 0 | 0 |
| Future Volume (vph) | 55 | 0 | 0 | 49 | 0 | 0 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt | | | | | | |
| Flt Protected | | | | | | |
| Satd. Flow (prot) | 1766 | 0 | 0 | 1625 | 918 | 918 |
| Flt Permitted | | | | | | |
| Satd. Flow (perm) | 1766 | 0 | 0 | 1625 | 918 | 918 |
| Link Speed (k/h) | 48 | | | 48 | 48 | |
| Link Distance (m) | 77.1 | | | 157.8 | 61.8 | |
| Travel Time (s) | 5.8 | | | 11.8 | 4.6 | |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles (%) | 4% | 100% | 100% | 13% | 100% | 100% |
| Adj. Flow (vph) | 60 | 0 | 0 | 53 | 0 | 0 |
| Shared Lane Traffic (%) | | | | | | |
| Lane Group Flow (vph) | 60 | 0 | 0 | 53 | 0 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No |
| Lane Alignment | Left | Right | Left | Left | Left | Right |
| Median Width(m) | 0.0 | | | 0.0 | 3.3 | |
| Link Offset(m) | 0.0 | | | 0.0 | 0.0 | |
| Crosswalk Width(m) | 1.6 | | | 1.6 | 1.6 | |
| Two way Left Turn Lane | | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | | 14 | 24 | | 24 | 14 |
| Sign Control | Free | | | Free | Stop | |

Intersection Summary

| | |
|-----------------------------------|------------------------|
| Area Type: | Other |
| Control Type: | Unsignalized |
| Intersection Capacity Utilization | 13.3% |
| Analysis Period (min) | 15 |
| | ICU Level of Service A |

Lanes, Volumes, Timings
15: Brant Street & Birch Avenue

AM Peak Period
05-16-2019



| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|----------------------------|------|-------|-------|-------|-------|-------|------|------|-------|-------|-------|-------|
| Lane Configurations | | | | | | | | | | | | |
| Traffic Volume (vph) | 0 | 24 | 31 | 27 | 32 | 0 | 0 | 0 | 0 | 21 | 208 | 16 |
| Future Volume (vph) | 0 | 24 | 31 | 27 | 32 | 0 | 0 | 0 | 0 | 21 | 208 | 16 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (m) | 0.0 | | 0.0 | 0.0 | | 0.0 | 0.0 | | 0.0 | 0.0 | | 150.0 |
| Storage Lanes | 0 | | 0 | 0 | | 0 | 0 | | 0 | 0 | | 0 |
| Taper Length (m) | 15.0 | | | 15.0 | | | 15.0 | | | 15.0 | | |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.91 | 0.91 | 0.91 |
| Frt | | 0.923 | | | | | | | | | | 0.990 |
| Flt Protected | | | | | 0.978 | | | | | | | 0.996 |
| Satd. Flow (prot) | 0 | 1398 | 0 | 0 | 1649 | 0 | 0 | 0 | 0 | 0 | 4466 | 0 |
| Flt Permitted | | | | | 0.888 | | | | | | | 0.996 |
| Satd. Flow (perm) | 0 | 1398 | 0 | 0 | 1497 | 0 | 0 | 0 | 0 | 0 | 4466 | 0 |
| Right Turn on Red | | | Yes | | | Yes | | | Yes | | | Yes |
| Satd. Flow (RTOR) | | 34 | | | | | | | | | | 15 |
| Link Speed (k/h) | | 50 | | | 50 | | | 50 | | | | 50 |
| Link Distance (m) | | 157.8 | | | 106.5 | | | 75.7 | | | | 242.7 |
| Travel Time (s) | | 11.4 | | | 7.7 | | | 5.5 | | | | 17.5 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles (%) | 0% | 36% | 10% | 4% | 13% | 0% | 0% | 0% | 0% | 16% | 10% | 13% |
| Adj. Flow (vph) | 0 | 26 | 34 | 29 | 35 | 0 | 0 | 0 | 0 | 23 | 226 | 17 |
| Shared Lane Traffic (%) | | | | | | | | | | | | |
| Lane Group Flow (vph) | 0 | 60 | 0 | 0 | 64 | 0 | 0 | 0 | 0 | 0 | 266 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(m) | | 0.0 | | | 0.0 | | | 0.0 | | | | 0.0 |
| Link Offset(m) | | 0.0 | | | 0.0 | | | 0.0 | | | | 0.0 |
| Crosswalk Width(m) | | 1.6 | | | 1.6 | | | 1.6 | | | | 1.6 |
| Two way Left Turn Lane | | | | | | | | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | 24 | | 14 | 24 | | 14 | 24 | | 14 | 24 | | 14 |
| Turn Type | | NA | | Perm | NA | | | | | | Perm | NA |
| Protected Phases | | 4 | | | 8 | | | | | | | 2 |
| Permitted Phases | | | | 8 | | | | | | 2 | | |
| Minimum Split (s) | | 32.5 | | 32.5 | 32.5 | | | | | 23.4 | 23.4 | |
| Total Split (s) | | 46.0 | | 46.0 | 46.0 | | | | | 44.0 | 44.0 | |
| Total Split (%) | | 51.1% | | 51.1% | 51.1% | | | | | 48.9% | 48.9% | |
| Maximum Green (s) | | 40.5 | | 40.5 | 40.5 | | | | | 38.6 | 38.6 | |
| Yellow Time (s) | | 3.3 | | 3.3 | 3.3 | | | | | 3.3 | 3.3 | |
| All-Red Time (s) | | 2.2 | | 2.2 | 2.2 | | | | | 2.1 | 2.1 | |
| Lost Time Adjust (s) | | 0.0 | | | 0.0 | | | | | | | 0.0 |
| Total Lost Time (s) | | 5.5 | | | 5.5 | | | | | | | 5.4 |
| Lead/Lag | | | | | | | | | | | | |
| Lead-Lag Optimize? | | | | | | | | | | | | |
| Walk Time (s) | | 17.0 | | 17.0 | 17.0 | | | | | 7.0 | 7.0 | |
| Flash Dont Walk (s) | | 10.0 | | 10.0 | 10.0 | | | | | 11.0 | 11.0 | |
| Pedestrian Calls (#/hr) | | 0 | | 0 | 0 | | | | | 0 | 0 | |
| Act Effct Green (s) | | 40.5 | | | 40.5 | | | | | | | 38.6 |
| Actuated g/C Ratio | | 0.45 | | | 0.45 | | | | | | | 0.43 |

Lanes, Volumes, Timings
15: Brant Street & Birch Avenue

AM Peak Period
05-16-2019

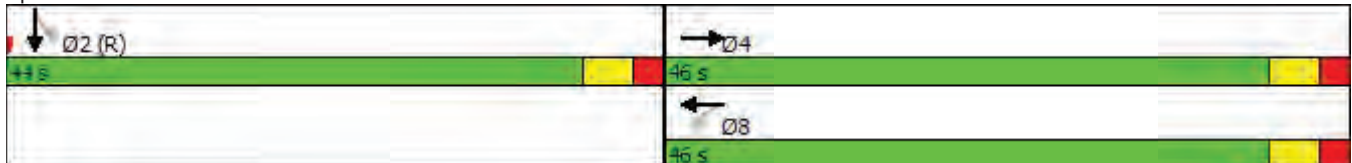


| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|----------------|-----|------|-----|-----|------|-----|-----|-----|-----|-----|-----|------|
| v/c Ratio | | 0.09 | | | 0.10 | | | | | | | 0.14 |
| Control Delay | | 8.2 | | | 14.8 | | | | | | | 7.6 |
| Queue Delay | | 0.0 | | | 0.0 | | | | | | | 0.0 |
| Total Delay | | 8.2 | | | 14.8 | | | | | | | 7.6 |
| LOS | | A | | | B | | | | | | | A |
| Approach Delay | | 8.2 | | | 14.8 | | | | | | | 7.6 |
| Approach LOS | | A | | | B | | | | | | | A |

Intersection Summary

| | |
|-----------------------------------|---|
| Area Type: | Other |
| Cycle Length: | 90 |
| Actuated Cycle Length: | 90 |
| Offset: | 0 (0%), Referenced to phase 2:SBTL and 6:, Start of Green |
| Natural Cycle: | 60 |
| Control Type: | Pretimed |
| Maximum v/c Ratio: | 0.14 |
| Intersection Signal Delay: | 8.9 |
| Intersection LOS: | A |
| Intersection Capacity Utilization | 27.3% |
| ICU Level of Service | A |
| Analysis Period (min) | 15 |

Splits and Phases: 15: Brant Street & Birch Avenue



Lanes, Volumes, Timings
25: Hillyard Street & Brant Street

AM Peak Period
05-16-2019



| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|----------------------------|------|-------|-------|------|-------|-------|------|-------|-------|------|-------|-------|
| Lane Configurations | | ↕ | | | ↕ | | | ↕ | | | ↕ | |
| Traffic Volume (vph) | 6 | 25 | 3 | 11 | 13 | 4 | 2 | 11 | 6 | 1 | 7 | 8 |
| Future Volume (vph) | 6 | 25 | 3 | 11 | 13 | 4 | 2 | 11 | 6 | 1 | 7 | 8 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt | | 0.989 | | | 0.982 | | | 0.955 | | | 0.932 | |
| Flt Protected | | 0.991 | | | 0.980 | | | 0.995 | | | 0.997 | |
| Satd. Flow (prot) | 0 | 1749 | 0 | 0 | 1591 | 0 | 0 | 1651 | 0 | 0 | 1514 | 0 |
| Flt Permitted | | 0.991 | | | 0.980 | | | 0.995 | | | 0.997 | |
| Satd. Flow (perm) | 0 | 1749 | 0 | 0 | 1591 | 0 | 0 | 1651 | 0 | 0 | 1514 | 0 |
| Link Speed (k/h) | | 48 | | | 48 | | | 48 | | | 48 | |
| Link Distance (m) | | 103.4 | | | 96.5 | | | 111.2 | | | 105.1 | |
| Travel Time (s) | | 7.8 | | | 7.2 | | | 8.3 | | | 7.9 | |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles (%) | 0% | 4% | 0% | 10% | 8% | 25% | 0% | 10% | 0% | 0% | 14% | 13% |
| Adj. Flow (vph) | 7 | 27 | 3 | 12 | 14 | 4 | 2 | 12 | 7 | 1 | 8 | 9 |
| Shared Lane Traffic (%) | | | | | | | | | | | | |
| Lane Group Flow (vph) | 0 | 37 | 0 | 0 | 30 | 0 | 0 | 21 | 0 | 0 | 18 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(m) | | 0.0 | | | 0.0 | | | 0.0 | | | 0.0 | |
| Link Offset(m) | | 0.0 | | | 0.0 | | | 0.0 | | | 0.0 | |
| Crosswalk Width(m) | | 1.6 | | | 1.6 | | | 1.6 | | | 1.6 | |
| Two way Left Turn Lane | | | | | | | | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | 24 | | 14 | 24 | | 14 | 24 | | 14 | 24 | | 14 |
| Sign Control | | Free | | | Free | | | Stop | | | Stop | |

Intersection Summary

| | |
|-----------------------------------|--------------|
| Area Type: | Other |
| Control Type: | Unsignalized |
| Intersection Capacity Utilization | 13.3% |
| ICU Level of Service | A |
| Analysis Period (min) | 15 |

Lanes, Volumes, Timings
27: Birch Avenue & Site Access #1

AM Peak Period
05-16-2019



| Lane Group | EBL | EBR | NBL | NBT | SBT | SBR |
|----------------------------|------|-------|------|-------|------|-------|
| Lane Configurations | | | | | | |
| Traffic Volume (vph) | 0 | 0 | 0 | 0 | 267 | 0 |
| Future Volume (vph) | 0 | 0 | 0 | 0 | 267 | 0 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 0.91 | 0.91 |
| Frt | | | | | | |
| Flt Protected | | | | | | |
| Satd. Flow (prot) | 1837 | 918 | 0 | 0 | 4558 | 0 |
| Flt Permitted | | | | | | |
| Satd. Flow (perm) | 1837 | 918 | 0 | 0 | 4558 | 0 |
| Link Speed (k/h) | 48 | | | 50 | 48 | |
| Link Distance (m) | 51.6 | | | 315.9 | 75.7 | |
| Travel Time (s) | 3.9 | | | 22.7 | 5.7 | |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles (%) | 0% | 100% | 0% | 0% | 10% | 100% |
| Adj. Flow (vph) | 0 | 0 | 0 | 0 | 290 | 0 |
| Shared Lane Traffic (%) | | | | | | |
| Lane Group Flow (vph) | 0 | 0 | 0 | 0 | 290 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No |
| Lane Alignment | Left | Right | Left | Left | Left | Right |
| Median Width(m) | 3.3 | | | 0.0 | 0.0 | |
| Link Offset(m) | 0.0 | | | 0.0 | 0.0 | |
| Crosswalk Width(m) | 1.6 | | | 1.6 | 1.6 | |
| Two way Left Turn Lane | | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | 24 | 14 | 24 | | | 14 |
| Sign Control | Stop | | | Free | Free | |

Intersection Summary

| | |
|-----------------------------------|------------------------|
| Area Type: | Other |
| Control Type: | Unsignalized |
| Intersection Capacity Utilization | 8.5% |
| Analysis Period (min) | 15 |
| | ICU Level of Service A |

Lanes, Volumes, Timings
31: Wentworth Street N & Munroe Street

AM Peak Period
05-16-2019












| Lane Group | WBL | WBR | NBT | NBR | SBL | SBT |
|----------------------------|-------|-------|-------|-------|------|-------|
| Lane Configurations | | | | | | |
| Traffic Volume (vph) | 25 | 8 | 158 | 32 | 2 | 308 |
| Future Volume (vph) | 25 | 8 | 158 | 32 | 2 | 308 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Util. Factor | 1.00 | 1.00 | 0.95 | 0.95 | 0.95 | 0.95 |
| Frt | 0.966 | | 0.975 | | | |
| Flt Protected | 0.964 | | | | | |
| Satd. Flow (prot) | 1462 | 0 | 3093 | 0 | 0 | 3194 |
| Flt Permitted | 0.964 | | | | | |
| Satd. Flow (perm) | 1462 | 0 | 3093 | 0 | 0 | 3194 |
| Link Speed (k/h) | 48 | | 50 | | | 50 |
| Link Distance (m) | 118.1 | | 172.4 | | | 236.6 |
| Travel Time (s) | 8.9 | | 12.4 | | | 17.0 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles (%) | 13% | 29% | 10% | 10% | 50% | 9% |
| Adj. Flow (vph) | 27 | 9 | 172 | 35 | 2 | 335 |
| Shared Lane Traffic (%) | | | | | | |
| Lane Group Flow (vph) | 36 | 0 | 207 | 0 | 0 | 337 |
| Enter Blocked Intersection | No | No | No | No | No | No |
| Lane Alignment | Left | Right | Left | Right | Left | Left |
| Median Width(m) | 3.3 | | 0.0 | | | 0.0 |
| Link Offset(m) | 0.0 | | 0.0 | | | 0.0 |
| Crosswalk Width(m) | 1.6 | | 1.6 | | | 1.6 |
| Two way Left Turn Lane | | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | 24 | 14 | | 14 | 24 | |
| Sign Control | Stop | | Free | | | Free |

Intersection Summary

| | |
|-----------------------------------|------------------------|
| Area Type: | Other |
| Control Type: | Unsignalized |
| Intersection Capacity Utilization | 19.9% |
| Analysis Period (min) | 15 |
| | ICU Level of Service A |

Lanes, Volumes, Timings
34: Hillyard Street & Site Access #3

AM Peak Period
05-16-2019

| |  |  |  |  |  |  |
|-----------------------------------|---|---|---|---|---|---|
| Lane Group | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations |  | |  | | |  |
| Traffic Volume (vph) | 0 | 0 | 32 | 0 | 0 | 33 |
| Future Volume (vph) | 0 | 0 | 32 | 0 | 0 | 33 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt | | | | | | |
| Flt Protected | | | | | | |
| Satd. Flow (prot) | 1837 | 0 | 1670 | 0 | 0 | 1670 |
| Flt Permitted | | | | | | |
| Satd. Flow (perm) | 1837 | 0 | 1670 | 0 | 0 | 1670 |
| Link Speed (k/h) | 48 | | 48 | | | 48 |
| Link Distance (m) | 48.3 | | 26.1 | | | 111.2 |
| Travel Time (s) | 3.6 | | 2.0 | | | 8.3 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles (%) | 0% | 0% | 10% | 0% | 0% | 10% |
| Adj. Flow (vph) | 0 | 0 | 35 | 0 | 0 | 36 |
| Shared Lane Traffic (%) | | | | | | |
| Lane Group Flow (vph) | 0 | 0 | 35 | 0 | 0 | 36 |
| Enter Blocked Intersection | No | No | No | No | No | No |
| Lane Alignment | Left | Right | Left | Right | Left | Left |
| Median Width(m) | 3.3 | | 0.0 | | | 0.0 |
| Link Offset(m) | 0.0 | | 0.0 | | | 0.0 |
| Crosswalk Width(m) | 1.6 | | 1.6 | | | 1.6 |
| Two way Left Turn Lane | | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | 24 | 14 | | 14 | 24 | |
| Sign Control | Stop | | Free | | | Free |
| Intersection Summary | | | | | | |
| Area Type: | Other | | | | | |
| Control Type: | Unsignalized | | | | | |
| Intersection Capacity Utilization | 13.3% | | | ICU Level of Service A | | |
| Analysis Period (min) | 15 | | | | | |

Lanes, Volumes, Timings
3: Wentworth Street N & Burlington Street E

PM Peak Period
05-16-2019



| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|----------------------------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|
| Lane Configurations | | | | | | | | | | | | |
| Traffic Volume (vph) | 10 | 636 | 33 | 157 | 648 | 10 | 10 | 7 | 106 | 10 | 12 | 8 |
| Future Volume (vph) | 10 | 636 | 33 | 157 | 648 | 10 | 10 | 7 | 106 | 10 | 12 | 8 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (m) | 60.0 | | 0.0 | 50.0 | | 0.0 | 0.0 | | 0.0 | 0.0 | | 0.0 |
| Storage Lanes | 1 | | 0 | 1 | | 0 | 0 | | 0 | 0 | | 0 |
| Taper Length (m) | 15.0 | | | 15.0 | | | 15.0 | | | 15.0 | | |
| Lane Util. Factor | 1.00 | 0.95 | 0.95 | 1.00 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 1.00 | 1.00 | 1.00 |
| Frt | | 0.993 | | | 0.998 | | | 0.871 | | | 0.963 | |
| Flt Protected | 0.950 | | | 0.950 | | | | 0.996 | | | 0.984 | |
| Satd. Flow (prot) | 1572 | 3078 | 0 | 1572 | 3161 | 0 | 0 | 2662 | 0 | 0 | 1420 | 0 |
| Flt Permitted | 0.380 | | | 0.232 | | | | 0.939 | | | 0.913 | |
| Satd. Flow (perm) | 629 | 3078 | 0 | 384 | 3161 | 0 | 0 | 2510 | 0 | 0 | 1317 | 0 |
| Right Turn on Red | | | Yes | | | Yes | | | Yes | | | Yes |
| Satd. Flow (RTOR) | | 6 | | | 3 | | | 115 | | | 9 | |
| Link Speed (k/h) | | 50 | | | 50 | | | 50 | | | 50 | |
| Link Distance (m) | | 125.0 | | | 716.7 | | | 215.5 | | | 91.4 | |
| Travel Time (s) | | 9.0 | | | 51.6 | | | 15.5 | | | 6.6 | |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles (%) | 11% | 12% | 24% | 11% | 10% | 22% | 0% | 0% | 16% | 22% | 11% | 40% |
| Adj. Flow (vph) | 11 | 691 | 36 | 171 | 704 | 11 | 11 | 8 | 115 | 11 | 13 | 9 |
| Shared Lane Traffic (%) | | | | | | | | | | | | |
| Lane Group Flow (vph) | 11 | 727 | 0 | 171 | 715 | 0 | 0 | 134 | 0 | 0 | 33 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(m) | | 3.3 | | | 3.3 | | | 0.0 | | | 0.0 | |
| Link Offset(m) | | 0.0 | | | 0.0 | | | 0.0 | | | 0.0 | |
| Crosswalk Width(m) | | 4.9 | | | 4.9 | | | 4.9 | | | 4.9 | |
| Two way Left Turn Lane | | | | | Yes | | | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | 24 | | 14 | 24 | | 14 | 24 | | 14 | 24 | | 14 |
| Turn Type | Perm | NA | | pm+pt | NA | | Perm | NA | | Perm | NA | |
| Protected Phases | | 2 | | 1 | 6 | | | 4 | | | 8 | |
| Permitted Phases | 2 | | | 6 | | | 4 | | | 8 | | |
| Minimum Split (s) | 29.0 | 29.0 | | 9.5 | 37.2 | | 35.0 | 35.0 | | 35.0 | 35.0 | |
| Total Split (s) | 38.0 | 38.0 | | 17.0 | 55.0 | | 35.0 | 35.0 | | 35.0 | 35.0 | |
| Total Split (%) | 42.2% | 42.2% | | 18.9% | 61.1% | | 38.9% | 38.9% | | 38.9% | 38.9% | |
| Maximum Green (s) | 32.0 | 32.0 | | 13.0 | 48.8 | | 29.0 | 29.0 | | 29.0 | 29.0 | |
| Yellow Time (s) | 3.3 | 3.3 | | 3.0 | 3.3 | | 3.3 | 3.3 | | 3.3 | 3.3 | |
| All-Red Time (s) | 2.7 | 2.7 | | 1.0 | 2.9 | | 2.7 | 2.7 | | 2.7 | 2.7 | |
| Lost Time Adjust (s) | 0.0 | 0.0 | | 0.0 | 0.0 | | 0.0 | 0.0 | | 0.0 | 0.0 | |
| Total Lost Time (s) | 6.0 | 6.0 | | 4.0 | 6.2 | | 6.0 | 6.0 | | 6.0 | 6.0 | |
| Lead/Lag | Lag | Lag | | Lead | | | | | | | | |
| Lead-Lag Optimize? | Yes | Yes | | Yes | | | | | | | | |
| Walk Time (s) | 7.0 | 7.0 | | | 15.0 | | 11.0 | 11.0 | | 11.0 | 11.0 | |
| Flash Dont Walk (s) | 16.0 | 16.0 | | | 16.0 | | 18.0 | 18.0 | | 18.0 | 18.0 | |
| Pedestrian Calls (#/hr) | 0 | 0 | | | 0 | | 0 | 0 | | 0 | 0 | |
| Act Effct Green (s) | 32.0 | 32.0 | | 51.0 | 48.8 | | 29.0 | 29.0 | | 29.0 | 29.0 | |
| Actuated g/C Ratio | 0.36 | 0.36 | | 0.57 | 0.54 | | 0.32 | 0.32 | | 0.32 | 0.32 | |

Lanes, Volumes, Timings
3: Wentworth Street N & Burlington Street E

PM Peak Period
05-16-2019

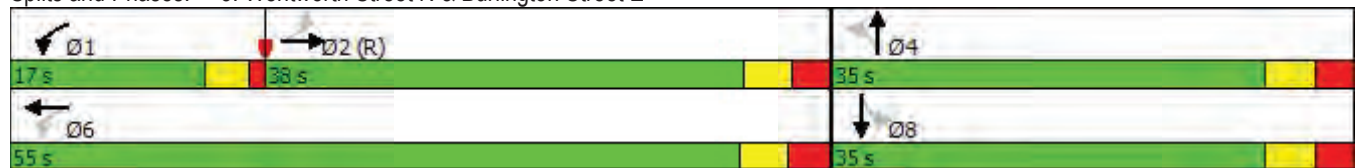


| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|----------------|------|------|-----|------|------|-----|-----|------|-----|-----|-----|------|
| v/c Ratio | 0.05 | 0.66 | | 0.44 | 0.42 | | | 0.15 | | | | 0.08 |
| Control Delay | 20.0 | 27.8 | | 13.2 | 13.0 | | | 6.4 | | | | 17.5 |
| Queue Delay | 0.0 | 0.0 | | 0.0 | 0.0 | | | 0.0 | | | | 0.0 |
| Total Delay | 20.0 | 27.8 | | 13.2 | 13.0 | | | 6.4 | | | | 17.5 |
| LOS | B | C | | B | B | | | A | | | | B |
| Approach Delay | | 27.7 | | | 13.1 | | | 6.4 | | | | 17.5 |
| Approach LOS | | C | | | B | | | A | | | | B |

Intersection Summary

| | |
|-----------------------------------|--|
| Area Type: | Other |
| Cycle Length: | 90 |
| Actuated Cycle Length: | 90 |
| Offset: | 0 (0%), Referenced to phase 2:EBTL, Start of Green |
| Natural Cycle: | 75 |
| Control Type: | Pretimed |
| Maximum v/c Ratio: | 0.66 |
| Intersection Signal Delay: | 18.7 |
| Intersection LOS: | B |
| Intersection Capacity Utilization | 58.4% |
| ICU Level of Service | B |
| Analysis Period (min) | 15 |

Splits and Phases: 3: Wentworth Street N & Burlington Street E



Lanes, Volumes, Timings
9: Birch Avenue & Burlington Street E

PM Peak Period
05-16-2019



| Lane Group | EBT | EBR | WBL | WBT | NBL | NBR |
|----------------------------|-------|-------|-------|-------|-------|-------|
| Lane Configurations | ↑↑↑ | | ↵↵ | ↑↑↑ | | |
| Traffic Volume (vph) | 1121 | 28 | 251 | 1226 | 0 | 0 |
| Future Volume (vph) | 1121 | 28 | 251 | 1226 | 0 | 0 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (m) | | 0.0 | 125.0 | | 0.0 | 0.0 |
| Storage Lanes | | 0 | 2 | | 0 | 0 |
| Taper Length (m) | | | 15.0 | | 15.0 | |
| Lane Util. Factor | 0.91 | 0.91 | 0.97 | 0.91 | 1.00 | 1.00 |
| Frt | 0.996 | | | | | |
| Flt Protected | | | 0.950 | | | |
| Satd. Flow (prot) | 4718 | 0 | 3134 | 4643 | 0 | 0 |
| Flt Permitted | | | 0.950 | | | |
| Satd. Flow (perm) | 4718 | 0 | 3134 | 4643 | 0 | 0 |
| Right Turn on Red | | Yes | | | | Yes |
| Satd. Flow (RTOR) | 8 | | | | | |
| Link Speed (k/h) | 50 | | | 50 | 50 | |
| Link Distance (m) | 716.7 | | | 130.8 | 242.7 | |
| Travel Time (s) | 51.6 | | | 9.4 | 17.5 | |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles (%) | 6% | 0% | 8% | 8% | 0% | 0% |
| Adj. Flow (vph) | 1218 | 30 | 273 | 1333 | 0 | 0 |
| Shared Lane Traffic (%) | | | | | | |
| Lane Group Flow (vph) | 1248 | 0 | 273 | 1333 | 0 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No |
| Lane Alignment | Left | Right | Left | Left | Left | Right |
| Median Width(m) | 6.6 | | | 6.6 | 0.0 | |
| Link Offset(m) | 0.0 | | | 0.0 | 0.0 | |
| Crosswalk Width(m) | 1.6 | | | 1.6 | 1.6 | |
| Two way Left Turn Lane | Yes | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | | 14 | 24 | | 24 | 14 |
| Turn Type | NA | | Prot | NA | | |
| Protected Phases | 2 | | 4 | 2 4 | | |
| Permitted Phases | | | | | | |
| Minimum Split (s) | 47.0 | | 15.1 | | | |
| Total Split (s) | 64.0 | | 26.0 | | | |
| Total Split (%) | 71.1% | | 28.9% | | | |
| Maximum Green (s) | 59.0 | | 20.9 | | | |
| Yellow Time (s) | 3.7 | | 3.7 | | | |
| All-Red Time (s) | 1.3 | | 1.4 | | | |
| Lost Time Adjust (s) | 0.0 | | 0.0 | | | |
| Total Lost Time (s) | 5.0 | | 5.1 | | | |
| Lead/Lag | | | | | | |
| Lead-Lag Optimize? | | | | | | |
| Walk Time (s) | 30.0 | | | | | |
| Flash Dont Walk (s) | 12.0 | | | | | |
| Pedestrian Calls (#/hr) | 0 | | | | | |
| Act Effct Green (s) | 59.0 | | 20.9 | 90.0 | | |
| Actuated g/C Ratio | 0.66 | | 0.23 | 1.00 | | |

Lanes, Volumes, Timings
 9: Birch Avenue & Burlington Street E

PM Peak Period
 05-16-2019



| Lane Group | EBT | EBR | WBL | WBT | NBL | NBR |
|----------------|------|-----|------|------|-----|-----|
| v/c Ratio | 0.40 | | 0.38 | 0.29 | | |
| Control Delay | 14.3 | | 30.9 | 0.2 | | |
| Queue Delay | 0.0 | | 0.0 | 0.0 | | |
| Total Delay | 14.3 | | 30.9 | 0.2 | | |
| LOS | B | | C | A | | |
| Approach Delay | 14.3 | | | 5.4 | | |
| Approach LOS | B | | | A | | |

Intersection Summary

| | |
|-----------------------------------|---|
| Area Type: | Other |
| Cycle Length: | 90 |
| Actuated Cycle Length: | 90 |
| Offset: | 0 (0%), Referenced to phase 2:EBWB and 6:, Start of Green |
| Natural Cycle: | 65 |
| Control Type: | Pretimed |
| Maximum v/c Ratio: | 0.40 |
| Intersection Signal Delay: | 9.3 |
| Intersection LOS: | A |
| Intersection Capacity Utilization | 39.0% |
| ICU Level of Service | A |
| Analysis Period (min) | 15 |

Splits and Phases: 9: Birch Avenue & Burlington Street E



Lanes, Volumes, Timings
13: Brant Street & Wentworth Street N

PM Peak Period
05-16-2019



| Lane Group | WBL | WBR | NBT | NBR | SBL | SBT |
|----------------------------|-------|-------|-------|-------|------|-------|
| Lane Configurations | | | | | | |
| Traffic Volume (vph) | 30 | 3 | 134 | 25 | 10 | 236 |
| Future Volume (vph) | 30 | 3 | 134 | 25 | 10 | 236 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Util. Factor | 1.00 | 1.00 | 0.95 | 0.95 | 0.95 | 0.95 |
| Frt | 0.989 | | 0.977 | | | |
| Flt Protected | 0.956 | | | | | 0.998 |
| Satd. Flow (prot) | 1632 | 0 | 3196 | 0 | 0 | 3236 |
| Flt Permitted | 0.956 | | | | | 0.998 |
| Satd. Flow (perm) | 1632 | 0 | 3196 | 0 | 0 | 3236 |
| Link Speed (k/h) | 48 | | 48 | | | 48 |
| Link Distance (m) | 104.2 | | 236.6 | | | 215.5 |
| Travel Time (s) | 7.8 | | 17.7 | | | 16.2 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles (%) | 7% | 0% | 7% | 5% | 22% | 7% |
| Adj. Flow (vph) | 33 | 3 | 146 | 27 | 11 | 257 |
| Shared Lane Traffic (%) | | | | | | |
| Lane Group Flow (vph) | 36 | 0 | 173 | 0 | 0 | 268 |
| Enter Blocked Intersection | No | No | No | No | No | No |
| Lane Alignment | Left | Right | Left | Right | Left | Left |
| Median Width(m) | 3.3 | | 0.0 | | | 0.0 |
| Link Offset(m) | 0.0 | | 0.0 | | | 0.0 |
| Crosswalk Width(m) | 1.6 | | 1.6 | | | 1.6 |
| Two way Left Turn Lane | | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | 24 | 14 | | 14 | 24 | |
| Sign Control | Stop | | Free | | | Free |

Intersection Summary

| | |
|-----------------------------------|------------------------|
| Area Type: | Other |
| Control Type: | Unsignalized |
| Intersection Capacity Utilization | 23.9% |
| Analysis Period (min) | 15 |
| | ICU Level of Service A |

Lanes, Volumes, Timings
 14: Site Access #2 & Brant Street

PM Peak Period
 05-16-2019



| Lane Group | EBT | EBR | WBL | WBT | NBL | NBR |
|----------------------------|------|-------|------|-------|------|-------|
| Lane Configurations | ↔ | | | ↔ | ↔ | ↔ |
| Traffic Volume (vph) | 54 | 0 | 0 | 41 | 0 | 0 |
| Future Volume (vph) | 54 | 0 | 0 | 41 | 0 | 0 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt | | | | | | |
| Flt Protected | | | | | | |
| Satd. Flow (prot) | 1655 | 0 | 0 | 1597 | 918 | 918 |
| Flt Permitted | | | | | | |
| Satd. Flow (perm) | 1655 | 0 | 0 | 1597 | 918 | 918 |
| Link Speed (k/h) | 48 | | | 48 | 48 | |
| Link Distance (m) | 77.1 | | | 157.8 | 61.8 | |
| Travel Time (s) | 5.8 | | | 11.8 | 4.6 | |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles (%) | 11% | 100% | 100% | 15% | 100% | 100% |
| Adj. Flow (vph) | 59 | 0 | 0 | 45 | 0 | 0 |
| Shared Lane Traffic (%) | | | | | | |
| Lane Group Flow (vph) | 59 | 0 | 0 | 45 | 0 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No |
| Lane Alignment | Left | Right | Left | Left | Left | Right |
| Median Width(m) | 0.0 | | | 0.0 | 3.3 | |
| Link Offset(m) | 0.0 | | | 0.0 | 0.0 | |
| Crosswalk Width(m) | 1.6 | | | 1.6 | 1.6 | |
| Two way Left Turn Lane | | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | | 14 | 24 | | 24 | 14 |
| Sign Control | Free | | | Free | Stop | |

Intersection Summary

| | |
|-----------------------------------|------------------------|
| Area Type: | Other |
| Control Type: | Unsignalized |
| Intersection Capacity Utilization | 13.3% |
| Analysis Period (min) | 15 |
| | ICU Level of Service A |

Lanes, Volumes, Timings
15: Brant Street & Birch Avenue

PM Peak Period
05-16-2019



| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|----------------------------|------|-------|-------|-------|-------|-------|------|------|-------|-------|-------|-------|
| Lane Configurations | | | | | | | | | | | | |
| Traffic Volume (vph) | 0 | 13 | 41 | 55 | 30 | 0 | 0 | 0 | 0 | 8 | 216 | 11 |
| Future Volume (vph) | 0 | 13 | 41 | 55 | 30 | 0 | 0 | 0 | 0 | 8 | 216 | 11 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (m) | 0.0 | | 0.0 | 0.0 | | 0.0 | 0.0 | | 0.0 | 0.0 | | 150.0 |
| Storage Lanes | 0 | | 0 | 0 | | 0 | 0 | | 0 | 0 | | 0 |
| Taper Length (m) | 15.0 | | | 15.0 | | | 15.0 | | | 15.0 | | |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.91 | 0.91 | 0.91 |
| Frt | | 0.897 | | | | | | | | | | 0.993 |
| Flt Protected | | | | | 0.969 | | | | | | | 0.998 |
| Satd. Flow (prot) | 0 | 1558 | 0 | 0 | 1692 | 0 | 0 | 0 | 0 | 0 | 4745 | 0 |
| Flt Permitted | | | | | 0.817 | | | | | | | 0.998 |
| Satd. Flow (perm) | 0 | 1558 | 0 | 0 | 1426 | 0 | 0 | 0 | 0 | 0 | 4745 | 0 |
| Right Turn on Red | | | Yes | | | Yes | | | Yes | | | Yes |
| Satd. Flow (RTOR) | | 45 | | | | | | | | | | 10 |
| Link Speed (k/h) | | 50 | | | 50 | | | 50 | | | | 50 |
| Link Distance (m) | | 157.8 | | | 106.5 | | | 75.7 | | | | 242.7 |
| Travel Time (s) | | 11.4 | | | 7.7 | | | 5.5 | | | | 17.5 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles (%) | 0% | 8% | 5% | 2% | 11% | 2% | 0% | 0% | 0% | 29% | 3% | 20% |
| Adj. Flow (vph) | 0 | 14 | 45 | 60 | 33 | 0 | 0 | 0 | 0 | 9 | 235 | 12 |
| Shared Lane Traffic (%) | | | | | | | | | | | | |
| Lane Group Flow (vph) | 0 | 59 | 0 | 0 | 93 | 0 | 0 | 0 | 0 | 0 | 256 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(m) | | 0.0 | | | 0.0 | | | 0.0 | | | | 0.0 |
| Link Offset(m) | | 0.0 | | | 0.0 | | | 0.0 | | | | 0.0 |
| Crosswalk Width(m) | | 1.6 | | | 1.6 | | | 1.6 | | | | 1.6 |
| Two way Left Turn Lane | | | | | | | | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | 24 | | 14 | 24 | | 14 | 24 | | 14 | 24 | | 14 |
| Turn Type | | NA | | Perm | NA | | | | | Perm | | NA |
| Protected Phases | | 4 | | | 8 | | | | | | | 2 |
| Permitted Phases | | | | 8 | | | | | | 2 | | |
| Minimum Split (s) | | 32.5 | | 32.5 | 32.5 | | | | | 23.4 | 23.4 | |
| Total Split (s) | | 50.0 | | 50.0 | 50.0 | | | | | 40.0 | 40.0 | |
| Total Split (%) | | 55.6% | | 55.6% | 55.6% | | | | | 44.4% | 44.4% | |
| Maximum Green (s) | | 44.5 | | 44.5 | 44.5 | | | | | 34.6 | 34.6 | |
| Yellow Time (s) | | 3.3 | | 3.3 | 3.3 | | | | | 3.3 | 3.3 | |
| All-Red Time (s) | | 2.2 | | 2.2 | 2.2 | | | | | 2.1 | 2.1 | |
| Lost Time Adjust (s) | | 0.0 | | | 0.0 | | | | | | | 0.0 |
| Total Lost Time (s) | | 5.5 | | | 5.5 | | | | | | | 5.4 |
| Lead/Lag | | | | | | | | | | | | |
| Lead-Lag Optimize? | | | | | | | | | | | | |
| Walk Time (s) | | 17.0 | | 17.0 | 17.0 | | | | | 7.0 | 7.0 | |
| Flash Dont Walk (s) | | 10.0 | | 10.0 | 10.0 | | | | | 11.0 | 11.0 | |
| Pedestrian Calls (#/hr) | | 0 | | 0 | 0 | | | | | 0 | 0 | |
| Act Effct Green (s) | | 44.5 | | | 44.5 | | | | | | | 34.6 |
| Actuated g/C Ratio | | 0.49 | | | 0.49 | | | | | | | 0.38 |

Lanes, Volumes, Timings
15: Brant Street & Birch Avenue

PM Peak Period
05-16-2019

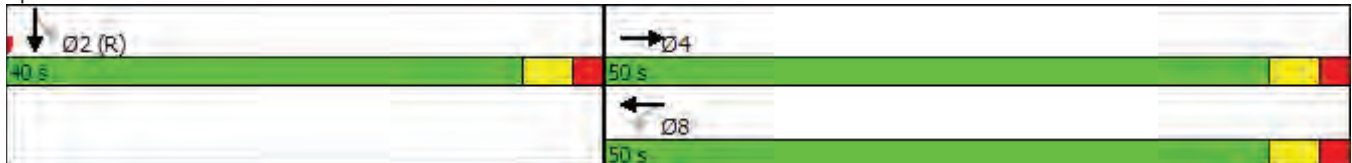


| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|----------------|-----|------|-----|-----|------|-----|-----|-----|-----|-----|-----|------|
| v/c Ratio | | 0.07 | | | 0.13 | | | | | | | 0.14 |
| Control Delay | | 5.4 | | | 13.0 | | | | | | | 6.2 |
| Queue Delay | | 0.0 | | | 0.0 | | | | | | | 0.0 |
| Total Delay | | 5.4 | | | 13.0 | | | | | | | 6.2 |
| LOS | | A | | | B | | | | | | | A |
| Approach Delay | | 5.4 | | | 13.0 | | | | | | | 6.2 |
| Approach LOS | | A | | | B | | | | | | | A |

Intersection Summary

| | |
|-----------------------------------|---|
| Area Type: | Other |
| Cycle Length: | 90 |
| Actuated Cycle Length: | 90 |
| Offset: | 0 (0%), Referenced to phase 2:SBTL and 6:, Start of Green |
| Natural Cycle: | 60 |
| Control Type: | Pretimed |
| Maximum v/c Ratio: | 0.14 |
| Intersection Signal Delay: | 7.6 |
| Intersection LOS: | A |
| Intersection Capacity Utilization | 28.7% |
| ICU Level of Service | A |
| Analysis Period (min) | 15 |

Splits and Phases: 15: Brant Street & Birch Avenue



Lanes, Volumes, Timings
25: Hillyard Street & Brant Street

PM Peak Period
05-16-2019



| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|----------------------------|------|-------|-------|------|-------|-------|------|-------|-------|------|-------|-------|
| Lane Configurations | | ↕ | | | ↕ | | | ↕ | | | ↕ | |
| Traffic Volume (vph) | 1 | 20 | 1 | 4 | 23 | 0 | 3 | 5 | 7 | 7 | 4 | 14 |
| Future Volume (vph) | 1 | 20 | 1 | 4 | 23 | 0 | 3 | 5 | 7 | 7 | 4 | 14 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt | | 0.994 | | | | | | 0.932 | | | 0.925 | |
| Flt Protected | | 0.998 | | | 0.993 | | | 0.991 | | | 0.985 | |
| Satd. Flow (prot) | 0 | 1655 | 0 | 0 | 1824 | 0 | 0 | 1585 | 0 | 0 | 1673 | 0 |
| Flt Permitted | | 0.998 | | | 0.993 | | | 0.991 | | | 0.985 | |
| Satd. Flow (perm) | 0 | 1655 | 0 | 0 | 1824 | 0 | 0 | 1585 | 0 | 0 | 1673 | 0 |
| Link Speed (k/h) | | 48 | | | 48 | | | 48 | | | 48 | |
| Link Distance (m) | | 103.4 | | | 96.5 | | | 111.2 | | | 105.1 | |
| Travel Time (s) | | 7.8 | | | 7.2 | | | 8.3 | | | 7.9 | |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles (%) | 0% | 11% | 0% | 0% | 0% | 0% | 0% | 0% | 14% | 0% | 0% | 0% |
| Adj. Flow (vph) | 1 | 22 | 1 | 4 | 25 | 0 | 3 | 5 | 8 | 8 | 4 | 15 |
| Shared Lane Traffic (%) | | | | | | | | | | | | |
| Lane Group Flow (vph) | 0 | 24 | 0 | 0 | 29 | 0 | 0 | 16 | 0 | 0 | 27 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(m) | | 0.0 | | | 0.0 | | | 0.0 | | | 0.0 | |
| Link Offset(m) | | 0.0 | | | 0.0 | | | 0.0 | | | 0.0 | |
| Crosswalk Width(m) | | 1.6 | | | 1.6 | | | 1.6 | | | 1.6 | |
| Two way Left Turn Lane | | | | | | | | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | 24 | | 14 | 24 | | 14 | 24 | | 14 | 24 | | 14 |
| Sign Control | | Free | | | Free | | | Stop | | | Stop | |

Intersection Summary

| | |
|-----------------------------------|--------------|
| Area Type: | Other |
| Control Type: | Unsignalized |
| Intersection Capacity Utilization | 13.3% |
| ICU Level of Service | A |
| Analysis Period (min) | 15 |

Lanes, Volumes, Timings
27: Birch Avenue & Site Access #1

PM Peak Period
05-16-2019



| Lane Group | EBL | EBR | NBL | NBT | SBT | SBR |
|----------------------------|------|-------|------|-------|------|-------|
| Lane Configurations | | | | | | |
| Traffic Volume (vph) | 0 | 0 | 0 | 0 | 313 | 0 |
| Future Volume (vph) | 0 | 0 | 0 | 0 | 313 | 0 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 0.91 | 0.91 |
| Frt | | | | | | |
| Flt Protected | | | | | | |
| Satd. Flow (prot) | 1837 | 918 | 0 | 0 | 4868 | 0 |
| Flt Permitted | | | | | | |
| Satd. Flow (perm) | 1837 | 918 | 0 | 0 | 4868 | 0 |
| Link Speed (k/h) | 48 | | | 50 | 48 | |
| Link Distance (m) | 49.2 | | | 315.9 | 75.7 | |
| Travel Time (s) | 3.7 | | | 22.7 | 5.7 | |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles (%) | 0% | 100% | 0% | 0% | 3% | 100% |
| Adj. Flow (vph) | 0 | 0 | 0 | 0 | 340 | 0 |
| Shared Lane Traffic (%) | | | | | | |
| Lane Group Flow (vph) | 0 | 0 | 0 | 0 | 340 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No |
| Lane Alignment | Left | Right | Left | Left | Left | Right |
| Median Width(m) | 3.3 | | | 0.0 | 0.0 | |
| Link Offset(m) | 0.0 | | | 0.0 | 0.0 | |
| Crosswalk Width(m) | 1.6 | | | 1.6 | 1.6 | |
| Two way Left Turn Lane | | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | 24 | 14 | 24 | | | 14 |
| Sign Control | Stop | | | Free | Free | |

Intersection Summary

| | |
|-----------------------------------|------------------------|
| Area Type: | Other |
| Control Type: | Unsignalized |
| Intersection Capacity Utilization | 9.4% |
| | ICU Level of Service A |
| Analysis Period (min) | 15 |

Lanes, Volumes, Timings
31: Wentworth Street N & Munroe Street

PM Peak Period
05-16-2019



| Lane Group | WBL | WBR | NBT | NBR | SBL | SBT |
|----------------------------|-------|-------|-------|-------|------|-------|
| Lane Configurations | | | | | | |
| Traffic Volume (vph) | 42 | 1 | 185 | 32 | 1 | 289 |
| Future Volume (vph) | 42 | 1 | 185 | 32 | 1 | 289 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Util. Factor | 1.00 | 1.00 | 0.95 | 0.95 | 0.95 | 0.95 |
| Frt | 0.997 | | 0.978 | | | |
| Flt Protected | 0.953 | | | | | |
| Satd. Flow (prot) | 1664 | 0 | 3176 | 0 | 0 | 3232 |
| Flt Permitted | 0.953 | | | | | |
| Satd. Flow (perm) | 1664 | 0 | 3176 | 0 | 0 | 3232 |
| Link Speed (k/h) | 48 | | 48 | | | 48 |
| Link Distance (m) | 118.1 | | 172.4 | | | 236.6 |
| Travel Time (s) | 8.9 | | 12.9 | | | 17.7 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles (%) | 5% | 0% | 7% | 10% | 0% | 8% |
| Adj. Flow (vph) | 46 | 1 | 201 | 35 | 1 | 314 |
| Shared Lane Traffic (%) | | | | | | |
| Lane Group Flow (vph) | 47 | 0 | 236 | 0 | 0 | 315 |
| Enter Blocked Intersection | No | No | No | No | No | No |
| Lane Alignment | Left | Right | Left | Right | Left | Left |
| Median Width(m) | 3.3 | | 0.0 | | | 0.0 |
| Link Offset(m) | 0.0 | | 0.0 | | | 0.0 |
| Crosswalk Width(m) | 1.6 | | 1.6 | | | 1.6 |
| Two way Left Turn Lane | | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | 24 | 14 | | 14 | 24 | |
| Sign Control | Stop | | Free | | | Free |

Intersection Summary

| | |
|-----------------------------------|------------------------|
| Area Type: | Other |
| Control Type: | Unsignalized |
| Intersection Capacity Utilization | 18.7% |
| Analysis Period (min) | 15 |
| | ICU Level of Service A |

Lanes, Volumes, Timings
34: Hillyard Street & Site Access #3

PM Peak Period
05-16-2019



| Lane Group | WBL | WBR | NBT | NBR | SBL | SBT |
|----------------------------|------|-------|------|-------|------|-------|
| Lane Configurations | | | | | | |
| Traffic Volume (vph) | 0 | 0 | 32 | 0 | 0 | 44 |
| Future Volume (vph) | 0 | 0 | 32 | 0 | 0 | 44 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt | | | | | | |
| Flt Protected | | | | | | |
| Satd. Flow (prot) | 1837 | 0 | 1670 | 0 | 0 | 1670 |
| Flt Permitted | | | | | | |
| Satd. Flow (perm) | 1837 | 0 | 1670 | 0 | 0 | 1670 |
| Link Speed (k/h) | 48 | | 48 | | | 48 |
| Link Distance (m) | 48.3 | | 26.1 | | | 111.2 |
| Travel Time (s) | 3.6 | | 2.0 | | | 8.3 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles (%) | 0% | 0% | 10% | 0% | 0% | 10% |
| Adj. Flow (vph) | 0 | 0 | 35 | 0 | 0 | 48 |
| Shared Lane Traffic (%) | | | | | | |
| Lane Group Flow (vph) | 0 | 0 | 35 | 0 | 0 | 48 |
| Enter Blocked Intersection | No | No | No | No | No | No |
| Lane Alignment | Left | Right | Left | Right | Left | Left |
| Median Width(m) | 3.3 | | 0.0 | | | 0.0 |
| Link Offset(m) | 0.0 | | 0.0 | | | 0.0 |
| Crosswalk Width(m) | 1.6 | | 1.6 | | | 1.6 |
| Two way Left Turn Lane | | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | 24 | 14 | | 14 | 24 | |
| Sign Control | Stop | | Free | | | Free |

Intersection Summary

| | |
|-----------------------------------|------------------------|
| Area Type: | Other |
| Control Type: | Unsignalized |
| Intersection Capacity Utilization | 13.3% |
| Analysis Period (min) | 15 |
| | ICU Level of Service A |

APPENDIX D

Synchro Outputs – Future Total 2022 Conditions

Lanes, Volumes, Timings

AM Peak Period

3: Wentworth Street N & Burlington St E/Burlington Street E

06-07-2019



| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|----------------------------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|
| Lane Configurations | | | | | | | | | | | | |
| Traffic Volume (vph) | 2 | 527 | 29 | 216 | 973 | 9 | 8 | 10 | 114 | 8 | 3 | 10 |
| Future Volume (vph) | 2 | 527 | 29 | 216 | 973 | 9 | 8 | 10 | 114 | 8 | 3 | 10 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (m) | 60.0 | | 0.0 | 50.0 | | 0.0 | 0.0 | | 0.0 | 0.0 | | 0.0 |
| Storage Lanes | 1 | | 0 | 1 | | 0 | 0 | | 0 | 0 | | 0 |
| Taper Length (m) | 15.0 | | | 15.0 | | | 15.0 | | | 15.0 | | |
| Lane Util. Factor | 1.00 | 0.95 | 0.95 | 1.00 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 1.00 | 1.00 | 1.00 |
| Frt | | 0.992 | | | 0.999 | | | 0.871 | | | 0.935 | |
| Flt Protected | 0.950 | | | 0.950 | | | | 0.997 | | | 0.981 | |
| Satd. Flow (prot) | 1745 | 3180 | 0 | 1616 | 3082 | 0 | 0 | 2747 | 0 | 0 | 1400 | 0 |
| Flt Permitted | 0.268 | | | 0.287 | | | | 0.944 | | | 0.904 | |
| Satd. Flow (perm) | 492 | 3180 | 0 | 488 | 3082 | 0 | 0 | 2601 | 0 | 0 | 1290 | 0 |
| Right Turn on Red | | | Yes | | | Yes | | | Yes | | | Yes |
| Satd. Flow (RTOR) | | 7 | | | 2 | | | 124 | | | 11 | |
| Link Speed (k/h) | | 50 | | | 50 | | | 50 | | | 50 | |
| Link Distance (m) | | 125.0 | | | 716.7 | | | 215.5 | | | 91.4 | |
| Travel Time (s) | | 9.0 | | | 51.6 | | | 15.5 | | | 6.6 | |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles (%) | 0% | 8% | 24% | 8% | 13% | 25% | 0% | 0% | 12% | 14% | 33% | 22% |
| Adj. Flow (vph) | 2 | 573 | 32 | 235 | 1058 | 10 | 9 | 11 | 124 | 9 | 3 | 11 |
| Shared Lane Traffic (%) | | | | | | | | | | | | |
| Lane Group Flow (vph) | 2 | 605 | 0 | 235 | 1068 | 0 | 0 | 144 | 0 | 0 | 23 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(m) | | 3.3 | | | 3.3 | | | 0.0 | | | 0.0 | |
| Link Offset(m) | | 0.0 | | | 0.0 | | | 0.0 | | | 0.0 | |
| Crosswalk Width(m) | | 4.9 | | | 4.9 | | | 4.9 | | | 4.9 | |
| Two way Left Turn Lane | | | | | Yes | | | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | 24 | | 14 | 24 | | 14 | 24 | | 14 | 24 | | 14 |
| Turn Type | Perm | NA | | pm+pt | NA | | Perm | NA | | Perm | NA | |
| Protected Phases | | 2 | | 1 | 6 | | | 4 | | | 8 | |
| Permitted Phases | 2 | | | 6 | | | 4 | | | 8 | | |
| Minimum Split (s) | 29.0 | 29.0 | | 9.5 | 37.2 | | 35.0 | 35.0 | | 35.0 | 35.0 | |
| Total Split (s) | 36.0 | 36.0 | | 19.0 | 55.0 | | 35.0 | 35.0 | | 35.0 | 35.0 | |
| Total Split (%) | 40.0% | 40.0% | | 21.1% | 61.1% | | 38.9% | 38.9% | | 38.9% | 38.9% | |
| Maximum Green (s) | 30.0 | 30.0 | | 15.0 | 48.8 | | 29.0 | 29.0 | | 29.0 | 29.0 | |
| Yellow Time (s) | 3.3 | 3.3 | | 3.0 | 3.3 | | 3.3 | 3.3 | | 3.3 | 3.3 | |
| All-Red Time (s) | 2.7 | 2.7 | | 1.0 | 2.9 | | 2.7 | 2.7 | | 2.7 | 2.7 | |
| Lost Time Adjust (s) | 0.0 | 0.0 | | 0.0 | 0.0 | | 0.0 | 0.0 | | 0.0 | 0.0 | |
| Total Lost Time (s) | 6.0 | 6.0 | | 4.0 | 6.2 | | 6.0 | 6.0 | | 6.0 | 6.0 | |
| Lead/Lag | Lag | Lag | | Lead | | | | | | | | |
| Lead-Lag Optimize? | Yes | Yes | | Yes | | | | | | | | |
| Walk Time (s) | 7.0 | 7.0 | | | 15.0 | | 11.0 | 11.0 | | 11.0 | 11.0 | |
| Flash Dont Walk (s) | 16.0 | 16.0 | | | 16.0 | | 18.0 | 18.0 | | 18.0 | 18.0 | |
| Pedestrian Calls (#/hr) | 0 | 0 | | | 0 | | 0 | 0 | | 0 | 0 | |
| Act Effct Green (s) | 30.0 | 30.0 | | 51.0 | 48.8 | | 29.0 | 29.0 | | 29.0 | 29.0 | |
| Actuated g/C Ratio | 0.33 | 0.33 | | 0.57 | 0.54 | | 0.32 | 0.32 | | 0.32 | 0.32 | |

Lanes, Volumes, Timings
 3: Wentworth Street N & Burlington St E/Burlington Street E

AM Peak Period
 06-07-2019



| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|------------------------|------|-------|-----|------|-------|-----|-----|-------|-----|-----|-----|------|
| v/c Ratio | 0.01 | 0.57 | | 0.51 | 0.64 | | | 0.16 | | | | 0.05 |
| Control Delay | 20.5 | 26.9 | | 13.9 | 16.5 | | | 6.2 | | | | 15.0 |
| Queue Delay | 0.0 | 0.0 | | 0.0 | 0.0 | | | 0.0 | | | | 0.0 |
| Total Delay | 20.5 | 26.9 | | 13.9 | 16.5 | | | 6.2 | | | | 15.0 |
| LOS | C | C | | B | B | | | A | | | | B |
| Approach Delay | | 26.9 | | | 16.1 | | | 6.2 | | | | 15.0 |
| Approach LOS | | C | | | B | | | A | | | | B |
| Queue Length 50th (m) | 0.2 | 44.2 | | 19.2 | 63.9 | | | 1.2 | | | | 1.4 |
| Queue Length 95th (m) | 1.8 | 60.8 | | 31.7 | 84.0 | | | 7.5 | | | | 6.7 |
| Internal Link Dist (m) | | 101.0 | | | 692.7 | | | 191.5 | | | | 67.4 |
| Turn Bay Length (m) | 60.0 | | | 50.0 | | | | | | | | |
| Base Capacity (vph) | 164 | 1064 | | 464 | 1672 | | | 922 | | | | 423 |
| Starvation Cap Reductn | 0 | 0 | | 0 | 0 | | | 0 | | | | 0 |
| Spillback Cap Reductn | 0 | 0 | | 0 | 0 | | | 0 | | | | 0 |
| Storage Cap Reductn | 0 | 0 | | 0 | 0 | | | 0 | | | | 0 |
| Reduced v/c Ratio | 0.01 | 0.57 | | 0.51 | 0.64 | | | 0.16 | | | | 0.05 |

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 0 (0%), Referenced to phase 2:EBTL, Start of Green
 Natural Cycle: 75
 Control Type: Pretimed
 Maximum v/c Ratio: 0.64
 Intersection Signal Delay: 18.5
 Intersection Capacity Utilization 67.4%
 Analysis Period (min) 15
 Intersection LOS: B
 ICU Level of Service C

Splits and Phases: 3: Wentworth Street N & Burlington St E/Burlington Street E



Lanes, Volumes, Timings
9: Birch Avenue & Burlington Street E

AM Peak Period
06-07-2019



| Lane Group | EBT | EBR | WBL | WBT | NBL | NBR |
|----------------------------|-------|-------|-------|-------|-------|-------|
| Lane Configurations | ↑↑↑ | | ↔ | ↑↑↑ | | |
| Traffic Volume (vph) | 650 | 23 | 283 | 1477 | 0 | 0 |
| Future Volume (vph) | 650 | 23 | 283 | 1477 | 0 | 0 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (m) | | 0.0 | 125.0 | | 0.0 | 0.0 |
| Storage Lanes | | 0 | 2 | | 0 | 0 |
| Taper Length (m) | | | 15.0 | | 15.0 | |
| Lane Util. Factor | 0.91 | 0.91 | 0.97 | 0.91 | 1.00 | 1.00 |
| Frt | 0.995 | | | | | |
| Flt Protected | | | 0.950 | | | |
| Satd. Flow (prot) | 4457 | 0 | 3224 | 4821 | 0 | 0 |
| Flt Permitted | | | 0.950 | | | |
| Satd. Flow (perm) | 4457 | 0 | 3224 | 4821 | 0 | 0 |
| Right Turn on Red | | Yes | | | | Yes |
| Satd. Flow (RTOR) | 11 | | | | | |
| Link Speed (k/h) | 50 | | | 50 | 50 | |
| Link Distance (m) | 716.7 | | | 130.8 | 242.7 | |
| Travel Time (s) | 51.6 | | | 9.4 | 17.5 | |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles (%) | 12% | 10% | 5% | 4% | 0% | 0% |
| Adj. Flow (vph) | 707 | 25 | 308 | 1605 | 0 | 0 |
| Shared Lane Traffic (%) | | | | | | |
| Lane Group Flow (vph) | 732 | 0 | 308 | 1605 | 0 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No |
| Lane Alignment | Left | Right | Left | Left | Left | Right |
| Median Width(m) | 6.6 | | | 6.6 | 0.0 | |
| Link Offset(m) | 0.0 | | | 0.0 | 0.0 | |
| Crosswalk Width(m) | 1.6 | | | 1.6 | 1.6 | |
| Two way Left Turn Lane | Yes | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | | 14 | 24 | | 24 | 14 |
| Turn Type | NA | | Prot | NA | | |
| Protected Phases | 2 | | 4 | 2 4 | | |
| Permitted Phases | | | | | | |
| Minimum Split (s) | 47.0 | | 15.1 | | | |
| Total Split (s) | 62.0 | | 28.0 | | | |
| Total Split (%) | 68.9% | | 31.1% | | | |
| Maximum Green (s) | 57.0 | | 22.9 | | | |
| Yellow Time (s) | 3.7 | | 3.7 | | | |
| All-Red Time (s) | 1.3 | | 1.4 | | | |
| Lost Time Adjust (s) | 0.0 | | 0.0 | | | |
| Total Lost Time (s) | 5.0 | | 5.1 | | | |
| Lead/Lag | | | | | | |
| Lead-Lag Optimize? | | | | | | |
| Walk Time (s) | 30.0 | | | | | |
| Flash Dont Walk (s) | 12.0 | | | | | |
| Pedestrian Calls (#/hr) | 0 | | | | | |
| Act Effct Green (s) | 57.0 | | 22.9 | 90.0 | | |
| Actuated g/C Ratio | 0.63 | | 0.25 | 1.00 | | |

Lanes, Volumes, Timings
 9: Birch Avenue & Burlington Street E

AM Peak Period
 06-07-2019



| Lane Group | EBT | EBR | WBL | WBT | NBL | NBR |
|------------------------|-------|-----|-------|-------|-------|-----|
| v/c Ratio | 0.26 | | 0.38 | 0.33 | | |
| Control Delay | 17.6 | | 29.3 | 0.2 | | |
| Queue Delay | 0.0 | | 0.0 | 0.0 | | |
| Total Delay | 17.6 | | 29.3 | 0.2 | | |
| LOS | B | | C | A | | |
| Approach Delay | 17.6 | | | 4.9 | | |
| Approach LOS | B | | | A | | |
| Queue Length 50th (m) | 31.6 | | 22.6 | 0.0 | | |
| Queue Length 95th (m) | 45.4 | | 34.0 | 0.0 | | |
| Internal Link Dist (m) | 692.7 | | | 106.8 | 218.7 | |
| Turn Bay Length (m) | | | 125.0 | | | |
| Base Capacity (vph) | 2826 | | 820 | 4821 | | |
| Starvation Cap Reductn | 0 | | 0 | 0 | | |
| Spillback Cap Reductn | 0 | | 0 | 0 | | |
| Storage Cap Reductn | 0 | | 0 | 0 | | |
| Reduced v/c Ratio | 0.26 | | 0.38 | 0.33 | | |

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 0 (0%), Referenced to phase 2:EBWB and 6:, Start of Green
 Natural Cycle: 65
 Control Type: Pretimed
 Maximum v/c Ratio: 0.38
 Intersection Signal Delay: 8.4
 Intersection Capacity Utilization 32.7%
 Analysis Period (min) 15
 Intersection LOS: A
 ICU Level of Service A

Splits and Phases: 9: Birch Avenue & Burlington Street E



Lanes, Volumes, Timings
13: Wentworth Street N & Brant Street

AM Peak Period
06-07-2019



| Lane Group | WBL | WBR | NBT | NBR | SBL | SBT |
|----------------------------|-------|-------|-------|-------|------|-------|
| Lane Configurations | | | | | | |
| Traffic Volume (vph) | 28 | 9 | 120 | 63 | 10 | 367 |
| Future Volume (vph) | 28 | 9 | 120 | 63 | 10 | 367 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Util. Factor | 1.00 | 1.00 | 0.95 | 0.95 | 0.95 | 0.95 |
| Frt | 0.966 | | 0.948 | | | |
| Flt Protected | 0.964 | | | | | 0.999 |
| Satd. Flow (prot) | 1500 | 0 | 3074 | 0 | 0 | 3243 |
| Flt Permitted | 0.964 | | | | | 0.999 |
| Satd. Flow (perm) | 1500 | 0 | 3074 | 0 | 0 | 3243 |
| Link Speed (k/h) | 48 | | 50 | | | 50 |
| Link Distance (m) | 104.2 | | 236.6 | | | 215.5 |
| Travel Time (s) | 7.8 | | 17.0 | | | 15.5 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles (%) | 14% | 14% | 9% | 5% | 25% | 7% |
| Adj. Flow (vph) | 30 | 10 | 130 | 68 | 11 | 399 |
| Shared Lane Traffic (%) | | | | | | |
| Lane Group Flow (vph) | 40 | 0 | 198 | 0 | 0 | 410 |
| Enter Blocked Intersection | No | No | No | No | No | No |
| Lane Alignment | Left | Right | Left | Right | Left | Left |
| Median Width(m) | 3.3 | | 0.0 | | | 0.0 |
| Link Offset(m) | 0.0 | | 0.0 | | | 0.0 |
| Crosswalk Width(m) | 1.6 | | 1.6 | | | 1.6 |
| Two way Left Turn Lane | | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | 24 | 14 | | 14 | 24 | |
| Sign Control | Stop | | Free | | | Free |

Intersection Summary

| | |
|-----------------------------------|------------------------|
| Area Type: | Other |
| Control Type: | Unsignalized |
| Intersection Capacity Utilization | 27.3% |
| Analysis Period (min) | 15 |
| | ICU Level of Service A |

Lanes, Volumes, Timings
14: Site Access #2 & Brant Street

AM Peak Period
06-07-2019



| Lane Group | EBT | EBR | WBL | WBT | NBL | NBR |
|----------------------------|------|-------|------|-------|------|-------|
| Lane Configurations | | | | | | |
| Traffic Volume (vph) | 60 | 0 | 0 | 91 | 0 | 1 |
| Future Volume (vph) | 60 | 0 | 0 | 91 | 0 | 1 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Fr _t | | | | | | 0.850 |
| Fl _t Protected | | | | | | |
| Satd. Flow (prot) | 1766 | 0 | 0 | 1625 | 918 | 781 |
| Fl _t Permitted | | | | | | |
| Satd. Flow (perm) | 1766 | 0 | 0 | 1625 | 918 | 781 |
| Link Speed (k/h) | 48 | | | 48 | 48 | |
| Link Distance (m) | 77.1 | | | 157.8 | 61.8 | |
| Travel Time (s) | 5.8 | | | 11.8 | 4.6 | |
| Peak Hour Factor | 0.92 | 0.70 | 0.70 | 0.92 | 0.70 | 0.70 |
| Heavy Vehicles (%) | 4% | 100% | 100% | 13% | 100% | 100% |
| Adj. Flow (vph) | 65 | 0 | 0 | 99 | 0 | 1 |
| Shared Lane Traffic (%) | | | | | | |
| Lane Group Flow (vph) | 65 | 0 | 0 | 99 | 0 | 1 |
| Enter Blocked Intersection | No | No | No | No | No | No |
| Lane Alignment | Left | Right | Left | Left | Left | Right |
| Median Width(m) | 0.0 | | | 0.0 | 3.3 | |
| Link Offset(m) | 0.0 | | | 0.0 | 0.0 | |
| Crosswalk Width(m) | 1.6 | | | 1.6 | 1.6 | |
| Two way Left Turn Lane | | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | | 14 | 24 | | 24 | 14 |
| Sign Control | Free | | | Free | Stop | |

Intersection Summary

Area Type: Other
 Control Type: Unsignalized
 Intersection Capacity Utilization 14.8% ICU Level of Service A
 Analysis Period (min) 15

Lanes, Volumes, Timings
15: Brant Street & Birch Avenue

AM Peak Period
06-07-2019



| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|----------------------------|------|-------|-------|-------|-------|-------|------|------|-------|-------|-------|-------|
| Lane Configurations | | | | | | | | | | | | |
| Traffic Volume (vph) | 0 | 24 | 35 | 27 | 32 | 0 | 0 | 0 | 0 | 21 | 208 | 33 |
| Future Volume (vph) | 0 | 24 | 35 | 27 | 32 | 0 | 0 | 0 | 0 | 21 | 208 | 33 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (m) | 0.0 | | 0.0 | 0.0 | | 0.0 | 0.0 | | 0.0 | 0.0 | | 150.0 |
| Storage Lanes | 0 | | 0 | 0 | | 0 | 0 | | 0 | 0 | | 0 |
| Taper Length (m) | 15.0 | | | 15.0 | | | 15.0 | | | 15.0 | | |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.91 | 0.91 | 0.91 |
| Frt | | 0.920 | | | | | | | | | | 0.981 |
| Flt Protected | | | | | 0.978 | | | | | | | 0.996 |
| Satd. Flow (prot) | 0 | 1402 | 0 | 0 | 1649 | 0 | 0 | 0 | 0 | 0 | 4419 | 0 |
| Flt Permitted | | | | | 0.887 | | | | | | | 0.996 |
| Satd. Flow (perm) | 0 | 1402 | 0 | 0 | 1496 | 0 | 0 | 0 | 0 | 0 | 4419 | 0 |
| Right Turn on Red | | | Yes | | | Yes | | | Yes | | | Yes |
| Satd. Flow (RTOR) | | 38 | | | | | | | | | | 36 |
| Link Speed (k/h) | | 50 | | | 50 | | | 50 | | | | 50 |
| Link Distance (m) | | 157.8 | | | 106.5 | | | 75.7 | | | | 242.7 |
| Travel Time (s) | | 11.4 | | | 7.7 | | | 5.5 | | | | 17.5 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles (%) | 0% | 36% | 10% | 4% | 13% | 0% | 0% | 0% | 0% | 16% | 10% | 13% |
| Adj. Flow (vph) | 0 | 26 | 38 | 29 | 35 | 0 | 0 | 0 | 0 | 23 | 226 | 36 |
| Shared Lane Traffic (%) | | | | | | | | | | | | |
| Lane Group Flow (vph) | 0 | 64 | 0 | 0 | 64 | 0 | 0 | 0 | 0 | 0 | 285 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(m) | | 0.0 | | | 0.0 | | | 0.0 | | | | 0.0 |
| Link Offset(m) | | 0.0 | | | 0.0 | | | 0.0 | | | | 0.0 |
| Crosswalk Width(m) | | 1.6 | | | 1.6 | | | 1.6 | | | | 1.6 |
| Two way Left Turn Lane | | | | | | | | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | 24 | | 14 | 24 | | 14 | 24 | | 14 | 24 | | 14 |
| Turn Type | | NA | | Perm | NA | | | | | | Perm | NA |
| Protected Phases | | 4 | | | 8 | | | | | | | 2 |
| Permitted Phases | | | | 8 | | | | | | 2 | | |
| Minimum Split (s) | | 32.5 | | 32.5 | 32.5 | | | | | 23.4 | 23.4 | |
| Total Split (s) | | 46.0 | | 46.0 | 46.0 | | | | | 44.0 | 44.0 | |
| Total Split (%) | | 51.1% | | 51.1% | 51.1% | | | | | 48.9% | 48.9% | |
| Maximum Green (s) | | 40.5 | | 40.5 | 40.5 | | | | | 38.6 | 38.6 | |
| Yellow Time (s) | | 3.3 | | 3.3 | 3.3 | | | | | 3.3 | 3.3 | |
| All-Red Time (s) | | 2.2 | | 2.2 | 2.2 | | | | | 2.1 | 2.1 | |
| Lost Time Adjust (s) | | 0.0 | | | 0.0 | | | | | | | 0.0 |
| Total Lost Time (s) | | 5.5 | | | 5.5 | | | | | | | 5.4 |
| Lead/Lag | | | | | | | | | | | | |
| Lead-Lag Optimize? | | | | | | | | | | | | |
| Walk Time (s) | | 17.0 | | 17.0 | 17.0 | | | | | 7.0 | 7.0 | |
| Flash Dont Walk (s) | | 10.0 | | 10.0 | 10.0 | | | | | 11.0 | 11.0 | |
| Pedestrian Calls (#/hr) | | 0 | | 0 | 0 | | | | | 0 | 0 | |
| Act Effct Green (s) | | 40.5 | | | 40.5 | | | | | | | 38.6 |
| Actuated g/C Ratio | | 0.45 | | | 0.45 | | | | | | | 0.43 |

Lanes, Volumes, Timings
15: Brant Street & Birch Avenue

AM Peak Period
06-07-2019

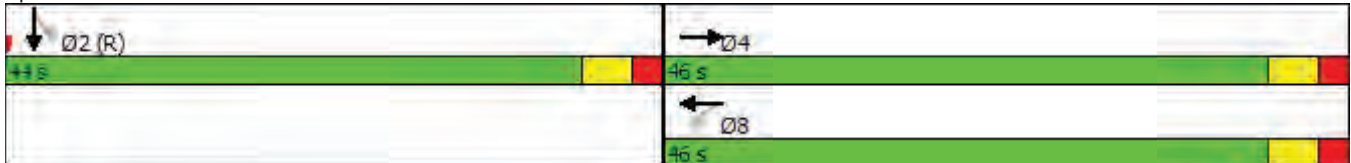


| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|------------------------|-----|-------|-----|-----|------|-----|-----|------|-----|-----|-----|-------|
| v/c Ratio | | 0.10 | | | 0.10 | | | | | | | 0.15 |
| Control Delay | | 7.9 | | | 14.8 | | | | | | | 7.4 |
| Queue Delay | | 0.0 | | | 0.0 | | | | | | | 0.0 |
| Total Delay | | 7.9 | | | 14.8 | | | | | | | 7.4 |
| LOS | | A | | | B | | | | | | | A |
| Approach Delay | | 7.9 | | | 14.8 | | | | | | | 7.4 |
| Approach LOS | | A | | | B | | | | | | | A |
| Queue Length 50th (m) | | 2.4 | | | 6.2 | | | | | | | 15.3 |
| Queue Length 95th (m) | | 9.3 | | | 13.3 | | | | | | | 23.1 |
| Internal Link Dist (m) | | 133.8 | | | 82.5 | | | 51.7 | | | | 218.7 |
| Turn Bay Length (m) | | | | | | | | | | | | |
| Base Capacity (vph) | | 651 | | | 673 | | | | | | | 1915 |
| Starvation Cap Reductn | | 0 | | | 0 | | | | | | | 0 |
| Spillback Cap Reductn | | 0 | | | 0 | | | | | | | 0 |
| Storage Cap Reductn | | 0 | | | 0 | | | | | | | 0 |
| Reduced v/c Ratio | | 0.10 | | | 0.10 | | | | | | | 0.15 |

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 0 (0%), Referenced to phase 2:SBTL and 6:, Start of Green
 Natural Cycle: 60
 Control Type: Pretimed
 Maximum v/c Ratio: 0.15
 Intersection Signal Delay: 8.6
 Intersection Capacity Utilization 27.3%
 Analysis Period (min) 15
 Intersection LOS: A
 ICU Level of Service A

Splits and Phases: 15: Brant Street & Birch Avenue



Lanes, Volumes, Timings
25: Hillyard Street & Brant Street

AM Peak Period
06-07-2019



| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|----------------------------|------|-------|-------|------|-------|-------|------|-------|-------|------|-------|-------|
| Lane Configurations | | ↕ | | | ↕ | | | ↕ | | | ↕ | |
| Traffic Volume (vph) | 6 | 25 | 29 | 53 | 13 | 4 | 5 | 11 | 11 | 1 | 7 | 8 |
| Future Volume (vph) | 6 | 25 | 29 | 53 | 13 | 4 | 5 | 11 | 11 | 1 | 7 | 8 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt | | 0.935 | | | 0.993 | | | 0.944 | | | 0.932 | |
| Flt Protected | | 0.995 | | | 0.963 | | | 0.991 | | | 0.997 | |
| Satd. Flow (prot) | 0 | 1681 | 0 | 0 | 1591 | 0 | 0 | 1650 | 0 | 0 | 1514 | 0 |
| Flt Permitted | | 0.995 | | | 0.963 | | | 0.991 | | | 0.997 | |
| Satd. Flow (perm) | 0 | 1681 | 0 | 0 | 1591 | 0 | 0 | 1650 | 0 | 0 | 1514 | 0 |
| Link Speed (k/h) | | 48 | | | 48 | | | 48 | | | 48 | |
| Link Distance (m) | | 103.4 | | | 96.5 | | | 111.2 | | | 105.1 | |
| Travel Time (s) | | 7.8 | | | 7.2 | | | 8.3 | | | 7.9 | |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles (%) | 0% | 4% | 0% | 10% | 8% | 25% | 0% | 10% | 0% | 0% | 14% | 13% |
| Adj. Flow (vph) | 7 | 27 | 32 | 58 | 14 | 4 | 5 | 12 | 12 | 1 | 8 | 9 |
| Shared Lane Traffic (%) | | | | | | | | | | | | |
| Lane Group Flow (vph) | 0 | 66 | 0 | 0 | 76 | 0 | 0 | 29 | 0 | 0 | 18 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(m) | | 0.0 | | | 0.0 | | | 0.0 | | | 0.0 | |
| Link Offset(m) | | 0.0 | | | 0.0 | | | 0.0 | | | 0.0 | |
| Crosswalk Width(m) | | 1.6 | | | 1.6 | | | 1.6 | | | 1.6 | |
| Two way Left Turn Lane | | | | | | | | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | 24 | | 14 | 24 | | 14 | 24 | | 14 | 24 | | 14 |
| Sign Control | | Free | | | Free | | | Stop | | | Stop | |

Intersection Summary

| | |
|-----------------------------------|--------------|
| Area Type: | Other |
| Control Type: | Unsignalized |
| Intersection Capacity Utilization | 20.5% |
| ICU Level of Service | A |
| Analysis Period (min) | 15 |

Lanes, Volumes, Timings
27: Birch Avenue & Site Access #1

AM Peak Period
06-07-2019



| Lane Group | EBL | EBR | NBL | NBT | SBT | SBR |
|----------------------------|-------|-------|------|-------|------|-------|
| Lane Configurations | | | | | | |
| Traffic Volume (vph) | 0 | 6 | 0 | 0 | 271 | 0 |
| Future Volume (vph) | 0 | 6 | 0 | 0 | 271 | 0 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 0.91 | 0.91 |
| Fr _t | 0.850 | | | | | |
| Fl _t Protected | | | | | | |
| Satd. Flow (prot) | 1837 | 781 | 0 | 0 | 4558 | 0 |
| Fl _t Permitted | | | | | | |
| Satd. Flow (perm) | 1837 | 781 | 0 | 0 | 4558 | 0 |
| Link Speed (k/h) | 48 | | | 50 | 48 | |
| Link Distance (m) | 51.6 | | | 315.9 | 75.7 | |
| Travel Time (s) | 3.9 | | | 22.7 | 5.7 | |
| Peak Hour Factor | 0.70 | 0.70 | 0.92 | 0.92 | 0.92 | 0.70 |
| Heavy Vehicles (%) | 0% | 100% | 0% | 0% | 10% | 100% |
| Adj. Flow (vph) | 0 | 9 | 0 | 0 | 295 | 0 |
| Shared Lane Traffic (%) | | | | | | |
| Lane Group Flow (vph) | 0 | 9 | 0 | 0 | 295 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No |
| Lane Alignment | Left | Right | Left | Left | Left | Right |
| Median Width(m) | 3.3 | | | 0.0 | 0.0 | |
| Link Offset(m) | 0.0 | | | 0.0 | 0.0 | |
| Crosswalk Width(m) | 1.6 | | | 1.6 | 1.6 | |
| Two way Left Turn Lane | | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | 24 | 14 | 24 | | | 14 |
| Sign Control | Stop | | | Free | Free | |

Intersection Summary

| | |
|-----------------------------------|------------------------|
| Area Type: | Other |
| Control Type: | Unsignalized |
| Intersection Capacity Utilization | 15.2% |
| Analysis Period (min) | 15 |
| | ICU Level of Service A |

Lanes, Volumes, Timings
31: Wentworth Street N & Munroe Street

AM Peak Period
06-07-2019



| Lane Group | WBL | WBR | NBT | NBR | SBL | SBT |
|----------------------------|-------|-------|-------|-------|------|-------|
| Lane Configurations | | | | | | |
| Traffic Volume (vph) | 27 | 8 | 179 | 49 | 2 | 311 |
| Future Volume (vph) | 27 | 8 | 179 | 49 | 2 | 311 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Util. Factor | 1.00 | 1.00 | 0.95 | 0.95 | 0.95 | 0.95 |
| Frt | 0.968 | | 0.968 | | | |
| Flt Protected | 0.963 | | | | | |
| Satd. Flow (prot) | 1466 | 0 | 3071 | 0 | 0 | 3194 |
| Flt Permitted | 0.963 | | | | | |
| Satd. Flow (perm) | 1466 | 0 | 3071 | 0 | 0 | 3194 |
| Link Speed (k/h) | 48 | | 50 | | | 50 |
| Link Distance (m) | 118.1 | | 172.4 | | | 236.6 |
| Travel Time (s) | 8.9 | | 12.4 | | | 17.0 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles (%) | 13% | 29% | 10% | 10% | 50% | 9% |
| Adj. Flow (vph) | 29 | 9 | 195 | 53 | 2 | 338 |
| Shared Lane Traffic (%) | | | | | | |
| Lane Group Flow (vph) | 38 | 0 | 248 | 0 | 0 | 340 |
| Enter Blocked Intersection | No | No | No | No | No | No |
| Lane Alignment | Left | Right | Left | Right | Left | Left |
| Median Width(m) | 3.3 | | 0.0 | | | 0.0 |
| Link Offset(m) | 0.0 | | 0.0 | | | 0.0 |
| Crosswalk Width(m) | 1.6 | | 1.6 | | | 1.6 |
| Two way Left Turn Lane | | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | 24 | 14 | | 14 | 24 | |
| Sign Control | Stop | | Free | | | Free |

Intersection Summary

| | |
|-----------------------------------|------------------------|
| Area Type: | Other |
| Control Type: | Unsignalized |
| Intersection Capacity Utilization | 20.0% |
| Analysis Period (min) | 15 |
| | ICU Level of Service A |

Lanes, Volumes, Timings
34: Hillyard Street & Site Access #3

AM Peak Period
06-07-2019



| Lane Group | WBL | WBR | NBT | NBR | SBL | SBT |
|----------------------------|-------|-------|-------|-------|------|-------|
| Lane Configurations | | | | | | |
| Traffic Volume (vph) | 2 | 8 | 32 | 17 | 68 | 33 |
| Future Volume (vph) | 2 | 8 | 32 | 17 | 68 | 33 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt | 0.894 | | 0.945 | | | |
| Flt Protected | 0.989 | | | | | 0.965 |
| Satd. Flow (prot) | 1624 | 0 | 1638 | 0 | 0 | 1726 |
| Flt Permitted | 0.989 | | | | | 0.965 |
| Satd. Flow (perm) | 1624 | 0 | 1638 | 0 | 0 | 1726 |
| Link Speed (k/h) | 48 | | 48 | | | 48 |
| Link Distance (m) | 48.3 | | 26.1 | | | 111.2 |
| Travel Time (s) | 3.6 | | 2.0 | | | 8.3 |
| Peak Hour Factor | 0.70 | 0.70 | 0.92 | 0.70 | 0.70 | 0.92 |
| Heavy Vehicles (%) | 0% | 0% | 10% | 0% | 0% | 10% |
| Adj. Flow (vph) | 3 | 11 | 35 | 24 | 97 | 36 |
| Shared Lane Traffic (%) | | | | | | |
| Lane Group Flow (vph) | 14 | 0 | 59 | 0 | 0 | 133 |
| Enter Blocked Intersection | No | No | No | No | No | No |
| Lane Alignment | Left | Right | Left | Right | Left | Left |
| Median Width(m) | 3.3 | | 0.0 | | | 0.0 |
| Link Offset(m) | 0.0 | | 0.0 | | | 0.0 |
| Crosswalk Width(m) | 1.6 | | 1.6 | | | 1.6 |
| Two way Left Turn Lane | | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | 24 | 14 | | 14 | 24 | |
| Sign Control | Stop | | Free | | | Free |

Intersection Summary

| | |
|-----------------------------------|------------------------|
| Area Type: | Other |
| Control Type: | Unsignalized |
| Intersection Capacity Utilization | 22.2% |
| Analysis Period (min) | 15 |
| | ICU Level of Service A |

Lanes, Volumes, Timings
3: Wentworth Street N & Burlington Street E

PM Peak Period
06-07-2019



| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|----------------------------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|
| Lane Configurations | | | | | | | | | | | | |
| Traffic Volume (vph) | 10 | 637 | 36 | 157 | 648 | 10 | 15 | 7 | 106 | 10 | 12 | 8 |
| Future Volume (vph) | 10 | 637 | 36 | 157 | 648 | 10 | 15 | 7 | 106 | 10 | 12 | 8 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (m) | 60.0 | | 0.0 | 50.0 | | 0.0 | 0.0 | | 0.0 | 0.0 | | 0.0 |
| Storage Lanes | 1 | | 0 | 1 | | 0 | 0 | | 0 | 0 | | 0 |
| Taper Length (m) | 15.0 | | | 15.0 | | | 15.0 | | | 15.0 | | |
| Lane Util. Factor | 1.00 | 0.95 | 0.95 | 1.00 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 1.00 | 1.00 | 1.00 |
| Frt | | 0.992 | | | 0.998 | | | 0.876 | | | 0.963 | |
| Flt Protected | 0.950 | | | 0.950 | | | | 0.994 | | | 0.984 | |
| Satd. Flow (prot) | 1572 | 3073 | 0 | 1572 | 3161 | 0 | 0 | 2683 | 0 | 0 | 1420 | 0 |
| Flt Permitted | 0.380 | | | 0.230 | | | | 0.929 | | | 0.912 | |
| Satd. Flow (perm) | 629 | 3073 | 0 | 381 | 3161 | 0 | 0 | 2508 | 0 | 0 | 1316 | 0 |
| Right Turn on Red | | | Yes | | | Yes | | | Yes | | | Yes |
| Satd. Flow (RTOR) | | 7 | | | 3 | | | 115 | | | 9 | |
| Link Speed (k/h) | | 50 | | | 50 | | | 50 | | | 50 | |
| Link Distance (m) | | 125.0 | | | 716.7 | | | 215.5 | | | 91.4 | |
| Travel Time (s) | | 9.0 | | | 51.6 | | | 15.5 | | | 6.6 | |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles (%) | 11% | 12% | 24% | 11% | 10% | 22% | 0% | 0% | 16% | 22% | 11% | 40% |
| Adj. Flow (vph) | 11 | 692 | 39 | 171 | 704 | 11 | 16 | 8 | 115 | 11 | 13 | 9 |
| Shared Lane Traffic (%) | | | | | | | | | | | | |
| Lane Group Flow (vph) | 11 | 731 | 0 | 171 | 715 | 0 | 0 | 139 | 0 | 0 | 33 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(m) | | 3.3 | | | 3.3 | | | 0.0 | | | 0.0 | |
| Link Offset(m) | | 0.0 | | | 0.0 | | | 0.0 | | | 0.0 | |
| Crosswalk Width(m) | | 4.9 | | | 4.9 | | | 4.9 | | | 4.9 | |
| Two way Left Turn Lane | | | | | Yes | | | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | 24 | | 14 | 24 | | 14 | 24 | | 14 | 24 | | 14 |
| Turn Type | Perm | NA | | pm+pt | NA | | Perm | NA | | Perm | NA | |
| Protected Phases | | 2 | | 1 | 6 | | | 4 | | | 8 | |
| Permitted Phases | 2 | | | 6 | | | 4 | | | 8 | | |
| Minimum Split (s) | 29.0 | 29.0 | | 9.5 | 37.2 | | 35.0 | 35.0 | | 35.0 | 35.0 | |
| Total Split (s) | 38.0 | 38.0 | | 17.0 | 55.0 | | 35.0 | 35.0 | | 35.0 | 35.0 | |
| Total Split (%) | 42.2% | 42.2% | | 18.9% | 61.1% | | 38.9% | 38.9% | | 38.9% | 38.9% | |
| Maximum Green (s) | 32.0 | 32.0 | | 13.0 | 48.8 | | 29.0 | 29.0 | | 29.0 | 29.0 | |
| Yellow Time (s) | 3.3 | 3.3 | | 3.0 | 3.3 | | 3.3 | 3.3 | | 3.3 | 3.3 | |
| All-Red Time (s) | 2.7 | 2.7 | | 1.0 | 2.9 | | 2.7 | 2.7 | | 2.7 | 2.7 | |
| Lost Time Adjust (s) | 0.0 | 0.0 | | 0.0 | 0.0 | | 0.0 | 0.0 | | 0.0 | 0.0 | |
| Total Lost Time (s) | 6.0 | 6.0 | | 4.0 | 6.2 | | 6.0 | 6.0 | | 6.0 | 6.0 | |
| Lead/Lag | Lag | Lag | | Lead | | | | | | | | |
| Lead-Lag Optimize? | Yes | Yes | | Yes | | | | | | | | |
| Walk Time (s) | 7.0 | 7.0 | | | 15.0 | | 11.0 | 11.0 | | 11.0 | 11.0 | |
| Flash Dont Walk (s) | 16.0 | 16.0 | | | 16.0 | | 18.0 | 18.0 | | 18.0 | 18.0 | |
| Pedestrian Calls (#/hr) | 0 | 0 | | | 0 | | 0 | 0 | | 0 | 0 | |
| Act Effct Green (s) | 32.0 | 32.0 | | 51.0 | 48.8 | | 29.0 | 29.0 | | 29.0 | 29.0 | |
| Actuated g/C Ratio | 0.36 | 0.36 | | 0.57 | 0.54 | | 0.32 | 0.32 | | 0.32 | 0.32 | |

Lanes, Volumes, Timings
3: Wentworth Street N & Burlington Street E

PM Peak Period
06-07-2019

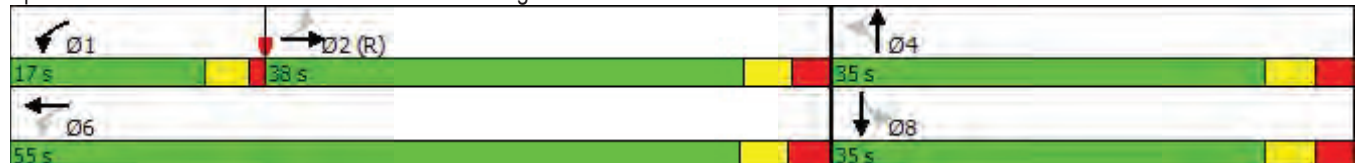


| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|------------------------|------|-------|-----|------|-------|-----|-----|-------|-----|-----|-----|------|
| v/c Ratio | 0.05 | 0.67 | | 0.44 | 0.42 | | | 0.16 | | | | 0.08 |
| Control Delay | 20.0 | 27.8 | | 13.2 | 13.0 | | | 6.8 | | | | 17.5 |
| Queue Delay | 0.0 | 0.0 | | 0.0 | 0.0 | | | 0.0 | | | | 0.0 |
| Total Delay | 20.0 | 27.8 | | 13.2 | 13.0 | | | 6.8 | | | | 17.5 |
| LOS | B | C | | B | B | | | A | | | | B |
| Approach Delay | | 27.7 | | | 13.1 | | | 6.8 | | | | 17.5 |
| Approach LOS | | C | | | B | | | A | | | | B |
| Queue Length 50th (m) | 1.2 | 54.8 | | 13.4 | 36.0 | | | 1.4 | | | | 2.8 |
| Queue Length 95th (m) | 4.8 | 74.2 | | 23.3 | 48.5 | | | 7.7 | | | | 9.3 |
| Internal Link Dist (m) | | 101.0 | | | 692.7 | | | 191.5 | | | | 67.4 |
| Turn Bay Length (m) | 60.0 | | | 50.0 | | | | | | | | |
| Base Capacity (vph) | 223 | 1097 | | 387 | 1715 | | | 886 | | | | 430 |
| Starvation Cap Reductn | 0 | 0 | | 0 | 0 | | | 0 | | | | 0 |
| Spillback Cap Reductn | 0 | 0 | | 0 | 0 | | | 0 | | | | 0 |
| Storage Cap Reductn | 0 | 0 | | 0 | 0 | | | 0 | | | | 0 |
| Reduced v/c Ratio | 0.05 | 0.67 | | 0.44 | 0.42 | | | 0.16 | | | | 0.08 |

Intersection Summary

| | |
|-----------------------------------|--|
| Area Type: | Other |
| Cycle Length: | 90 |
| Actuated Cycle Length: | 90 |
| Offset: | 0 (0%), Referenced to phase 2:EBTL, Start of Green |
| Natural Cycle: | 75 |
| Control Type: | Pretimed |
| Maximum v/c Ratio: | 0.67 |
| Intersection Signal Delay: | 18.7 |
| Intersection LOS: | B |
| Intersection Capacity Utilization | 58.4% |
| ICU Level of Service | B |
| Analysis Period (min) | 15 |

Splits and Phases: 3: Wentworth Street N & Burlington Street E



Lanes, Volumes, Timings
9: Birch Avenue & Burlington Street E

PM Peak Period
06-07-2019



| Lane Group | EBT | EBR | WBL | WBT | NBL | NBR |
|----------------------------|-------|-------|-------|-------|-------|-------|
| Lane Configurations | ↑↑↑ | | ↔ | ↑↑↑ | | |
| Traffic Volume (vph) | 1121 | 29 | 261 | 1226 | 0 | 0 |
| Future Volume (vph) | 1121 | 29 | 261 | 1226 | 0 | 0 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (m) | | 0.0 | 125.0 | | 0.0 | 0.0 |
| Storage Lanes | | 0 | 2 | | 0 | 0 |
| Taper Length (m) | | | 15.0 | | 15.0 | |
| Lane Util. Factor | 0.91 | 0.91 | 0.97 | 0.91 | 1.00 | 1.00 |
| Frt | 0.996 | | | | | |
| Flt Protected | | | 0.950 | | | |
| Satd. Flow (prot) | 4718 | 0 | 3134 | 4643 | 0 | 0 |
| Flt Permitted | | | 0.950 | | | |
| Satd. Flow (perm) | 4718 | 0 | 3134 | 4643 | 0 | 0 |
| Right Turn on Red | | Yes | | | | Yes |
| Satd. Flow (RTOR) | 9 | | | | | |
| Link Speed (k/h) | 50 | | | 50 | 50 | |
| Link Distance (m) | 716.7 | | | 130.8 | 242.7 | |
| Travel Time (s) | 51.6 | | | 9.4 | 17.5 | |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles (%) | 6% | 0% | 8% | 8% | 0% | 0% |
| Adj. Flow (vph) | 1218 | 32 | 284 | 1333 | 0 | 0 |
| Shared Lane Traffic (%) | | | | | | |
| Lane Group Flow (vph) | 1250 | 0 | 284 | 1333 | 0 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No |
| Lane Alignment | Left | Right | Left | Left | Left | Right |
| Median Width(m) | 6.6 | | | 6.6 | 0.0 | |
| Link Offset(m) | 0.0 | | | 0.0 | 0.0 | |
| Crosswalk Width(m) | 1.6 | | | 1.6 | 1.6 | |
| Two way Left Turn Lane | Yes | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | | 14 | 24 | | 24 | 14 |
| Turn Type | NA | | Prot | NA | | |
| Protected Phases | 2 | | 4 | 2 4 | | |
| Permitted Phases | | | | | | |
| Minimum Split (s) | 47.0 | | 15.1 | | | |
| Total Split (s) | 64.0 | | 26.0 | | | |
| Total Split (%) | 71.1% | | 28.9% | | | |
| Maximum Green (s) | 59.0 | | 20.9 | | | |
| Yellow Time (s) | 3.7 | | 3.7 | | | |
| All-Red Time (s) | 1.3 | | 1.4 | | | |
| Lost Time Adjust (s) | 0.0 | | 0.0 | | | |
| Total Lost Time (s) | 5.0 | | 5.1 | | | |
| Lead/Lag | | | | | | |
| Lead-Lag Optimize? | | | | | | |
| Walk Time (s) | 30.0 | | | | | |
| Flash Dont Walk (s) | 12.0 | | | | | |
| Pedestrian Calls (#/hr) | 0 | | | | | |
| Act Effct Green (s) | 59.0 | | 20.9 | 90.0 | | |
| Actuated g/C Ratio | 0.66 | | 0.23 | 1.00 | | |

Lanes, Volumes, Timings
 9: Birch Avenue & Burlington Street E

PM Peak Period
 06-07-2019



| Lane Group | EBT | EBR | WBL | WBT | NBL | NBR |
|------------------------|-------|-----|-------|-------|-------|-----|
| v/c Ratio | 0.40 | | 0.39 | 0.29 | | |
| Control Delay | 14.3 | | 31.1 | 0.2 | | |
| Queue Delay | 0.0 | | 0.0 | 0.0 | | |
| Total Delay | 14.3 | | 31.1 | 0.2 | | |
| LOS | B | | C | A | | |
| Approach Delay | 14.3 | | | 5.6 | | |
| Approach LOS | B | | | A | | |
| Queue Length 50th (m) | 47.9 | | 21.4 | 0.0 | | |
| Queue Length 95th (m) | 63.7 | | 32.6 | 0.0 | | |
| Internal Link Dist (m) | 692.7 | | | 106.8 | 218.7 | |
| Turn Bay Length (m) | | | 125.0 | | | |
| Base Capacity (vph) | 3096 | | 727 | 4643 | | |
| Starvation Cap Reductn | 0 | | 0 | 0 | | |
| Spillback Cap Reductn | 0 | | 0 | 0 | | |
| Storage Cap Reductn | 0 | | 0 | 0 | | |
| Reduced v/c Ratio | 0.40 | | 0.39 | 0.29 | | |

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 0 (0%), Referenced to phase 2:EBWB and 6:, Start of Green
 Natural Cycle: 65
 Control Type: Pretimed
 Maximum v/c Ratio: 0.40
 Intersection Signal Delay: 9.4
 Intersection Capacity Utilization 39.1%
 Analysis Period (min) 15
 Intersection LOS: A
 ICU Level of Service A

Splits and Phases: 9: Birch Avenue & Burlington Street E



Lanes, Volumes, Timings
13: Brant Street & Wentworth Street N

PM Peak Period
06-07-2019



| Lane Group | WBL | WBR | NBT | NBR | SBL | SBT |
|----------------------------|-------|-------|-------|-------|------|-------|
| Lane Configurations | | | | | | |
| Traffic Volume (vph) | 53 | 8 | 134 | 37 | 13 | 236 |
| Future Volume (vph) | 53 | 8 | 134 | 37 | 13 | 236 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Util. Factor | 1.00 | 1.00 | 0.95 | 0.95 | 0.95 | 0.95 |
| Frt | 0.982 | | 0.968 | | | |
| Flt Protected | 0.959 | | | | | 0.997 |
| Satd. Flow (prot) | 1631 | 0 | 3170 | 0 | 0 | 3228 |
| Flt Permitted | 0.959 | | | | | 0.997 |
| Satd. Flow (perm) | 1631 | 0 | 3170 | 0 | 0 | 3228 |
| Link Speed (k/h) | 48 | | 48 | | | 48 |
| Link Distance (m) | 104.2 | | 236.6 | | | 215.5 |
| Travel Time (s) | 7.8 | | 17.7 | | | 16.2 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles (%) | 7% | 0% | 7% | 5% | 22% | 7% |
| Adj. Flow (vph) | 58 | 9 | 146 | 40 | 14 | 257 |
| Shared Lane Traffic (%) | | | | | | |
| Lane Group Flow (vph) | 67 | 0 | 186 | 0 | 0 | 271 |
| Enter Blocked Intersection | No | No | No | No | No | No |
| Lane Alignment | Left | Right | Left | Right | Left | Left |
| Median Width(m) | 3.3 | | 0.0 | | | 0.0 |
| Link Offset(m) | 0.0 | | 0.0 | | | 0.0 |
| Crosswalk Width(m) | 1.6 | | 1.6 | | | 1.6 |
| Two way Left Turn Lane | | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | 24 | 14 | | 14 | 24 | |
| Sign Control | Stop | | Free | | | Free |

Intersection Summary

| | |
|-----------------------------------|------------------------|
| Area Type: | Other |
| Control Type: | Unsignalized |
| Intersection Capacity Utilization | 25.2% |
| Analysis Period (min) | 15 |
| | ICU Level of Service A |

Lanes, Volumes, Timings
 14: Site Access #2 & Brant Street

PM Peak Period
 06-07-2019



| Lane Group | EBT | EBR | WBL | WBT | NBL | NBR |
|----------------------------|------|-------|------|-------|------|-------|
| Lane Configurations | | | | | | |
| Traffic Volume (vph) | 99 | 0 | 1 | 64 | 0 | 0 |
| Future Volume (vph) | 99 | 0 | 1 | 64 | 0 | 0 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt | | | | | | |
| Flt Protected | | | | 0.999 | | |
| Satd. Flow (prot) | 1655 | 0 | 0 | 1579 | 918 | 918 |
| Flt Permitted | | | | 0.999 | | |
| Satd. Flow (perm) | 1655 | 0 | 0 | 1579 | 918 | 918 |
| Link Speed (k/h) | 48 | | | 48 | 48 | |
| Link Distance (m) | 77.1 | | | 157.8 | 61.8 | |
| Travel Time (s) | 5.8 | | | 11.8 | 4.6 | |
| Peak Hour Factor | 0.92 | 0.70 | 0.70 | 0.92 | 0.70 | 0.70 |
| Heavy Vehicles (%) | 11% | 100% | 100% | 15% | 100% | 100% |
| Adj. Flow (vph) | 108 | 0 | 1 | 70 | 0 | 0 |
| Shared Lane Traffic (%) | | | | | | |
| Lane Group Flow (vph) | 108 | 0 | 0 | 71 | 0 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No |
| Lane Alignment | Left | Right | Left | Left | Left | Right |
| Median Width(m) | 0.0 | | | 0.0 | 3.3 | |
| Link Offset(m) | 0.0 | | | 0.0 | 0.0 | |
| Crosswalk Width(m) | 1.6 | | | 1.6 | 1.6 | |
| Two way Left Turn Lane | | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | | 14 | 24 | | 24 | 14 |
| Sign Control | Free | | | Free | Stop | |

Intersection Summary

| | |
|-----------------------------------|------------------------|
| Area Type: | Other |
| Control Type: | Unsignalized |
| Intersection Capacity Utilization | 8.5% |
| Analysis Period (min) | 15 |
| | ICU Level of Service A |

Lanes, Volumes, Timings
15: Brant Street & Birch Avenue

PM Peak Period
06-07-2019



| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|----------------------------|------|-------|-------|-------|-------|-------|------|------|-------|-------|-------|-------|
| Lane Configurations | | | | | | | | | | | | |
| Traffic Volume (vph) | 0 | 13 | 68 | 55 | 30 | 0 | 0 | 0 | 0 | 8 | 218 | 20 |
| Future Volume (vph) | 0 | 13 | 68 | 55 | 30 | 0 | 0 | 0 | 0 | 8 | 218 | 20 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (m) | 0.0 | | 0.0 | 0.0 | | 0.0 | 0.0 | | 0.0 | 0.0 | | 150.0 |
| Storage Lanes | 0 | | 0 | 0 | | 0 | 0 | | 0 | 0 | | 0 |
| Taper Length (m) | 15.0 | | | 15.0 | | | 15.0 | | | 15.0 | | |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.91 | 0.91 | 0.91 |
| Frt | | 0.886 | | | | | | | | | | 0.988 |
| Flt Protected | | | | | 0.969 | | | | | | | 0.998 |
| Satd. Flow (prot) | 0 | 1543 | 0 | 0 | 1692 | 0 | 0 | 0 | 0 | 0 | 4697 | 0 |
| Flt Permitted | | | | | 0.803 | | | | | | | 0.998 |
| Satd. Flow (perm) | 0 | 1543 | 0 | 0 | 1402 | 0 | 0 | 0 | 0 | 0 | 4697 | 0 |
| Right Turn on Red | | | Yes | | | Yes | | | Yes | | | Yes |
| Satd. Flow (RTOR) | | 74 | | | | | | | | | | 19 |
| Link Speed (k/h) | | 50 | | | 50 | | | 50 | | | | 50 |
| Link Distance (m) | | 157.8 | | | 106.5 | | | 75.7 | | | | 242.7 |
| Travel Time (s) | | 11.4 | | | 7.7 | | | 5.5 | | | | 17.5 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles (%) | 0% | 8% | 5% | 2% | 11% | 2% | 0% | 0% | 0% | 29% | 3% | 20% |
| Adj. Flow (vph) | 0 | 14 | 74 | 60 | 33 | 0 | 0 | 0 | 0 | 9 | 237 | 22 |
| Shared Lane Traffic (%) | | | | | | | | | | | | |
| Lane Group Flow (vph) | 0 | 88 | 0 | 0 | 93 | 0 | 0 | 0 | 0 | 0 | 268 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(m) | | 0.0 | | | 0.0 | | | 0.0 | | | | 0.0 |
| Link Offset(m) | | 0.0 | | | 0.0 | | | 0.0 | | | | 0.0 |
| Crosswalk Width(m) | | 1.6 | | | 1.6 | | | 1.6 | | | | 1.6 |
| Two way Left Turn Lane | | | | | | | | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | 24 | | 14 | 24 | | 14 | 24 | | 14 | 24 | | 14 |
| Turn Type | | NA | | Perm | NA | | | | | Perm | | NA |
| Protected Phases | | 4 | | | 8 | | | | | | | 2 |
| Permitted Phases | | | | 8 | | | | | | 2 | | |
| Minimum Split (s) | | 32.5 | | 32.5 | 32.5 | | | | | 23.4 | 23.4 | |
| Total Split (s) | | 50.0 | | 50.0 | 50.0 | | | | | 40.0 | 40.0 | |
| Total Split (%) | | 55.6% | | 55.6% | 55.6% | | | | | 44.4% | 44.4% | |
| Maximum Green (s) | | 44.5 | | 44.5 | 44.5 | | | | | 34.6 | 34.6 | |
| Yellow Time (s) | | 3.3 | | 3.3 | 3.3 | | | | | 3.3 | 3.3 | |
| All-Red Time (s) | | 2.2 | | 2.2 | 2.2 | | | | | 2.1 | 2.1 | |
| Lost Time Adjust (s) | | 0.0 | | | 0.0 | | | | | | | 0.0 |
| Total Lost Time (s) | | 5.5 | | | 5.5 | | | | | | | 5.4 |
| Lead/Lag | | | | | | | | | | | | |
| Lead-Lag Optimize? | | | | | | | | | | | | |
| Walk Time (s) | | 17.0 | | 17.0 | 17.0 | | | | | 7.0 | 7.0 | |
| Flash Dont Walk (s) | | 10.0 | | 10.0 | 10.0 | | | | | 11.0 | 11.0 | |
| Pedestrian Calls (#/hr) | | 0 | | 0 | 0 | | | | | 0 | 0 | |
| Act Effct Green (s) | | 44.5 | | | 44.5 | | | | | | | 34.6 |
| Actuated g/C Ratio | | 0.49 | | | 0.49 | | | | | | | 0.38 |

Lanes, Volumes, Timings
 15: Brant Street & Birch Avenue

PM Peak Period
 06-07-2019

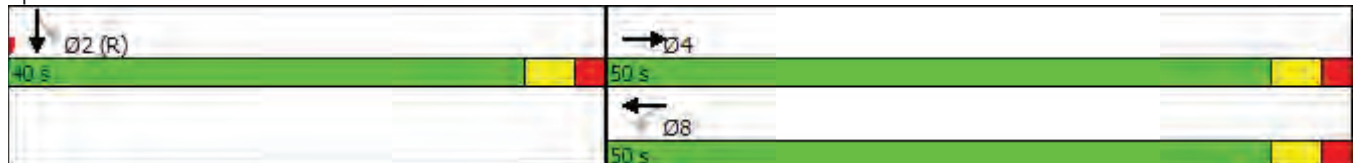


| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|------------------------|-----|-------|-----|-----|------|-----|-----|------|-----|-----|-----|-------|
| v/c Ratio | | 0.11 | | | 0.13 | | | | | | | 0.15 |
| Control Delay | | 4.4 | | | 13.0 | | | | | | | 5.9 |
| Queue Delay | | 0.0 | | | 0.0 | | | | | | | 0.0 |
| Total Delay | | 4.4 | | | 13.0 | | | | | | | 5.9 |
| LOS | | A | | | B | | | | | | | A |
| Approach Delay | | 4.4 | | | 13.0 | | | | | | | 5.9 |
| Approach LOS | | A | | | B | | | | | | | A |
| Queue Length 50th (m) | | 1.2 | | | 8.3 | | | | | | | 12.4 |
| Queue Length 95th (m) | | 8.3 | | | 16.6 | | | | | | | 12.3 |
| Internal Link Dist (m) | | 133.8 | | | 82.5 | | | 51.7 | | | | 218.7 |
| Turn Bay Length (m) | | | | | | | | | | | | |
| Base Capacity (vph) | | 800 | | | 693 | | | | | | | 1817 |
| Starvation Cap Reductn | | 0 | | | 0 | | | | | | | 0 |
| Spillback Cap Reductn | | 0 | | | 0 | | | | | | | 0 |
| Storage Cap Reductn | | 0 | | | 0 | | | | | | | 0 |
| Reduced v/c Ratio | | 0.11 | | | 0.13 | | | | | | | 0.15 |

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 0 (0%), Referenced to phase 2:SBTL and 6:, Start of Green
 Natural Cycle: 60
 Control Type: Pretimed
 Maximum v/c Ratio: 0.15
 Intersection Signal Delay: 7.1
 Intersection Capacity Utilization 28.7%
 Analysis Period (min) 15
 Intersection LOS: A
 ICU Level of Service A

Splits and Phases: 15: Brant Street & Birch Avenue



Lanes, Volumes, Timings
25: Hillyard Street & Brant Street

PM Peak Period
06-07-2019



| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|----------------------------|------|-------|-------|------|-------|-------|------|-------|-------|------|-------|-------|
| Lane Configurations | | ↕ | | | ↕ | | | ↕ | | | ↕ | |
| Traffic Volume (vph) | 1 | 20 | 15 | 29 | 23 | 0 | 30 | 5 | 52 | 7 | 4 | 14 |
| Future Volume (vph) | 1 | 20 | 15 | 29 | 23 | 0 | 30 | 5 | 52 | 7 | 4 | 14 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt | | 0.945 | | | | | | 0.919 | | | 0.925 | |
| Flt Protected | | 0.999 | | | 0.973 | | | 0.983 | | | 0.985 | |
| Satd. Flow (prot) | 0 | 1633 | 0 | 0 | 1787 | 0 | 0 | 1531 | 0 | 0 | 1673 | 0 |
| Flt Permitted | | 0.999 | | | 0.973 | | | 0.983 | | | 0.985 | |
| Satd. Flow (perm) | 0 | 1633 | 0 | 0 | 1787 | 0 | 0 | 1531 | 0 | 0 | 1673 | 0 |
| Link Speed (k/h) | | 48 | | | 48 | | | 48 | | | 48 | |
| Link Distance (m) | | 103.4 | | | 96.5 | | | 111.2 | | | 105.1 | |
| Travel Time (s) | | 7.8 | | | 7.2 | | | 8.3 | | | 7.9 | |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles (%) | 0% | 11% | 0% | 0% | 0% | 0% | 0% | 0% | 14% | 0% | 0% | 0% |
| Adj. Flow (vph) | 1 | 22 | 16 | 32 | 25 | 0 | 33 | 5 | 57 | 8 | 4 | 15 |
| Shared Lane Traffic (%) | | | | | | | | | | | | |
| Lane Group Flow (vph) | 0 | 39 | 0 | 0 | 57 | 0 | 0 | 95 | 0 | 0 | 27 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(m) | | 0.0 | | | 0.0 | | | 0.0 | | | 0.0 | |
| Link Offset(m) | | 0.0 | | | 0.0 | | | 0.0 | | | 0.0 | |
| Crosswalk Width(m) | | 1.6 | | | 1.6 | | | 1.6 | | | 1.6 | |
| Two way Left Turn Lane | | | | | | | | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | 24 | | 14 | 24 | | 14 | 24 | | 14 | 24 | | 14 |
| Sign Control | | Free | | | Free | | | Stop | | | Stop | |

Intersection Summary

| | |
|-----------------------------------|--------------|
| Area Type: | Other |
| Control Type: | Unsignalized |
| Intersection Capacity Utilization | 23.5% |
| ICU Level of Service | A |
| Analysis Period (min) | 15 |

Lanes, Volumes, Timings
27: Birch Avenue & Site Access #1

PM Peak Period
06-07-2019



| Lane Group | EBL | EBR | NBL | NBT | SBT | SBR |
|----------------------------|------|-------|------|-------|-------|-------|
| Lane Configurations | | | | | | |
| Traffic Volume (vph) | 0 | 6 | 0 | 0 | 340 | 2 |
| Future Volume (vph) | 0 | 6 | 0 | 0 | 340 | 2 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 0.91 | 0.91 |
| Frt | | 0.850 | | | 0.999 | |
| Flt Protected | | | | | | |
| Satd. Flow (prot) | 1837 | 781 | 0 | 0 | 4827 | 0 |
| Flt Permitted | | | | | | |
| Satd. Flow (perm) | 1837 | 781 | 0 | 0 | 4827 | 0 |
| Link Speed (k/h) | 48 | | | 50 | 48 | |
| Link Distance (m) | 49.2 | | | 315.9 | 75.7 | |
| Travel Time (s) | 3.7 | | | 22.7 | 5.7 | |
| Peak Hour Factor | 0.70 | 0.70 | 0.92 | 0.92 | 0.92 | 0.70 |
| Heavy Vehicles (%) | 0% | 100% | 0% | 0% | 3% | 100% |
| Adj. Flow (vph) | 0 | 9 | 0 | 0 | 370 | 3 |
| Shared Lane Traffic (%) | | | | | | |
| Lane Group Flow (vph) | 0 | 9 | 0 | 0 | 373 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No |
| Lane Alignment | Left | Right | Left | Left | Left | Right |
| Median Width(m) | 3.3 | | | 0.0 | 0.0 | |
| Link Offset(m) | 0.0 | | | 0.0 | 0.0 | |
| Crosswalk Width(m) | 1.6 | | | 1.6 | 1.6 | |
| Two way Left Turn Lane | | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | 24 | 14 | 24 | | | 14 |
| Sign Control | Stop | | | Free | Free | |

Intersection Summary

| | |
|-----------------------------------|------------------------|
| Area Type: | Other |
| Control Type: | Unsignalized |
| Intersection Capacity Utilization | 16.6% |
| Analysis Period (min) | 15 |
| | ICU Level of Service A |

Lanes, Volumes, Timings
31: Wentworth Street N & Munroe Street

PM Peak Period
06-07-2019



| Lane Group | WBL | WBR | NBT | NBR | SBL | SBT |
|----------------------------|-------|-------|-------|-------|------|-------|
| Lane Configurations | | | | | | |
| Traffic Volume (vph) | 60 | 1 | 197 | 41 | 1 | 312 |
| Future Volume (vph) | 60 | 1 | 197 | 41 | 1 | 312 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Util. Factor | 1.00 | 1.00 | 0.95 | 0.95 | 0.95 | 0.95 |
| Frt | 0.998 | | 0.974 | | | |
| Flt Protected | 0.953 | | | | | |
| Satd. Flow (prot) | 1665 | 0 | 3161 | 0 | 0 | 3232 |
| Flt Permitted | 0.953 | | | | | |
| Satd. Flow (perm) | 1665 | 0 | 3161 | 0 | 0 | 3232 |
| Link Speed (k/h) | 48 | | 48 | | | 48 |
| Link Distance (m) | 118.1 | | 172.4 | | | 236.6 |
| Travel Time (s) | 8.9 | | 12.9 | | | 17.7 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles (%) | 5% | 0% | 7% | 10% | 0% | 8% |
| Adj. Flow (vph) | 65 | 1 | 214 | 45 | 1 | 339 |
| Shared Lane Traffic (%) | | | | | | |
| Lane Group Flow (vph) | 66 | 0 | 259 | 0 | 0 | 340 |
| Enter Blocked Intersection | No | No | No | No | No | No |
| Lane Alignment | Left | Right | Left | Right | Left | Left |
| Median Width(m) | 3.3 | | 0.0 | | | 0.0 |
| Link Offset(m) | 0.0 | | 0.0 | | | 0.0 |
| Crosswalk Width(m) | 1.6 | | 1.6 | | | 1.6 |
| Two way Left Turn Lane | | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | 24 | 14 | | 14 | 24 | |
| Sign Control | Stop | | Free | | | Free |

Intersection Summary

| | |
|-----------------------------------|------------------------|
| Area Type: | Other |
| Control Type: | Unsignalized |
| Intersection Capacity Utilization | 19.4% |
| Analysis Period (min) | 15 |
| | ICU Level of Service A |

Lanes, Volumes, Timings
34: Hillyard Street & Site Access #3

PM Peak Period
06-07-2019



| Lane Group | WBL | WBR | NBT | NBR | SBL | SBT |
|----------------------------|-------|-------|-------|-------|------|-------|
| Lane Configurations | | | | | | |
| Traffic Volume (vph) | 18 | 72 | 32 | 9 | 36 | 44 |
| Future Volume (vph) | 18 | 72 | 32 | 9 | 36 | 44 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt | 0.892 | | 0.963 | | | |
| Flt Protected | 0.990 | | | | | 0.975 |
| Satd. Flow (prot) | 1622 | 0 | 1649 | 0 | 0 | 1708 |
| Flt Permitted | 0.990 | | | | | 0.975 |
| Satd. Flow (perm) | 1622 | 0 | 1649 | 0 | 0 | 1708 |
| Link Speed (k/h) | 48 | | 48 | | | 48 |
| Link Distance (m) | 48.3 | | 26.1 | | | 111.2 |
| Travel Time (s) | 3.6 | | 2.0 | | | 8.3 |
| Peak Hour Factor | 0.70 | 0.70 | 0.92 | 0.70 | 0.70 | 0.92 |
| Heavy Vehicles (%) | 0% | 0% | 10% | 0% | 0% | 10% |
| Adj. Flow (vph) | 26 | 103 | 35 | 13 | 51 | 48 |
| Shared Lane Traffic (%) | | | | | | |
| Lane Group Flow (vph) | 129 | 0 | 48 | 0 | 0 | 99 |
| Enter Blocked Intersection | No | No | No | No | No | No |
| Lane Alignment | Left | Right | Left | Right | Left | Left |
| Median Width(m) | 3.3 | | 0.0 | | | 0.0 |
| Link Offset(m) | 0.0 | | 0.0 | | | 0.0 |
| Crosswalk Width(m) | 1.6 | | 1.6 | | | 1.6 |
| Two way Left Turn Lane | | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | 24 | 14 | | 14 | 24 | |
| Sign Control | Stop | | Free | | | Free |

Intersection Summary

| | |
|-----------------------------------|------------------------|
| Area Type: | Other |
| Control Type: | Unsignalized |
| Intersection Capacity Utilization | 23.1% |
| Analysis Period (min) | 15 |
| | ICU Level of Service A |

APPENDIX E

Synchro Outputs – Future Background 2027 Conditions

Lanes, Volumes, Timings

AM Peak Period

3: Wentworth Street N & Burlington St E/Burlington Street E

05-16-2019



| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|----------------------------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|
| Lane Configurations | | | | | | | | | | | | |
| Traffic Volume (vph) | 2 | 582 | 26 | 239 | 1073 | 10 | 7 | 11 | 126 | 9 | 4 | 11 |
| Future Volume (vph) | 2 | 582 | 26 | 239 | 1073 | 10 | 7 | 11 | 126 | 9 | 4 | 11 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (m) | 60.0 | | 0.0 | 50.0 | | 0.0 | 0.0 | | 0.0 | 0.0 | | 0.0 |
| Storage Lanes | 1 | | 0 | 1 | | 0 | 0 | | 0 | 0 | | 0 |
| Taper Length (m) | 15.0 | | | 15.0 | | | 15.0 | | | 15.0 | | |
| Lane Util. Factor | 1.00 | 0.95 | 0.95 | 1.00 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 1.00 | 1.00 | 1.00 |
| Frt | | 0.994 | | | 0.999 | | | 0.869 | | | 0.938 | |
| Flt Protected | 0.950 | | | 0.950 | | | | 0.997 | | | 0.981 | |
| Satd. Flow (prot) | 1745 | 3192 | 0 | 1616 | 3082 | 0 | 0 | 2737 | 0 | 0 | 1401 | 0 |
| Flt Permitted | 0.240 | | | 0.254 | | | | 0.946 | | | 0.900 | |
| Satd. Flow (perm) | 441 | 3192 | 0 | 432 | 3082 | 0 | 0 | 2597 | 0 | 0 | 1286 | 0 |
| Right Turn on Red | | | Yes | | | Yes | | | Yes | | | Yes |
| Satd. Flow (RTOR) | | 5 | | | 2 | | | 137 | | | 12 | |
| Link Speed (k/h) | | 50 | | | 50 | | | 50 | | | 50 | |
| Link Distance (m) | | 125.0 | | | 716.7 | | | 215.5 | | | 91.4 | |
| Travel Time (s) | | 9.0 | | | 51.6 | | | 15.5 | | | 6.6 | |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles (%) | 0% | 8% | 24% | 8% | 13% | 25% | 0% | 0% | 12% | 14% | 33% | 22% |
| Adj. Flow (vph) | 2 | 633 | 28 | 260 | 1166 | 11 | 8 | 12 | 137 | 10 | 4 | 12 |
| Shared Lane Traffic (%) | | | | | | | | | | | | |
| Lane Group Flow (vph) | 2 | 661 | 0 | 260 | 1177 | 0 | 0 | 157 | 0 | 0 | 26 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(m) | | 3.3 | | | 3.3 | | | 0.0 | | | 0.0 | |
| Link Offset(m) | | 0.0 | | | 0.0 | | | 0.0 | | | 0.0 | |
| Crosswalk Width(m) | | 4.9 | | | 4.9 | | | 4.9 | | | 4.9 | |
| Two way Left Turn Lane | | | | | Yes | | | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | 24 | | 14 | 24 | | 14 | 24 | | 14 | 24 | | 14 |
| Turn Type | Perm | NA | | pm+pt | NA | | Perm | NA | | Perm | NA | |
| Protected Phases | | 2 | | 1 | 6 | | | 4 | | | 8 | |
| Permitted Phases | 2 | | | 6 | | | 4 | | | 8 | | |
| Minimum Split (s) | 29.0 | 29.0 | | 9.5 | 37.2 | | 35.0 | 35.0 | | 35.0 | 35.0 | |
| Total Split (s) | 36.0 | 36.0 | | 19.0 | 55.0 | | 35.0 | 35.0 | | 35.0 | 35.0 | |
| Total Split (%) | 40.0% | 40.0% | | 21.1% | 61.1% | | 38.9% | 38.9% | | 38.9% | 38.9% | |
| Maximum Green (s) | 30.0 | 30.0 | | 15.0 | 48.8 | | 29.0 | 29.0 | | 29.0 | 29.0 | |
| Yellow Time (s) | 3.3 | 3.3 | | 3.0 | 3.3 | | 3.3 | 3.3 | | 3.3 | 3.3 | |
| All-Red Time (s) | 2.7 | 2.7 | | 1.0 | 2.9 | | 2.7 | 2.7 | | 2.7 | 2.7 | |
| Lost Time Adjust (s) | 0.0 | 0.0 | | 0.0 | 0.0 | | 0.0 | 0.0 | | 0.0 | 0.0 | |
| Total Lost Time (s) | 6.0 | 6.0 | | 4.0 | 6.2 | | 6.0 | 6.0 | | 6.0 | 6.0 | |
| Lead/Lag | Lag | Lag | | Lead | | | | | | | | |
| Lead-Lag Optimize? | Yes | Yes | | Yes | | | | | | | | |
| Walk Time (s) | 7.0 | 7.0 | | | 15.0 | | 11.0 | 11.0 | | 11.0 | 11.0 | |
| Flash Dont Walk (s) | 16.0 | 16.0 | | | 16.0 | | 18.0 | 18.0 | | 18.0 | 18.0 | |
| Pedestrian Calls (#/hr) | 0 | 0 | | | 0 | | 0 | 0 | | 0 | 0 | |
| Act Effct Green (s) | 30.0 | 30.0 | | 51.0 | 48.8 | | 29.0 | 29.0 | | 29.0 | 29.0 | |
| Actuated g/C Ratio | 0.33 | 0.33 | | 0.57 | 0.54 | | 0.32 | 0.32 | | 0.32 | 0.32 | |

Lanes, Volumes, Timings
 3: Wentworth Street N & Burlington St E/Burlington Street E

AM Peak Period
 05-16-2019



| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|----------------|------|------|-----|------|------|-----|-----|------|-----|-----|-----|------|
| v/c Ratio | 0.01 | 0.62 | | 0.59 | 0.70 | | | 0.17 | | | | 0.06 |
| Control Delay | 20.5 | 28.1 | | 15.7 | 18.0 | | | 6.0 | | | | 15.0 |
| Queue Delay | 0.0 | 0.0 | | 0.0 | 0.0 | | | 0.0 | | | | 0.0 |
| Total Delay | 20.5 | 28.1 | | 15.7 | 18.0 | | | 6.0 | | | | 15.0 |
| LOS | C | C | | B | B | | | A | | | | B |
| Approach Delay | | 28.0 | | | 17.6 | | | 6.0 | | | | 15.0 |
| Approach LOS | | C | | | B | | | A | | | | B |

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 0 (0%), Referenced to phase 2:EBTL, Start of Green
 Natural Cycle: 75
 Control Type: Pretimed
 Maximum v/c Ratio: 0.70
 Intersection Signal Delay: 19.8
 Intersection LOS: B
 Intersection Capacity Utilization 71.2%
 ICU Level of Service C
 Analysis Period (min) 15

Splits and Phases: 3: Wentworth Street N & Burlington St E/Burlington Street E



Lanes, Volumes, Timings
9: Birch Avenue & Burlington Street E

AM Peak Period
05-16-2019



| Lane Group | EBT | EBR | WBL | WBT | NBL | NBR |
|----------------------------|-------|-------|-------|-------|-------|-------|
| Lane Configurations | ↑↑↑ | | ↵↵ | ↑↑↑ | | |
| Traffic Volume (vph) | 718 | 26 | 294 | 1631 | 0 | 0 |
| Future Volume (vph) | 718 | 26 | 294 | 1631 | 0 | 0 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (m) | | 0.0 | 125.0 | | 0.0 | 0.0 |
| Storage Lanes | | 0 | 2 | | 0 | 0 |
| Taper Length (m) | | | 15.0 | | 15.0 | |
| Lane Util. Factor | 0.91 | 0.91 | 0.97 | 0.91 | 1.00 | 1.00 |
| Frt | 0.995 | | | | | |
| Flt Protected | | | 0.950 | | | |
| Satd. Flow (prot) | 4457 | 0 | 3224 | 4821 | 0 | 0 |
| Flt Permitted | | | 0.950 | | | |
| Satd. Flow (perm) | 4457 | 0 | 3224 | 4821 | 0 | 0 |
| Right Turn on Red | | Yes | | | | Yes |
| Satd. Flow (RTOR) | 11 | | | | | |
| Link Speed (k/h) | 50 | | | 50 | 50 | |
| Link Distance (m) | 716.7 | | | 130.8 | 242.7 | |
| Travel Time (s) | 51.6 | | | 9.4 | 17.5 | |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles (%) | 12% | 10% | 5% | 4% | 0% | 0% |
| Adj. Flow (vph) | 780 | 28 | 320 | 1773 | 0 | 0 |
| Shared Lane Traffic (%) | | | | | | |
| Lane Group Flow (vph) | 808 | 0 | 320 | 1773 | 0 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No |
| Lane Alignment | Left | Right | Left | Left | Left | Right |
| Median Width(m) | 6.6 | | | 6.6 | 0.0 | |
| Link Offset(m) | 0.0 | | | 0.0 | 0.0 | |
| Crosswalk Width(m) | 1.6 | | | 1.6 | 1.6 | |
| Two way Left Turn Lane | Yes | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | | 14 | 24 | | 24 | 14 |
| Turn Type | NA | | Prot | NA | | |
| Protected Phases | 2 | | 4 | 2 4 | | |
| Permitted Phases | | | | | | |
| Minimum Split (s) | 47.0 | | 15.1 | | | |
| Total Split (s) | 62.0 | | 28.0 | | | |
| Total Split (%) | 68.9% | | 31.1% | | | |
| Maximum Green (s) | 57.0 | | 22.9 | | | |
| Yellow Time (s) | 3.7 | | 3.7 | | | |
| All-Red Time (s) | 1.3 | | 1.4 | | | |
| Lost Time Adjust (s) | 0.0 | | 0.0 | | | |
| Total Lost Time (s) | 5.0 | | 5.1 | | | |
| Lead/Lag | | | | | | |
| Lead-Lag Optimize? | | | | | | |
| Walk Time (s) | 30.0 | | | | | |
| Flash Dont Walk (s) | 12.0 | | | | | |
| Pedestrian Calls (#/hr) | 0 | | | | | |
| Act Effct Green (s) | 57.0 | | 22.9 | 90.0 | | |
| Actuated g/C Ratio | 0.63 | | 0.25 | 1.00 | | |

Lanes, Volumes, Timings
 9: Birch Avenue & Burlington Street E

AM Peak Period
 05-16-2019



| Lane Group | EBT | EBR | WBL | WBT | NBL | NBR |
|----------------|------|-----|------|------|-----|-----|
| v/c Ratio | 0.29 | | 0.39 | 0.37 | | |
| Control Delay | 18.6 | | 29.5 | 0.2 | | |
| Queue Delay | 0.0 | | 0.0 | 0.0 | | |
| Total Delay | 18.6 | | 29.5 | 0.2 | | |
| LOS | B | | C | A | | |
| Approach Delay | 18.6 | | | 4.7 | | |
| Approach LOS | B | | | A | | |

Intersection Summary

| | |
|-----------------------------------|---|
| Area Type: | Other |
| Cycle Length: | 90 |
| Actuated Cycle Length: | 90 |
| Offset: | 0 (0%), Referenced to phase 2:EBWB and 6:, Start of Green |
| Natural Cycle: | 65 |
| Control Type: | Pretimed |
| Maximum v/c Ratio: | 0.39 |
| Intersection Signal Delay: | 8.6 |
| Intersection LOS: | A |
| Intersection Capacity Utilization | 35.7% |
| ICU Level of Service | A |
| Analysis Period (min) | 15 |

Splits and Phases: 9: Birch Avenue & Burlington Street E



Lanes, Volumes, Timings
13: Wentworth Street N & Brant Street

AM Peak Period
05-16-2019



| Lane Group | WBL | WBR | NBT | NBR | SBL | SBT |
|----------------------------|-------|-------|-------|-------|------|-------|
| Lane Configurations | | | | | | |
| Traffic Volume (vph) | 27 | 9 | 133 | 46 | 5 | 405 |
| Future Volume (vph) | 27 | 9 | 133 | 46 | 5 | 405 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Util. Factor | 1.00 | 1.00 | 0.95 | 0.95 | 0.95 | 0.95 |
| Frt | 0.965 | | 0.962 | | | |
| Flt Protected | 0.964 | | | | | 0.999 |
| Satd. Flow (prot) | 1499 | 0 | 3109 | 0 | 0 | 3252 |
| Flt Permitted | 0.964 | | | | | 0.999 |
| Satd. Flow (perm) | 1499 | 0 | 3109 | 0 | 0 | 3252 |
| Link Speed (k/h) | 48 | | 50 | | | 50 |
| Link Distance (m) | 104.2 | | 236.6 | | | 215.5 |
| Travel Time (s) | 7.8 | | 17.0 | | | 15.5 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles (%) | 14% | 14% | 9% | 5% | 25% | 7% |
| Adj. Flow (vph) | 29 | 10 | 145 | 50 | 5 | 440 |
| Shared Lane Traffic (%) | | | | | | |
| Lane Group Flow (vph) | 39 | 0 | 195 | 0 | 0 | 445 |
| Enter Blocked Intersection | No | No | No | No | No | No |
| Lane Alignment | Left | Right | Left | Right | Left | Left |
| Median Width(m) | 3.3 | | 0.0 | | | 0.0 |
| Link Offset(m) | 0.0 | | 0.0 | | | 0.0 |
| Crosswalk Width(m) | 1.6 | | 1.6 | | | 1.6 |
| Two way Left Turn Lane | | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | 24 | 14 | | 14 | 24 | |
| Sign Control | Stop | | Free | | | Free |

Intersection Summary

| | |
|-----------------------------------|------------------------|
| Area Type: | Other |
| Control Type: | Unsignalized |
| Intersection Capacity Utilization | 24.7% |
| Analysis Period (min) | 15 |
| | ICU Level of Service A |

Lanes, Volumes, Timings
 14: Site Access #2 & Brant Street

AM Peak Period
 05-16-2019



| Lane Group | EBT | EBR | WBL | WBT | NBL | NBR |
|----------------------------|------|-------|------|-------|------|-------|
| Lane Configurations | | | | | | |
| Traffic Volume (vph) | 61 | 0 | 0 | 54 | 0 | 0 |
| Future Volume (vph) | 61 | 0 | 0 | 54 | 0 | 0 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt | | | | | | |
| Flt Protected | | | | | | |
| Satd. Flow (prot) | 1766 | 0 | 0 | 1625 | 918 | 918 |
| Flt Permitted | | | | | | |
| Satd. Flow (perm) | 1766 | 0 | 0 | 1625 | 918 | 918 |
| Link Speed (k/h) | 48 | | | 48 | 48 | |
| Link Distance (m) | 77.1 | | | 157.8 | 61.8 | |
| Travel Time (s) | 5.8 | | | 11.8 | 4.6 | |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles (%) | 4% | 100% | 100% | 13% | 100% | 100% |
| Adj. Flow (vph) | 66 | 0 | 0 | 59 | 0 | 0 |
| Shared Lane Traffic (%) | | | | | | |
| Lane Group Flow (vph) | 66 | 0 | 0 | 59 | 0 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No |
| Lane Alignment | Left | Right | Left | Left | Left | Right |
| Median Width(m) | 0.0 | | | 0.0 | 3.3 | |
| Link Offset(m) | 0.0 | | | 0.0 | 0.0 | |
| Crosswalk Width(m) | 1.6 | | | 1.6 | 1.6 | |
| Two way Left Turn Lane | | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | | 14 | 24 | | 24 | 14 |
| Sign Control | Free | | | Free | Stop | |

Intersection Summary

| | |
|-----------------------------------|------------------------|
| Area Type: | Other |
| Control Type: | Unsignalized |
| Intersection Capacity Utilization | 13.3% |
| Analysis Period (min) | 15 |
| | ICU Level of Service A |

Lanes, Volumes, Timings
15: Brant Street & Birch Avenue

AM Peak Period
05-16-2019



| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|----------------------------|------|-------|-------|-------|-------|-------|------|------|-------|-------|-------|-------|
| Lane Configurations | | | | | | | | | | | | |
| Traffic Volume (vph) | 0 | 26 | 35 | 30 | 36 | 0 | 0 | 0 | 0 | 23 | 229 | 18 |
| Future Volume (vph) | 0 | 26 | 35 | 30 | 36 | 0 | 0 | 0 | 0 | 23 | 229 | 18 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (m) | 0.0 | | 0.0 | 0.0 | | 0.0 | 0.0 | | 0.0 | 0.0 | | 150.0 |
| Storage Lanes | 0 | | 0 | 0 | | 0 | 0 | | 0 | 0 | | 0 |
| Taper Length (m) | 15.0 | | | 15.0 | | | 15.0 | | | 15.0 | | |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.91 | 0.91 | 0.91 |
| Frt | | 0.922 | | | | | | | | | | 0.990 |
| Flt Protected | | | | | 0.978 | | | | | | | 0.996 |
| Satd. Flow (prot) | 0 | 1399 | 0 | 0 | 1650 | 0 | 0 | 0 | 0 | 0 | 4466 | 0 |
| Flt Permitted | | | | | 0.880 | | | | | | | 0.996 |
| Satd. Flow (perm) | 0 | 1399 | 0 | 0 | 1485 | 0 | 0 | 0 | 0 | 0 | 4466 | 0 |
| Right Turn on Red | | | Yes | | | Yes | | | Yes | | | Yes |
| Satd. Flow (RTOR) | | 38 | | | | | | | | | | 16 |
| Link Speed (k/h) | | 50 | | | 50 | | | 50 | | | | 50 |
| Link Distance (m) | | 157.8 | | | 106.5 | | | 75.7 | | | | 242.7 |
| Travel Time (s) | | 11.4 | | | 7.7 | | | 5.5 | | | | 17.5 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles (%) | 0% | 36% | 10% | 4% | 13% | 0% | 0% | 0% | 0% | 16% | 10% | 13% |
| Adj. Flow (vph) | 0 | 28 | 38 | 33 | 39 | 0 | 0 | 0 | 0 | 25 | 249 | 20 |
| Shared Lane Traffic (%) | | | | | | | | | | | | |
| Lane Group Flow (vph) | 0 | 66 | 0 | 0 | 72 | 0 | 0 | 0 | 0 | 0 | 294 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(m) | | 0.0 | | | 0.0 | | | 0.0 | | | | 0.0 |
| Link Offset(m) | | 0.0 | | | 0.0 | | | 0.0 | | | | 0.0 |
| Crosswalk Width(m) | | 1.6 | | | 1.6 | | | 1.6 | | | | 1.6 |
| Two way Left Turn Lane | | | | | | | | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | 24 | | 14 | 24 | | 14 | 24 | | 14 | 24 | | 14 |
| Turn Type | | NA | | Perm | NA | | | | | | Perm | NA |
| Protected Phases | | 4 | | | 8 | | | | | | | 2 |
| Permitted Phases | | | | 8 | | | | | | 2 | | |
| Minimum Split (s) | | 32.5 | | 32.5 | 32.5 | | | | | 23.4 | 23.4 | |
| Total Split (s) | | 46.0 | | 46.0 | 46.0 | | | | | 44.0 | 44.0 | |
| Total Split (%) | | 51.1% | | 51.1% | 51.1% | | | | | 48.9% | 48.9% | |
| Maximum Green (s) | | 40.5 | | 40.5 | 40.5 | | | | | 38.6 | 38.6 | |
| Yellow Time (s) | | 3.3 | | 3.3 | 3.3 | | | | | 3.3 | 3.3 | |
| All-Red Time (s) | | 2.2 | | 2.2 | 2.2 | | | | | 2.1 | 2.1 | |
| Lost Time Adjust (s) | | 0.0 | | | 0.0 | | | | | | | 0.0 |
| Total Lost Time (s) | | 5.5 | | | 5.5 | | | | | | | 5.4 |
| Lead/Lag | | | | | | | | | | | | |
| Lead-Lag Optimize? | | | | | | | | | | | | |
| Walk Time (s) | | 17.0 | | 17.0 | 17.0 | | | | | 7.0 | 7.0 | |
| Flash Dont Walk (s) | | 10.0 | | 10.0 | 10.0 | | | | | 11.0 | 11.0 | |
| Pedestrian Calls (#/hr) | | 0 | | 0 | 0 | | | | | 0 | 0 | |
| Act Effct Green (s) | | 40.5 | | | 40.5 | | | | | | | 38.6 |
| Actuated g/C Ratio | | 0.45 | | | 0.45 | | | | | | | 0.43 |

Lanes, Volumes, Timings
 15: Brant Street & Birch Avenue

AM Peak Period
 05-16-2019

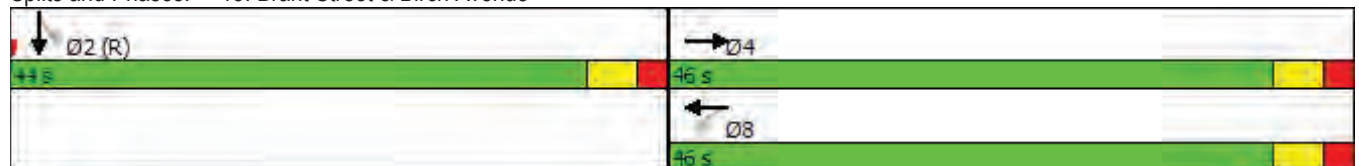


| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|----------------|-----|------|-----|-----|------|-----|-----|-----|-----|-----|-----|------|
| v/c Ratio | | 0.10 | | | 0.11 | | | | | | | 0.15 |
| Control Delay | | 8.0 | | | 14.9 | | | | | | | 7.5 |
| Queue Delay | | 0.0 | | | 0.0 | | | | | | | 0.0 |
| Total Delay | | 8.0 | | | 14.9 | | | | | | | 7.5 |
| LOS | | A | | | B | | | | | | | A |
| Approach Delay | | 8.0 | | | 14.9 | | | | | | | 7.5 |
| Approach LOS | | A | | | B | | | | | | | A |

Intersection Summary

| | |
|-----------------------------------|---|
| Area Type: | Other |
| Cycle Length: | 90 |
| Actuated Cycle Length: | 90 |
| Offset: | 0 (0%), Referenced to phase 2:SBTL and 6:, Start of Green |
| Natural Cycle: | 60 |
| Control Type: | Pretimed |
| Maximum v/c Ratio: | 0.15 |
| Intersection Signal Delay: | 8.9 |
| Intersection LOS: | A |
| Intersection Capacity Utilization | 27.6% |
| ICU Level of Service | A |
| Analysis Period (min) | 15 |

Splits and Phases: 15: Brant Street & Birch Avenue



Lanes, Volumes, Timings
25: Hillyard Street & Brant Street

AM Peak Period
05-16-2019



| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|----------------------------|------|-------|-------|------|-------|-------|------|-------|-------|------|-------|-------|
| Lane Configurations | | ↕ | | | ↕ | | | ↕ | | | ↕ | |
| Traffic Volume (vph) | 7 | 28 | 4 | 12 | 14 | 5 | 2 | 12 | 7 | 1 | 8 | 9 |
| Future Volume (vph) | 7 | 28 | 4 | 12 | 14 | 5 | 2 | 12 | 7 | 1 | 8 | 9 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt | | 0.987 | | | 0.980 | | | 0.953 | | | 0.932 | |
| Flt Protected | | 0.991 | | | 0.981 | | | 0.996 | | | 0.998 | |
| Satd. Flow (prot) | 0 | 1747 | 0 | 0 | 1586 | 0 | 0 | 1650 | 0 | 0 | 1514 | 0 |
| Flt Permitted | | 0.991 | | | 0.981 | | | 0.996 | | | 0.998 | |
| Satd. Flow (perm) | 0 | 1747 | 0 | 0 | 1586 | 0 | 0 | 1650 | 0 | 0 | 1514 | 0 |
| Link Speed (k/h) | | 48 | | | 48 | | | 48 | | | 48 | |
| Link Distance (m) | | 103.4 | | | 96.5 | | | 111.2 | | | 105.1 | |
| Travel Time (s) | | 7.8 | | | 7.2 | | | 8.3 | | | 7.9 | |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles (%) | 0% | 4% | 0% | 10% | 8% | 25% | 0% | 10% | 0% | 0% | 14% | 13% |
| Adj. Flow (vph) | 8 | 30 | 4 | 13 | 15 | 5 | 2 | 13 | 8 | 1 | 9 | 10 |
| Shared Lane Traffic (%) | | | | | | | | | | | | |
| Lane Group Flow (vph) | 0 | 42 | 0 | 0 | 33 | 0 | 0 | 23 | 0 | 0 | 20 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(m) | | 0.0 | | | 0.0 | | | 0.0 | | | 0.0 | |
| Link Offset(m) | | 0.0 | | | 0.0 | | | 0.0 | | | 0.0 | |
| Crosswalk Width(m) | | 1.6 | | | 1.6 | | | 1.6 | | | 1.6 | |
| Two way Left Turn Lane | | | | | | | | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | 24 | | 14 | 24 | | 14 | 24 | | 14 | 24 | | 14 |
| Sign Control | | Free | | | Free | | | Stop | | | Stop | |

Intersection Summary

| | |
|-----------------------------------|--------------|
| Area Type: | Other |
| Control Type: | Unsignalized |
| Intersection Capacity Utilization | 13.3% |
| ICU Level of Service | A |
| Analysis Period (min) | 15 |

Lanes, Volumes, Timings
27: Birch Avenue & Site Access #1

AM Peak Period
05-16-2019



| Lane Group | EBL | EBR | NBL | NBT | SBT | SBR |
|----------------------------|------|-------|------|-------|------|-------|
| Lane Configurations | | | | | | |
| Traffic Volume (vph) | 0 | 0 | 0 | 0 | 295 | 0 |
| Future Volume (vph) | 0 | 0 | 0 | 0 | 295 | 0 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 0.91 | 0.91 |
| Frt | | | | | | |
| Flt Protected | | | | | | |
| Satd. Flow (prot) | 1837 | 918 | 0 | 0 | 4558 | 0 |
| Flt Permitted | | | | | | |
| Satd. Flow (perm) | 1837 | 918 | 0 | 0 | 4558 | 0 |
| Link Speed (k/h) | 48 | | | 50 | 48 | |
| Link Distance (m) | 51.6 | | | 315.9 | 75.7 | |
| Travel Time (s) | 3.9 | | | 22.7 | 5.7 | |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles (%) | 0% | 100% | 0% | 0% | 10% | 100% |
| Adj. Flow (vph) | 0 | 0 | 0 | 0 | 321 | 0 |
| Shared Lane Traffic (%) | | | | | | |
| Lane Group Flow (vph) | 0 | 0 | 0 | 0 | 321 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No |
| Lane Alignment | Left | Right | Left | Left | Left | Right |
| Median Width(m) | 3.3 | | | 0.0 | 0.0 | |
| Link Offset(m) | 0.0 | | | 0.0 | 0.0 | |
| Crosswalk Width(m) | 1.6 | | | 1.6 | 1.6 | |
| Two way Left Turn Lane | | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | 24 | 14 | 24 | | | 14 |
| Sign Control | Stop | | | Free | Free | |

Intersection Summary

| | |
|-----------------------------------|------------------------|
| Area Type: | Other |
| Control Type: | Unsignalized |
| Intersection Capacity Utilization | 9.0% |
| | ICU Level of Service A |
| Analysis Period (min) | 15 |

Lanes, Volumes, Timings
31: Wentworth Street N & Munroe Street

AM Peak Period
05-16-2019












| Lane Group | WBL | WBR | NBT | NBR | SBL | SBT |
|----------------------------|-------|-------|-------|-------|------|-------|
| Lane Configurations | | | | | | |
| Traffic Volume (vph) | 28 | 9 | 174 | 35 | 2 | 340 |
| Future Volume (vph) | 28 | 9 | 174 | 35 | 2 | 340 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Util. Factor | 1.00 | 1.00 | 0.95 | 0.95 | 0.95 | 0.95 |
| Frt | 0.966 | | 0.975 | | | |
| Flt Protected | 0.964 | | | | | |
| Satd. Flow (prot) | 1462 | 0 | 3093 | 0 | 0 | 3195 |
| Flt Permitted | 0.964 | | | | | |
| Satd. Flow (perm) | 1462 | 0 | 3093 | 0 | 0 | 3195 |
| Link Speed (k/h) | 48 | | 50 | | | 50 |
| Link Distance (m) | 118.1 | | 172.4 | | | 236.6 |
| Travel Time (s) | 8.9 | | 12.4 | | | 17.0 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles (%) | 13% | 29% | 10% | 10% | 50% | 9% |
| Adj. Flow (vph) | 30 | 10 | 189 | 38 | 2 | 370 |
| Shared Lane Traffic (%) | | | | | | |
| Lane Group Flow (vph) | 40 | 0 | 227 | 0 | 0 | 372 |
| Enter Blocked Intersection | No | No | No | No | No | No |
| Lane Alignment | Left | Right | Left | Right | Left | Left |
| Median Width(m) | 3.3 | | 0.0 | | | 0.0 |
| Link Offset(m) | 0.0 | | 0.0 | | | 0.0 |
| Crosswalk Width(m) | 1.6 | | 1.6 | | | 1.6 |
| Two way Left Turn Lane | | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | 24 | 14 | | 14 | 24 | |
| Sign Control | Stop | | Free | | | Free |

Intersection Summary

| | |
|-----------------------------------|------------------------|
| Area Type: | Other |
| Control Type: | Unsignalized |
| Intersection Capacity Utilization | 20.8% |
| Analysis Period (min) | 15 |
| | ICU Level of Service A |

Lanes, Volumes, Timings
34: Hillyard Street & Site Access #3

AM Peak Period
05-16-2019

| |  |  |  |  |  |  |
|-----------------------------------|---|---|---|---|---|---|
| Lane Group | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations |  | |  | | |  |
| Traffic Volume (vph) | 0 | 0 | 35 | 0 | 0 | 36 |
| Future Volume (vph) | 0 | 0 | 35 | 0 | 0 | 36 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt | | | | | | |
| Flt Protected | | | | | | |
| Satd. Flow (prot) | 1837 | 0 | 1670 | 0 | 0 | 1670 |
| Flt Permitted | | | | | | |
| Satd. Flow (perm) | 1837 | 0 | 1670 | 0 | 0 | 1670 |
| Link Speed (k/h) | 48 | | 48 | | | 48 |
| Link Distance (m) | 48.3 | | 26.1 | | | 111.2 |
| Travel Time (s) | 3.6 | | 2.0 | | | 8.3 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles (%) | 0% | 0% | 10% | 0% | 0% | 10% |
| Adj. Flow (vph) | 0 | 0 | 38 | 0 | 0 | 39 |
| Shared Lane Traffic (%) | | | | | | |
| Lane Group Flow (vph) | 0 | 0 | 38 | 0 | 0 | 39 |
| Enter Blocked Intersection | No | No | No | No | No | No |
| Lane Alignment | Left | Right | Left | Right | Left | Left |
| Median Width(m) | 3.3 | | 0.0 | | | 0.0 |
| Link Offset(m) | 0.0 | | 0.0 | | | 0.0 |
| Crosswalk Width(m) | 1.6 | | 1.6 | | | 1.6 |
| Two way Left Turn Lane | | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | 24 | 14 | | 14 | 24 | |
| Sign Control | Stop | | Free | | | Free |
| Intersection Summary | | | | | | |
| Area Type: | Other | | | | | |
| Control Type: | Unsignalized | | | | | |
| Intersection Capacity Utilization | 13.3% | | | ICU Level of Service A | | |
| Analysis Period (min) | 15 | | | | | |

Lanes, Volumes, Timings
3: Wentworth Street N & Burlington Street E

PM Peak Period
05-16-2019



| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|----------------------------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|
| Lane Configurations | | | | | | | | | | | | |
| Traffic Volume (vph) | 11 | 703 | 36 | 173 | 715 | 11 | 11 | 7 | 117 | 11 | 14 | 9 |
| Future Volume (vph) | 11 | 703 | 36 | 173 | 715 | 11 | 11 | 7 | 117 | 11 | 14 | 9 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (m) | 60.0 | | 0.0 | 50.0 | | 0.0 | 0.0 | | 0.0 | 0.0 | | 0.0 |
| Storage Lanes | 1 | | 0 | 1 | | 0 | 0 | | 0 | 0 | | 0 |
| Taper Length (m) | 15.0 | | | 15.0 | | | 15.0 | | | 15.0 | | |
| Lane Util. Factor | 1.00 | 0.95 | 0.95 | 1.00 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 1.00 | 1.00 | 1.00 |
| Frt | | 0.993 | | | 0.998 | | | 0.870 | | | 0.964 | |
| Flt Protected | 0.950 | | | 0.950 | | | | 0.996 | | | 0.984 | |
| Satd. Flow (prot) | 1572 | 3078 | 0 | 1572 | 3161 | 0 | 0 | 2657 | 0 | 0 | 1423 | 0 |
| Flt Permitted | 0.353 | | | 0.195 | | | | 0.938 | | | 0.910 | |
| Satd. Flow (perm) | 584 | 3078 | 0 | 323 | 3161 | 0 | 0 | 2502 | 0 | 0 | 1316 | 0 |
| Right Turn on Red | | | Yes | | | Yes | | | Yes | | | Yes |
| Satd. Flow (RTOR) | | 6 | | | 3 | | | 127 | | | 10 | |
| Link Speed (k/h) | | 50 | | | 50 | | | 50 | | | 50 | |
| Link Distance (m) | | 125.0 | | | 716.7 | | | 215.5 | | | 91.4 | |
| Travel Time (s) | | 9.0 | | | 51.6 | | | 15.5 | | | 6.6 | |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles (%) | 11% | 12% | 24% | 11% | 10% | 22% | 0% | 0% | 16% | 22% | 11% | 40% |
| Adj. Flow (vph) | 12 | 764 | 39 | 188 | 777 | 12 | 12 | 8 | 127 | 12 | 15 | 10 |
| Shared Lane Traffic (%) | | | | | | | | | | | | |
| Lane Group Flow (vph) | 12 | 803 | 0 | 188 | 789 | 0 | 0 | 147 | 0 | 0 | 37 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(m) | | 3.3 | | | 3.3 | | | 0.0 | | | 0.0 | |
| Link Offset(m) | | 0.0 | | | 0.0 | | | 0.0 | | | 0.0 | |
| Crosswalk Width(m) | | 4.9 | | | 4.9 | | | 4.9 | | | 4.9 | |
| Two way Left Turn Lane | | | | | Yes | | | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | 24 | | 14 | 24 | | 14 | 24 | | 14 | 24 | | 14 |
| Turn Type | Perm | NA | | pm+pt | NA | | Perm | NA | | Perm | NA | |
| Protected Phases | | 2 | | 1 | 6 | | | 4 | | | 8 | |
| Permitted Phases | 2 | | | 6 | | | 4 | | | 8 | | |
| Minimum Split (s) | 29.0 | 29.0 | | 9.5 | 37.2 | | 35.0 | 35.0 | | 35.0 | 35.0 | |
| Total Split (s) | 38.0 | 38.0 | | 17.0 | 55.0 | | 35.0 | 35.0 | | 35.0 | 35.0 | |
| Total Split (%) | 42.2% | 42.2% | | 18.9% | 61.1% | | 38.9% | 38.9% | | 38.9% | 38.9% | |
| Maximum Green (s) | 32.0 | 32.0 | | 13.0 | 48.8 | | 29.0 | 29.0 | | 29.0 | 29.0 | |
| Yellow Time (s) | 3.3 | 3.3 | | 3.0 | 3.3 | | 3.3 | 3.3 | | 3.3 | 3.3 | |
| All-Red Time (s) | 2.7 | 2.7 | | 1.0 | 2.9 | | 2.7 | 2.7 | | 2.7 | 2.7 | |
| Lost Time Adjust (s) | 0.0 | 0.0 | | 0.0 | 0.0 | | 0.0 | 0.0 | | 0.0 | 0.0 | |
| Total Lost Time (s) | 6.0 | 6.0 | | 4.0 | 6.2 | | 6.0 | 6.0 | | 6.0 | 6.0 | |
| Lead/Lag | Lag | Lag | | Lead | | | | | | | | |
| Lead-Lag Optimize? | Yes | Yes | | Yes | | | | | | | | |
| Walk Time (s) | 7.0 | 7.0 | | | 15.0 | | 11.0 | 11.0 | | 11.0 | 11.0 | |
| Flash Dont Walk (s) | 16.0 | 16.0 | | | 16.0 | | 18.0 | 18.0 | | 18.0 | 18.0 | |
| Pedestrian Calls (#/hr) | 0 | 0 | | | 0 | | 0 | 0 | | 0 | 0 | |
| Act Effct Green (s) | 32.0 | 32.0 | | 51.0 | 48.8 | | 29.0 | 29.0 | | 29.0 | 29.0 | |
| Actuated g/C Ratio | 0.36 | 0.36 | | 0.57 | 0.54 | | 0.32 | 0.32 | | 0.32 | 0.32 | |

Lanes, Volumes, Timings
3: Wentworth Street N & Burlington Street E

PM Peak Period
05-16-2019

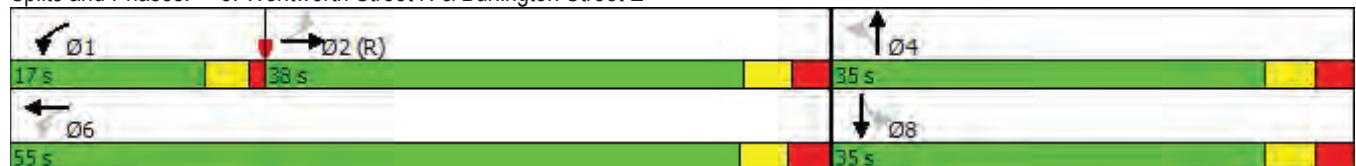


| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|----------------|------|------|-----|------|------|-----|-----|------|-----|-----|-----|------|
| v/c Ratio | 0.06 | 0.73 | | 0.52 | 0.46 | | | 0.16 | | | | 0.09 |
| Control Delay | 20.3 | 29.8 | | 14.8 | 13.6 | | | 6.2 | | | | 17.6 |
| Queue Delay | 0.0 | 0.0 | | 0.0 | 0.0 | | | 0.0 | | | | 0.0 |
| Total Delay | 20.3 | 29.8 | | 14.8 | 13.6 | | | 6.2 | | | | 17.6 |
| LOS | C | C | | B | B | | | A | | | | B |
| Approach Delay | | 29.7 | | | 13.8 | | | 6.2 | | | | 17.6 |
| Approach LOS | | C | | | B | | | A | | | | B |

Intersection Summary

| | |
|-----------------------------------|--|
| Area Type: | Other |
| Cycle Length: | 90 |
| Actuated Cycle Length: | 90 |
| Offset: | 0 (0%), Referenced to phase 2:EBTL, Start of Green |
| Natural Cycle: | 75 |
| Control Type: | Pretimed |
| Maximum v/c Ratio: | 0.73 |
| Intersection Signal Delay: | 19.9 |
| Intersection LOS: | B |
| Intersection Capacity Utilization | 63.3% |
| ICU Level of Service | B |
| Analysis Period (min) | 15 |

Splits and Phases: 3: Wentworth Street N & Burlington Street E



Lanes, Volumes, Timings
9: Birch Avenue & Burlington Street E

PM Peak Period
05-16-2019



| Lane Group | EBT | EBR | WBL | WBT | NBL | NBR |
|----------------------------|-------|-------|-------|-------|-------|-------|
| Lane Configurations | ↑↑↑ | | ↵↵ | ↑↑↑ | | |
| Traffic Volume (vph) | 1238 | 31 | 277 | 1353 | 0 | 0 |
| Future Volume (vph) | 1238 | 31 | 277 | 1353 | 0 | 0 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (m) | | 0.0 | 125.0 | | 0.0 | 0.0 |
| Storage Lanes | | 0 | 2 | | 0 | 0 |
| Taper Length (m) | | | 15.0 | | 15.0 | |
| Lane Util. Factor | 0.91 | 0.91 | 0.97 | 0.91 | 1.00 | 1.00 |
| Frt | 0.996 | | | | | |
| Flt Protected | | | 0.950 | | | |
| Satd. Flow (prot) | 4718 | 0 | 3134 | 4643 | 0 | 0 |
| Flt Permitted | | | 0.950 | | | |
| Satd. Flow (perm) | 4718 | 0 | 3134 | 4643 | 0 | 0 |
| Right Turn on Red | | Yes | | | | Yes |
| Satd. Flow (RTOR) | 8 | | | | | |
| Link Speed (k/h) | 50 | | | 50 | 50 | |
| Link Distance (m) | 716.7 | | | 130.8 | 242.7 | |
| Travel Time (s) | 51.6 | | | 9.4 | 17.5 | |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles (%) | 6% | 0% | 8% | 8% | 0% | 0% |
| Adj. Flow (vph) | 1346 | 34 | 301 | 1471 | 0 | 0 |
| Shared Lane Traffic (%) | | | | | | |
| Lane Group Flow (vph) | 1380 | 0 | 301 | 1471 | 0 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No |
| Lane Alignment | Left | Right | Left | Left | Left | Right |
| Median Width(m) | 6.6 | | | 6.6 | 0.0 | |
| Link Offset(m) | 0.0 | | | 0.0 | 0.0 | |
| Crosswalk Width(m) | 1.6 | | | 1.6 | 1.6 | |
| Two way Left Turn Lane | Yes | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | | 14 | 24 | | 24 | 14 |
| Turn Type | NA | | Prot | NA | | |
| Protected Phases | 2 | | 4 | 2 4 | | |
| Permitted Phases | | | | | | |
| Minimum Split (s) | 47.0 | | 15.1 | | | |
| Total Split (s) | 64.0 | | 26.0 | | | |
| Total Split (%) | 71.1% | | 28.9% | | | |
| Maximum Green (s) | 59.0 | | 20.9 | | | |
| Yellow Time (s) | 3.7 | | 3.7 | | | |
| All-Red Time (s) | 1.3 | | 1.4 | | | |
| Lost Time Adjust (s) | 0.0 | | 0.0 | | | |
| Total Lost Time (s) | 5.0 | | 5.1 | | | |
| Lead/Lag | | | | | | |
| Lead-Lag Optimize? | | | | | | |
| Walk Time (s) | 30.0 | | | | | |
| Flash Dont Walk (s) | 12.0 | | | | | |
| Pedestrian Calls (#/hr) | 0 | | | | | |
| Act Effct Green (s) | 59.0 | | 20.9 | 90.0 | | |
| Actuated g/C Ratio | 0.66 | | 0.23 | 1.00 | | |

Lanes, Volumes, Timings
 9: Birch Avenue & Burlington Street E

PM Peak Period
 05-16-2019



| Lane Group | EBT | EBR | WBL | WBT | NBL | NBR |
|----------------|------|-----|------|------|-----|-----|
| v/c Ratio | 0.45 | | 0.41 | 0.32 | | |
| Control Delay | 15.3 | | 31.4 | 0.2 | | |
| Queue Delay | 0.0 | | 0.0 | 0.0 | | |
| Total Delay | 15.3 | | 31.4 | 0.2 | | |
| LOS | B | | C | A | | |
| Approach Delay | 15.3 | | | 5.5 | | |
| Approach LOS | B | | | A | | |

Intersection Summary

| | |
|-----------------------------------|---|
| Area Type: | Other |
| Cycle Length: | 90 |
| Actuated Cycle Length: | 90 |
| Offset: | 0 (0%), Referenced to phase 2:EBWB and 6:, Start of Green |
| Natural Cycle: | 65 |
| Control Type: | Pretimed |
| Maximum v/c Ratio: | 0.45 |
| Intersection Signal Delay: | 9.8 |
| Intersection Capacity Utilization | 41.4% |
| Analysis Period (min) | 15 |
| Intersection LOS: | A |
| ICU Level of Service | A |

Splits and Phases: 9: Birch Avenue & Burlington Street E



Lanes, Volumes, Timings
13: Brant Street & Wentworth Street N

PM Peak Period
05-16-2019



| Lane Group | WBL | WBR | NBT | NBR | SBL | SBT |
|----------------------------|-------|-------|-------|-------|------|-------|
| Lane Configurations | | | | | | |
| Traffic Volume (vph) | 34 | 4 | 148 | 27 | 11 | 261 |
| Future Volume (vph) | 34 | 4 | 148 | 27 | 11 | 261 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Util. Factor | 1.00 | 1.00 | 0.95 | 0.95 | 0.95 | 0.95 |
| Frt | 0.987 | | 0.977 | | | |
| Flt Protected | 0.957 | | | | | 0.998 |
| Satd. Flow (prot) | 1632 | 0 | 3195 | 0 | 0 | 3236 |
| Flt Permitted | 0.957 | | | | | 0.998 |
| Satd. Flow (perm) | 1632 | 0 | 3195 | 0 | 0 | 3236 |
| Link Speed (k/h) | 48 | | 48 | | | 48 |
| Link Distance (m) | 104.2 | | 236.6 | | | 215.5 |
| Travel Time (s) | 7.8 | | 17.7 | | | 16.2 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles (%) | 7% | 0% | 7% | 5% | 22% | 7% |
| Adj. Flow (vph) | 37 | 4 | 161 | 29 | 12 | 284 |
| Shared Lane Traffic (%) | | | | | | |
| Lane Group Flow (vph) | 41 | 0 | 190 | 0 | 0 | 296 |
| Enter Blocked Intersection | No | No | No | No | No | No |
| Lane Alignment | Left | Right | Left | Right | Left | Left |
| Median Width(m) | 3.3 | | 0.0 | | | 0.0 |
| Link Offset(m) | 0.0 | | 0.0 | | | 0.0 |
| Crosswalk Width(m) | 1.6 | | 1.6 | | | 1.6 |
| Two way Left Turn Lane | | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | 24 | 14 | | 14 | 24 | |
| Sign Control | Stop | | Free | | | Free |

Intersection Summary

| | |
|-----------------------------------|------------------------|
| Area Type: | Other |
| Control Type: | Unsignalized |
| Intersection Capacity Utilization | 25.3% |
| Analysis Period (min) | 15 |
| | ICU Level of Service A |

Lanes, Volumes, Timings
14: Site Access #2 & Brant Street

PM Peak Period
05-16-2019



| Lane Group | EBT | EBR | WBL | WBT | NBL | NBR |
|----------------------------|------|-------|------|-------|------|-------|
| Lane Configurations | | | | | | |
| Traffic Volume (vph) | 60 | 0 | 0 | 46 | 0 | 0 |
| Future Volume (vph) | 60 | 0 | 0 | 46 | 0 | 0 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt | | | | | | |
| Flt Protected | | | | | | |
| Satd. Flow (prot) | 1655 | 0 | 0 | 1597 | 918 | 918 |
| Flt Permitted | | | | | | |
| Satd. Flow (perm) | 1655 | 0 | 0 | 1597 | 918 | 918 |
| Link Speed (k/h) | 48 | | | 48 | 48 | |
| Link Distance (m) | 77.1 | | | 157.8 | 61.8 | |
| Travel Time (s) | 5.8 | | | 11.8 | 4.6 | |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles (%) | 11% | 100% | 100% | 15% | 100% | 100% |
| Adj. Flow (vph) | 65 | 0 | 0 | 50 | 0 | 0 |
| Shared Lane Traffic (%) | | | | | | |
| Lane Group Flow (vph) | 65 | 0 | 0 | 50 | 0 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No |
| Lane Alignment | Left | Right | Left | Left | Left | Right |
| Median Width(m) | 0.0 | | | 0.0 | 3.3 | |
| Link Offset(m) | 0.0 | | | 0.0 | 0.0 | |
| Crosswalk Width(m) | 1.6 | | | 1.6 | 1.6 | |
| Two way Left Turn Lane | | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | | 14 | 24 | | 24 | 14 |
| Sign Control | Free | | | Free | Stop | |

Intersection Summary

| | |
|-----------------------------------|------------------------|
| Area Type: | Other |
| Control Type: | Unsignalized |
| Intersection Capacity Utilization | 13.3% |
| Analysis Period (min) | 15 |
| | ICU Level of Service A |

Lanes, Volumes, Timings
15: Brant Street & Birch Avenue

PM Peak Period
05-16-2019



| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|----------------------------|------|-------|-------|-------|-------|-------|------|------|-------|-------|-------|-------|
| Lane Configurations | | | | | | | | | | | | |
| Traffic Volume (vph) | 0 | 14 | 45 | 61 | 33 | 0 | 0 | 0 | 0 | 8 | 239 | 12 |
| Future Volume (vph) | 0 | 14 | 45 | 61 | 33 | 0 | 0 | 0 | 0 | 8 | 239 | 12 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (m) | 0.0 | | 0.0 | 0.0 | | 0.0 | 0.0 | | 0.0 | 0.0 | | 150.0 |
| Storage Lanes | 0 | | 0 | 0 | | 0 | 0 | | 0 | 0 | | 0 |
| Taper Length (m) | 15.0 | | | 15.0 | | | 15.0 | | | 15.0 | | |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.91 | 0.91 | 0.91 |
| Frt | | 0.897 | | | | | | | | | | 0.993 |
| Flt Protected | | | | | 0.969 | | | | | | | 0.998 |
| Satd. Flow (prot) | 0 | 1559 | 0 | 0 | 1692 | 0 | 0 | 0 | 0 | 0 | 4750 | 0 |
| Flt Permitted | | | | | 0.809 | | | | | | | 0.998 |
| Satd. Flow (perm) | 0 | 1559 | 0 | 0 | 1413 | 0 | 0 | 0 | 0 | 0 | 4750 | 0 |
| Right Turn on Red | | | Yes | | | Yes | | | Yes | | | Yes |
| Satd. Flow (RTOR) | | 49 | | | | | | | | | | 9 |
| Link Speed (k/h) | | 50 | | | 50 | | | 50 | | | | 50 |
| Link Distance (m) | | 157.8 | | | 106.5 | | | 75.7 | | | | 242.7 |
| Travel Time (s) | | 11.4 | | | 7.7 | | | 5.5 | | | | 17.5 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles (%) | 0% | 8% | 5% | 2% | 11% | 2% | 0% | 0% | 0% | 29% | 3% | 20% |
| Adj. Flow (vph) | 0 | 15 | 49 | 66 | 36 | 0 | 0 | 0 | 0 | 9 | 260 | 13 |
| Shared Lane Traffic (%) | | | | | | | | | | | | |
| Lane Group Flow (vph) | 0 | 64 | 0 | 0 | 102 | 0 | 0 | 0 | 0 | 0 | 282 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(m) | | 0.0 | | | 0.0 | | | 0.0 | | | | 0.0 |
| Link Offset(m) | | 0.0 | | | 0.0 | | | 0.0 | | | | 0.0 |
| Crosswalk Width(m) | | 1.6 | | | 1.6 | | | 1.6 | | | | 1.6 |
| Two way Left Turn Lane | | | | | | | | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | 24 | | 14 | 24 | | 14 | 24 | | 14 | 24 | | 14 |
| Turn Type | | NA | | Perm | NA | | | | | | Perm | NA |
| Protected Phases | | 4 | | | 8 | | | | | | | 2 |
| Permitted Phases | | | | 8 | | | | | | 2 | | |
| Minimum Split (s) | | 32.5 | | 32.5 | 32.5 | | | | | 23.4 | 23.4 | |
| Total Split (s) | | 50.0 | | 50.0 | 50.0 | | | | | 40.0 | 40.0 | |
| Total Split (%) | | 55.6% | | 55.6% | 55.6% | | | | | 44.4% | 44.4% | |
| Maximum Green (s) | | 44.5 | | 44.5 | 44.5 | | | | | 34.6 | 34.6 | |
| Yellow Time (s) | | 3.3 | | 3.3 | 3.3 | | | | | 3.3 | 3.3 | |
| All-Red Time (s) | | 2.2 | | 2.2 | 2.2 | | | | | 2.1 | 2.1 | |
| Lost Time Adjust (s) | | 0.0 | | | 0.0 | | | | | | | 0.0 |
| Total Lost Time (s) | | 5.5 | | | 5.5 | | | | | | | 5.4 |
| Lead/Lag | | | | | | | | | | | | |
| Lead-Lag Optimize? | | | | | | | | | | | | |
| Walk Time (s) | | 17.0 | | 17.0 | 17.0 | | | | | 7.0 | 7.0 | |
| Flash Dont Walk (s) | | 10.0 | | 10.0 | 10.0 | | | | | 11.0 | 11.0 | |
| Pedestrian Calls (#/hr) | | 0 | | 0 | 0 | | | | | 0 | 0 | |
| Act Effct Green (s) | | 44.5 | | | 44.5 | | | | | | | 34.6 |
| Actuated g/C Ratio | | 0.49 | | | 0.49 | | | | | | | 0.38 |

Lanes, Volumes, Timings
15: Brant Street & Birch Avenue

PM Peak Period
05-16-2019

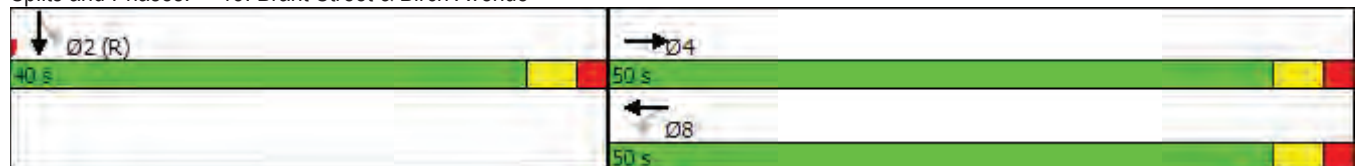


| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|----------------|-----|------|-----|-----|------|-----|-----|-----|-----|-----|-----|------|
| v/c Ratio | | 0.08 | | | 0.15 | | | | | | | 0.15 |
| Control Delay | | 5.3 | | | 13.1 | | | | | | | 6.3 |
| Queue Delay | | 0.0 | | | 0.0 | | | | | | | 0.0 |
| Total Delay | | 5.3 | | | 13.1 | | | | | | | 6.3 |
| LOS | | A | | | B | | | | | | | A |
| Approach Delay | | 5.3 | | | 13.1 | | | | | | | 6.3 |
| Approach LOS | | A | | | B | | | | | | | A |

Intersection Summary

| | |
|-----------------------------------|---|
| Area Type: | Other |
| Cycle Length: | 90 |
| Actuated Cycle Length: | 90 |
| Offset: | 0 (0%), Referenced to phase 2:SBTL and 6:, Start of Green |
| Natural Cycle: | 60 |
| Control Type: | Pretimed |
| Maximum v/c Ratio: | 0.15 |
| Intersection Signal Delay: | 7.7 |
| Intersection LOS: | A |
| Intersection Capacity Utilization | 29.2% |
| ICU Level of Service | A |
| Analysis Period (min) | 15 |

Splits and Phases: 15: Brant Street & Birch Avenue



Lanes, Volumes, Timings
25: Hillyard Street & Brant Street

PM Peak Period
05-16-2019



| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|----------------------------|------|-------|-------|------|-------|-------|------|-------|-------|------|-------|-------|
| Lane Configurations | | ↕ | | | ↕ | | | ↕ | | | ↕ | |
| Traffic Volume (vph) | 1 | 22 | 1 | 5 | 26 | 0 | 4 | 6 | 8 | 8 | 5 | 15 |
| Future Volume (vph) | 1 | 22 | 1 | 5 | 26 | 0 | 4 | 6 | 8 | 8 | 5 | 15 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Fr _t | | 0.995 | | | | | | 0.939 | | | 0.928 | |
| Fl _t Protected | | 0.998 | | | 0.992 | | | 0.990 | | | 0.985 | |
| Satd. Flow (prot) | 0 | 1656 | 0 | 0 | 1822 | 0 | 0 | 1606 | 0 | 0 | 1679 | 0 |
| Fl _t Permitted | | 0.998 | | | 0.992 | | | 0.990 | | | 0.985 | |
| Satd. Flow (perm) | 0 | 1656 | 0 | 0 | 1822 | 0 | 0 | 1606 | 0 | 0 | 1679 | 0 |
| Link Speed (k/h) | | 48 | | | 48 | | | 48 | | | 48 | |
| Link Distance (m) | | 103.4 | | | 96.5 | | | 111.2 | | | 105.1 | |
| Travel Time (s) | | 7.8 | | | 7.2 | | | 8.3 | | | 7.9 | |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles (%) | 0% | 11% | 0% | 0% | 0% | 0% | 0% | 0% | 14% | 0% | 0% | 0% |
| Adj. Flow (vph) | 1 | 24 | 1 | 5 | 28 | 0 | 4 | 7 | 9 | 9 | 5 | 16 |
| Shared Lane Traffic (%) | | | | | | | | | | | | |
| Lane Group Flow (vph) | 0 | 26 | 0 | 0 | 33 | 0 | 0 | 20 | 0 | 0 | 30 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(m) | | 0.0 | | | 0.0 | | | 0.0 | | | 0.0 | |
| Link Offset(m) | | 0.0 | | | 0.0 | | | 0.0 | | | 0.0 | |
| Crosswalk Width(m) | | 1.6 | | | 1.6 | | | 1.6 | | | 1.6 | |
| Two way Left Turn Lane | | | | | | | | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | 24 | | 14 | 24 | | 14 | 24 | | 14 | 24 | | 14 |
| Sign Control | | Free | | | Free | | | Stop | | | Stop | |

Intersection Summary

| | |
|-----------------------------------|--------------|
| Area Type: | Other |
| Control Type: | Unsignalized |
| Intersection Capacity Utilization | 13.4% |
| ICU Level of Service | A |
| Analysis Period (min) | 15 |

Lanes, Volumes, Timings
27: Birch Avenue & Site Access #1

PM Peak Period
05-16-2019



| Lane Group | EBL | EBR | NBL | NBT | SBT | SBR |
|----------------------------|------|-------|------|-------|------|-------|
| Lane Configurations | | | | | | |
| Traffic Volume (vph) | 0 | 0 | 0 | 0 | 346 | 0 |
| Future Volume (vph) | 0 | 0 | 0 | 0 | 346 | 0 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 0.91 | 0.91 |
| Frt | | | | | | |
| Flt Protected | | | | | | |
| Satd. Flow (prot) | 1837 | 918 | 0 | 0 | 4868 | 0 |
| Flt Permitted | | | | | | |
| Satd. Flow (perm) | 1837 | 918 | 0 | 0 | 4868 | 0 |
| Link Speed (k/h) | 48 | | | 50 | 48 | |
| Link Distance (m) | 49.2 | | | 315.9 | 75.7 | |
| Travel Time (s) | 3.7 | | | 22.7 | 5.7 | |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles (%) | 0% | 100% | 0% | 0% | 3% | 100% |
| Adj. Flow (vph) | 0 | 0 | 0 | 0 | 376 | 0 |
| Shared Lane Traffic (%) | | | | | | |
| Lane Group Flow (vph) | 0 | 0 | 0 | 0 | 376 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No |
| Lane Alignment | Left | Right | Left | Left | Left | Right |
| Median Width(m) | 3.3 | | | 0.0 | 0.0 | |
| Link Offset(m) | 0.0 | | | 0.0 | 0.0 | |
| Crosswalk Width(m) | 1.6 | | | 1.6 | 1.6 | |
| Two way Left Turn Lane | | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | 24 | 14 | 24 | | | 14 |
| Sign Control | Stop | | | Free | Free | |

Intersection Summary

| | |
|-----------------------------------|------------------------|
| Area Type: | Other |
| Control Type: | Unsignalized |
| Intersection Capacity Utilization | 10.0% |
| Analysis Period (min) | 15 |
| | ICU Level of Service A |

Lanes, Volumes, Timings
31: Wentworth Street N & Munroe Street

PM Peak Period
05-16-2019



| Lane Group | WBL | WBR | NBT | NBR | SBL | SBT |
|----------------------------|-------|-------|-------|-------|------|-------|
| Lane Configurations | | | | | | |
| Traffic Volume (vph) | 46 | 1 | 205 | 35 | 1 | 319 |
| Future Volume (vph) | 46 | 1 | 205 | 35 | 1 | 319 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Util. Factor | 1.00 | 1.00 | 0.95 | 0.95 | 0.95 | 0.95 |
| Frt | 0.997 | | 0.978 | | | |
| Flt Protected | 0.953 | | | | | |
| Satd. Flow (prot) | 1664 | 0 | 3177 | 0 | 0 | 3232 |
| Flt Permitted | 0.953 | | | | | |
| Satd. Flow (perm) | 1664 | 0 | 3177 | 0 | 0 | 3232 |
| Link Speed (k/h) | 48 | | 48 | | | 48 |
| Link Distance (m) | 118.1 | | 172.4 | | | 236.6 |
| Travel Time (s) | 8.9 | | 12.9 | | | 17.7 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles (%) | 5% | 0% | 7% | 10% | 0% | 8% |
| Adj. Flow (vph) | 50 | 1 | 223 | 38 | 1 | 347 |
| Shared Lane Traffic (%) | | | | | | |
| Lane Group Flow (vph) | 51 | 0 | 261 | 0 | 0 | 348 |
| Enter Blocked Intersection | No | No | No | No | No | No |
| Lane Alignment | Left | Right | Left | Right | Left | Left |
| Median Width(m) | 3.3 | | 0.0 | | | 0.0 |
| Link Offset(m) | 0.0 | | 0.0 | | | 0.0 |
| Crosswalk Width(m) | 1.6 | | 1.6 | | | 1.6 |
| Two way Left Turn Lane | | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | 24 | 14 | | 14 | 24 | |
| Sign Control | Stop | | Free | | | Free |

Intersection Summary

| | |
|-----------------------------------|------------------------|
| Area Type: | Other |
| Control Type: | Unsignalized |
| Intersection Capacity Utilization | 19.5% |
| Analysis Period (min) | 15 |
| | ICU Level of Service A |

Lanes, Volumes, Timings
34: Hillyard Street & Site Access #3

PM Peak Period
05-16-2019



| Lane Group | WBL | WBR | NBT | NBR | SBL | SBT |
|----------------------------|------|-------|------|-------|------|-------|
| Lane Configurations | | | | | | |
| Traffic Volume (vph) | 0 | 0 | 35 | 0 | 0 | 48 |
| Future Volume (vph) | 0 | 0 | 35 | 0 | 0 | 48 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt | | | | | | |
| Flt Protected | | | | | | |
| Satd. Flow (prot) | 1837 | 0 | 1670 | 0 | 0 | 1670 |
| Flt Permitted | | | | | | |
| Satd. Flow (perm) | 1837 | 0 | 1670 | 0 | 0 | 1670 |
| Link Speed (k/h) | 48 | | 48 | | | 48 |
| Link Distance (m) | 48.3 | | 26.1 | | | 111.2 |
| Travel Time (s) | 3.6 | | 2.0 | | | 8.3 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles (%) | 0% | 0% | 10% | 0% | 0% | 10% |
| Adj. Flow (vph) | 0 | 0 | 38 | 0 | 0 | 52 |
| Shared Lane Traffic (%) | | | | | | |
| Lane Group Flow (vph) | 0 | 0 | 38 | 0 | 0 | 52 |
| Enter Blocked Intersection | No | No | No | No | No | No |
| Lane Alignment | Left | Right | Left | Right | Left | Left |
| Median Width(m) | 3.3 | | 0.0 | | | 0.0 |
| Link Offset(m) | 0.0 | | 0.0 | | | 0.0 |
| Crosswalk Width(m) | 1.6 | | 1.6 | | | 1.6 |
| Two way Left Turn Lane | | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | 24 | 14 | | 14 | 24 | |
| Sign Control | Stop | | Free | | | Free |

Intersection Summary

| | |
|-----------------------------------|------------------------|
| Area Type: | Other |
| Control Type: | Unsignalized |
| Intersection Capacity Utilization | 13.3% |
| Analysis Period (min) | 15 |
| | ICU Level of Service A |

APPENDIX F

Synchro Outputs – Future Total 2027 Conditions

Lanes, Volumes, Timings
 3: Wentworth Street N & Burlington St E/Burlington Street E

AM Peak Period
 06-07-2019



| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|----------------------------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|
| Lane Configurations | | | | | | | | | | | | |
| Traffic Volume (vph) | 2 | 582 | 31 | 239 | 1074 | 10 | 8 | 11 | 126 | 9 | 4 | 11 |
| Future Volume (vph) | 2 | 582 | 31 | 239 | 1074 | 10 | 8 | 11 | 126 | 9 | 4 | 11 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (m) | 60.0 | | 0.0 | 50.0 | | 0.0 | 0.0 | | 0.0 | 0.0 | | 0.0 |
| Storage Lanes | 1 | | 0 | 1 | | 0 | 0 | | 0 | 0 | | 0 |
| Taper Length (m) | 15.0 | | | 15.0 | | | 15.0 | | | 15.0 | | |
| Lane Util. Factor | 1.00 | 0.95 | 0.95 | 1.00 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 1.00 | 1.00 | 1.00 |
| Frt | | 0.992 | | | 0.999 | | | 0.870 | | | 0.938 | |
| Flt Protected | 0.950 | | | 0.950 | | | | 0.997 | | | 0.981 | |
| Satd. Flow (prot) | 1745 | 3181 | 0 | 1616 | 3082 | 0 | 0 | 2742 | 0 | 0 | 1401 | 0 |
| Flt Permitted | 0.240 | | | 0.250 | | | | 0.945 | | | 0.899 | |
| Satd. Flow (perm) | 441 | 3181 | 0 | 425 | 3082 | 0 | 0 | 2599 | 0 | 0 | 1284 | 0 |
| Right Turn on Red | | | Yes | | | Yes | | | Yes | | | Yes |
| Satd. Flow (RTOR) | | 6 | | | 2 | | | 137 | | | 12 | |
| Link Speed (k/h) | | 50 | | | 50 | | | 50 | | | 50 | |
| Link Distance (m) | | 125.0 | | | 716.7 | | | 215.5 | | | 91.4 | |
| Travel Time (s) | | 9.0 | | | 51.6 | | | 15.5 | | | 6.6 | |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles (%) | 0% | 8% | 24% | 8% | 13% | 25% | 0% | 0% | 12% | 14% | 33% | 22% |
| Adj. Flow (vph) | 2 | 633 | 34 | 260 | 1167 | 11 | 9 | 12 | 137 | 10 | 4 | 12 |
| Shared Lane Traffic (%) | | | | | | | | | | | | |
| Lane Group Flow (vph) | 2 | 667 | 0 | 260 | 1178 | 0 | 0 | 158 | 0 | 0 | 26 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(m) | | 3.3 | | | 3.3 | | | 0.0 | | | 0.0 | |
| Link Offset(m) | | 0.0 | | | 0.0 | | | 0.0 | | | 0.0 | |
| Crosswalk Width(m) | | 4.9 | | | 4.9 | | | 4.9 | | | 4.9 | |
| Two way Left Turn Lane | | | | | Yes | | | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | 24 | | 14 | 24 | | 14 | 24 | | 14 | 24 | | 14 |
| Turn Type | Perm | NA | | pm+pt | NA | | Perm | NA | | Perm | NA | |
| Protected Phases | | 2 | | 1 | 6 | | | 4 | | | 8 | |
| Permitted Phases | 2 | | | 6 | | | 4 | | | 8 | | |
| Minimum Split (s) | 29.0 | 29.0 | | 9.5 | 37.2 | | 35.0 | 35.0 | | 35.0 | 35.0 | |
| Total Split (s) | 36.0 | 36.0 | | 19.0 | 55.0 | | 35.0 | 35.0 | | 35.0 | 35.0 | |
| Total Split (%) | 40.0% | 40.0% | | 21.1% | 61.1% | | 38.9% | 38.9% | | 38.9% | 38.9% | |
| Maximum Green (s) | 30.0 | 30.0 | | 15.0 | 48.8 | | 29.0 | 29.0 | | 29.0 | 29.0 | |
| Yellow Time (s) | 3.3 | 3.3 | | 3.0 | 3.3 | | 3.3 | 3.3 | | 3.3 | 3.3 | |
| All-Red Time (s) | 2.7 | 2.7 | | 1.0 | 2.9 | | 2.7 | 2.7 | | 2.7 | 2.7 | |
| Lost Time Adjust (s) | 0.0 | 0.0 | | 0.0 | 0.0 | | 0.0 | 0.0 | | 0.0 | 0.0 | |
| Total Lost Time (s) | 6.0 | 6.0 | | 4.0 | 6.2 | | 6.0 | 6.0 | | 6.0 | 6.0 | |
| Lead/Lag | Lag | Lag | | Lead | | | | | | | | |
| Lead-Lag Optimize? | Yes | Yes | | Yes | | | | | | | | |
| Walk Time (s) | 7.0 | 7.0 | | | 15.0 | | 11.0 | 11.0 | | 11.0 | 11.0 | |
| Flash Dont Walk (s) | 16.0 | 16.0 | | | 16.0 | | 18.0 | 18.0 | | 18.0 | 18.0 | |
| Pedestrian Calls (#/hr) | 0 | 0 | | | 0 | | 0 | 0 | | 0 | 0 | |
| Act Effct Green (s) | 30.0 | 30.0 | | 51.0 | 48.8 | | 29.0 | 29.0 | | 29.0 | 29.0 | |
| Actuated g/C Ratio | 0.33 | 0.33 | | 0.57 | 0.54 | | 0.32 | 0.32 | | 0.32 | 0.32 | |

Lanes, Volumes, Timings
 3: Wentworth Street N & Burlington St E/Burlington Street E

AM Peak Period
 06-07-2019



| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|------------------------|------|-------|-----|------|-------|-----|-----|-------|-----|-----|-----|------|
| v/c Ratio | 0.01 | 0.63 | | 0.59 | 0.70 | | | 0.17 | | | | 0.06 |
| Control Delay | 20.5 | 28.2 | | 15.8 | 18.0 | | | 6.0 | | | | 15.0 |
| Queue Delay | 0.0 | 0.0 | | 0.0 | 0.0 | | | 0.0 | | | | 0.0 |
| Total Delay | 20.5 | 28.2 | | 15.8 | 18.0 | | | 6.0 | | | | 15.0 |
| LOS | C | C | | B | B | | | A | | | | B |
| Approach Delay | | 28.2 | | | 17.6 | | | 6.0 | | | | 15.0 |
| Approach LOS | | C | | | B | | | A | | | | B |
| Queue Length 50th (m) | 0.2 | 50.1 | | 21.6 | 74.5 | | | 1.3 | | | | 1.6 |
| Queue Length 95th (m) | 1.8 | 68.2 | | 35.2 | 97.9 | | | 7.8 | | | | 7.2 |
| Internal Link Dist (m) | | 101.0 | | | 692.7 | | | 191.5 | | | | 67.4 |
| Turn Bay Length (m) | 60.0 | | | 50.0 | | | | | | | | |
| Base Capacity (vph) | 147 | 1064 | | 439 | 1672 | | | 930 | | | | 421 |
| Starvation Cap Reductn | 0 | 0 | | 0 | 0 | | | 0 | | | | 0 |
| Spillback Cap Reductn | 0 | 0 | | 0 | 0 | | | 0 | | | | 0 |
| Storage Cap Reductn | 0 | 0 | | 0 | 0 | | | 0 | | | | 0 |
| Reduced v/c Ratio | 0.01 | 0.63 | | 0.59 | 0.70 | | | 0.17 | | | | 0.06 |

Intersection Summary

| | |
|-----------------------------------|--|
| Area Type: | Other |
| Cycle Length: | 90 |
| Actuated Cycle Length: | 90 |
| Offset: | 0 (0%), Referenced to phase 2:EBTL, Start of Green |
| Natural Cycle: | 75 |
| Control Type: | Pretimed |
| Maximum v/c Ratio: | 0.70 |
| Intersection Signal Delay: | 19.9 |
| Intersection LOS: | B |
| Intersection Capacity Utilization | 71.3% |
| ICU Level of Service | C |
| Analysis Period (min) | 15 |

Splits and Phases: 3: Wentworth Street N & Burlington St E/Burlington Street E



Lanes, Volumes, Timings
9: Birch Avenue & Burlington Street E

AM Peak Period
06-07-2019



| Lane Group | EBT | EBR | WBL | WBT | NBL | NBR |
|----------------------------|-------|-------|-------|-------|-------|-------|
| Lane Configurations | ↑↑↑ | | ↵↵ | ↑↑↑ | | |
| Traffic Volume (vph) | 718 | 26 | 311 | 1631 | 0 | 0 |
| Future Volume (vph) | 718 | 26 | 311 | 1631 | 0 | 0 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (m) | | 0.0 | 125.0 | | 0.0 | 0.0 |
| Storage Lanes | | 0 | 2 | | 0 | 0 |
| Taper Length (m) | | | 15.0 | | 15.0 | |
| Lane Util. Factor | 0.91 | 0.91 | 0.97 | 0.91 | 1.00 | 1.00 |
| Frt | 0.995 | | | | | |
| Flt Protected | | | 0.950 | | | |
| Satd. Flow (prot) | 4457 | 0 | 3224 | 4821 | 0 | 0 |
| Flt Permitted | | | 0.950 | | | |
| Satd. Flow (perm) | 4457 | 0 | 3224 | 4821 | 0 | 0 |
| Right Turn on Red | | Yes | | | | Yes |
| Satd. Flow (RTOR) | 11 | | | | | |
| Link Speed (k/h) | 50 | | | 50 | 50 | |
| Link Distance (m) | 716.7 | | | 130.8 | 242.7 | |
| Travel Time (s) | 51.6 | | | 9.4 | 17.5 | |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles (%) | 12% | 10% | 5% | 4% | 0% | 0% |
| Adj. Flow (vph) | 780 | 28 | 338 | 1773 | 0 | 0 |
| Shared Lane Traffic (%) | | | | | | |
| Lane Group Flow (vph) | 808 | 0 | 338 | 1773 | 0 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No |
| Lane Alignment | Left | Right | Left | Left | Left | Right |
| Median Width(m) | 6.6 | | | 6.6 | 0.0 | |
| Link Offset(m) | 0.0 | | | 0.0 | 0.0 | |
| Crosswalk Width(m) | 1.6 | | | 1.6 | 1.6 | |
| Two way Left Turn Lane | Yes | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | | 14 | 24 | | 24 | 14 |
| Turn Type | NA | | Prot | NA | | |
| Protected Phases | 2 | | 4 | 2 4 | | |
| Permitted Phases | | | | | | |
| Minimum Split (s) | 47.0 | | 15.1 | | | |
| Total Split (s) | 62.0 | | 28.0 | | | |
| Total Split (%) | 68.9% | | 31.1% | | | |
| Maximum Green (s) | 57.0 | | 22.9 | | | |
| Yellow Time (s) | 3.7 | | 3.7 | | | |
| All-Red Time (s) | 1.3 | | 1.4 | | | |
| Lost Time Adjust (s) | 0.0 | | 0.0 | | | |
| Total Lost Time (s) | 5.0 | | 5.1 | | | |
| Lead/Lag | | | | | | |
| Lead-Lag Optimize? | | | | | | |
| Walk Time (s) | 30.0 | | | | | |
| Flash Dont Walk (s) | 12.0 | | | | | |
| Pedestrian Calls (#/hr) | 0 | | | | | |
| Act Effct Green (s) | 57.0 | | 22.9 | 90.0 | | |
| Actuated g/C Ratio | 0.63 | | 0.25 | 1.00 | | |

Lanes, Volumes, Timings
 9: Birch Avenue & Burlington Street E

AM Peak Period
 06-07-2019



| Lane Group | EBT | EBR | WBL | WBT | NBL | NBR |
|------------------------|-------|-----|-------|-------|-------|-----|
| v/c Ratio | 0.29 | | 0.41 | 0.37 | | |
| Control Delay | 18.6 | | 29.8 | 0.2 | | |
| Queue Delay | 0.0 | | 0.0 | 0.0 | | |
| Total Delay | 18.6 | | 29.8 | 0.2 | | |
| LOS | B | | C | A | | |
| Approach Delay | 18.6 | | | 5.0 | | |
| Approach LOS | B | | | A | | |
| Queue Length 50th (m) | 36.5 | | 25.1 | 0.0 | | |
| Queue Length 95th (m) | 50.9 | | 37.1 | 0.0 | | |
| Internal Link Dist (m) | 692.7 | | | 106.8 | 218.7 | |
| Turn Bay Length (m) | | | 125.0 | | | |
| Base Capacity (vph) | 2826 | | 820 | 4821 | | |
| Starvation Cap Reductn | 0 | | 0 | 0 | | |
| Spillback Cap Reductn | 0 | | 0 | 0 | | |
| Storage Cap Reductn | 0 | | 0 | 0 | | |
| Reduced v/c Ratio | 0.29 | | 0.41 | 0.37 | | |

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 0 (0%), Referenced to phase 2:EBWB and 6:, Start of Green
 Natural Cycle: 65
 Control Type: Pretimed
 Maximum v/c Ratio: 0.41
 Intersection Signal Delay: 8.7
 Intersection Capacity Utilization 35.7%
 Analysis Period (min) 15
 Intersection LOS: A
 ICU Level of Service A

Splits and Phases: 9: Birch Avenue & Burlington Street E



Lanes, Volumes, Timings
13: Wentworth Street N & Brant Street

AM Peak Period
06-07-2019



| Lane Group | WBL | WBR | NBT | NBR | SBL | SBT |
|----------------------------|-------|-------|-------|-------|------|-------|
| Lane Configurations | | | | | | |
| Traffic Volume (vph) | 30 | 10 | 133 | 67 | 10 | 405 |
| Future Volume (vph) | 30 | 10 | 133 | 67 | 10 | 405 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Util. Factor | 1.00 | 1.00 | 0.95 | 0.95 | 0.95 | 0.95 |
| Frt | 0.966 | | 0.950 | | | |
| Flt Protected | 0.964 | | | | | 0.999 |
| Satd. Flow (prot) | 1500 | 0 | 3079 | 0 | 0 | 3245 |
| Flt Permitted | 0.964 | | | | | 0.999 |
| Satd. Flow (perm) | 1500 | 0 | 3079 | 0 | 0 | 3245 |
| Link Speed (k/h) | 48 | | 50 | | | 50 |
| Link Distance (m) | 104.2 | | 236.6 | | | 215.5 |
| Travel Time (s) | 7.8 | | 17.0 | | | 15.5 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles (%) | 14% | 14% | 9% | 5% | 25% | 7% |
| Adj. Flow (vph) | 33 | 11 | 145 | 73 | 11 | 440 |
| Shared Lane Traffic (%) | | | | | | |
| Lane Group Flow (vph) | 44 | 0 | 218 | 0 | 0 | 451 |
| Enter Blocked Intersection | No | No | No | No | No | No |
| Lane Alignment | Left | Right | Left | Right | Left | Left |
| Median Width(m) | 3.3 | | 0.0 | | | 0.0 |
| Link Offset(m) | 0.0 | | 0.0 | | | 0.0 |
| Crosswalk Width(m) | 1.6 | | 1.6 | | | 1.6 |
| Two way Left Turn Lane | | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | 24 | 14 | | 14 | 24 | |
| Sign Control | Stop | | Free | | | Free |

Intersection Summary

| | |
|-----------------------------------|------------------------|
| Area Type: | Other |
| Control Type: | Unsignalized |
| Intersection Capacity Utilization | 28.4% |
| Analysis Period (min) | 15 |
| | ICU Level of Service A |

Lanes, Volumes, Timings
 14: Site Access #2 & Brant Street

AM Peak Period
 06-07-2019



| Lane Group | EBT | EBR | WBL | WBT | NBL | NBR |
|----------------------------|------|-------|------|-------|------|-------|
| Lane Configurations | | | | | | |
| Traffic Volume (vph) | 66 | 0 | 0 | 96 | 0 | 1 |
| Future Volume (vph) | 66 | 0 | 0 | 96 | 0 | 1 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Fr _t | | | | | | 0.850 |
| Fl _t Protected | | | | | | |
| Satd. Flow (prot) | 1766 | 0 | 0 | 1625 | 918 | 781 |
| Fl _t Permitted | | | | | | |
| Satd. Flow (perm) | 1766 | 0 | 0 | 1625 | 918 | 781 |
| Link Speed (k/h) | 48 | | | 48 | 48 | |
| Link Distance (m) | 77.1 | | | 157.8 | 61.8 | |
| Travel Time (s) | 5.8 | | | 11.8 | 4.6 | |
| Peak Hour Factor | 0.92 | 0.70 | 0.70 | 0.92 | 0.70 | 0.70 |
| Heavy Vehicles (%) | 4% | 100% | 100% | 13% | 100% | 100% |
| Adj. Flow (vph) | 72 | 0 | 0 | 104 | 0 | 1 |
| Shared Lane Traffic (%) | | | | | | |
| Lane Group Flow (vph) | 72 | 0 | 0 | 104 | 0 | 1 |
| Enter Blocked Intersection | No | No | No | No | No | No |
| Lane Alignment | Left | Right | Left | Left | Left | Right |
| Median Width(m) | 0.0 | | | 0.0 | 3.3 | |
| Link Offset(m) | 0.0 | | | 0.0 | 0.0 | |
| Crosswalk Width(m) | 1.6 | | | 1.6 | 1.6 | |
| Two way Left Turn Lane | | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | | 14 | 24 | | 24 | 14 |
| Sign Control | Free | | | Free | Stop | |

Intersection Summary

| | |
|-----------------------------------|------------------------|
| Area Type: | Other |
| Control Type: | Unsignalized |
| Intersection Capacity Utilization | 15.1% |
| Analysis Period (min) | 15 |
| | ICU Level of Service A |

Lanes, Volumes, Timings
15: Brant Street & Birch Avenue

AM Peak Period
06-07-2019



| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|----------------------------|------|-------|-------|-------|-------|-------|------|------|-------|-------|-------|-------|
| Lane Configurations | | | | | | | | | | | | |
| Traffic Volume (vph) | 0 | 26 | 39 | 30 | 36 | 0 | 0 | 0 | 0 | 23 | 229 | 35 |
| Future Volume (vph) | 0 | 26 | 39 | 30 | 36 | 0 | 0 | 0 | 0 | 23 | 229 | 35 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (m) | 0.0 | | 0.0 | 0.0 | | 0.0 | 0.0 | | 0.0 | 0.0 | | 150.0 |
| Storage Lanes | 0 | | 0 | 0 | | 0 | 0 | | 0 | 0 | | 0 |
| Taper Length (m) | 15.0 | | | 15.0 | | | 15.0 | | | 15.0 | | |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.91 | 0.91 | 0.91 |
| Frt | | 0.919 | | | | | | | | | | 0.982 |
| Flt Protected | | | | | 0.978 | | | | | | | 0.996 |
| Satd. Flow (prot) | 0 | 1402 | 0 | 0 | 1650 | 0 | 0 | 0 | 0 | 0 | 4424 | 0 |
| Flt Permitted | | | | | 0.879 | | | | | | | 0.996 |
| Satd. Flow (perm) | 0 | 1402 | 0 | 0 | 1483 | 0 | 0 | 0 | 0 | 0 | 4424 | 0 |
| Right Turn on Red | | | Yes | | | Yes | | | Yes | | | Yes |
| Satd. Flow (RTOR) | | 42 | | | | | | | | | | 35 |
| Link Speed (k/h) | | 50 | | | 50 | | | 50 | | | | 50 |
| Link Distance (m) | | 157.8 | | | 106.5 | | | 75.7 | | | | 242.7 |
| Travel Time (s) | | 11.4 | | | 7.7 | | | 5.5 | | | | 17.5 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles (%) | 0% | 36% | 10% | 4% | 13% | 0% | 0% | 0% | 0% | 16% | 10% | 13% |
| Adj. Flow (vph) | 0 | 28 | 42 | 33 | 39 | 0 | 0 | 0 | 0 | 25 | 249 | 38 |
| Shared Lane Traffic (%) | | | | | | | | | | | | |
| Lane Group Flow (vph) | 0 | 70 | 0 | 0 | 72 | 0 | 0 | 0 | 0 | 0 | 312 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(m) | | 0.0 | | | 0.0 | | | 0.0 | | | | 0.0 |
| Link Offset(m) | | 0.0 | | | 0.0 | | | 0.0 | | | | 0.0 |
| Crosswalk Width(m) | | 1.6 | | | 1.6 | | | 1.6 | | | | 1.6 |
| Two way Left Turn Lane | | | | | | | | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | 24 | | 14 | 24 | | 14 | 24 | | 14 | 24 | | 14 |
| Turn Type | | NA | | Perm | NA | | | | | | Perm | NA |
| Protected Phases | | 4 | | | 8 | | | | | | | 2 |
| Permitted Phases | | | | 8 | | | | | | 2 | | |
| Minimum Split (s) | | 32.5 | | 32.5 | 32.5 | | | | | 23.4 | 23.4 | |
| Total Split (s) | | 46.0 | | 46.0 | 46.0 | | | | | 44.0 | 44.0 | |
| Total Split (%) | | 51.1% | | 51.1% | 51.1% | | | | | 48.9% | 48.9% | |
| Maximum Green (s) | | 40.5 | | 40.5 | 40.5 | | | | | 38.6 | 38.6 | |
| Yellow Time (s) | | 3.3 | | 3.3 | 3.3 | | | | | 3.3 | 3.3 | |
| All-Red Time (s) | | 2.2 | | 2.2 | 2.2 | | | | | 2.1 | 2.1 | |
| Lost Time Adjust (s) | | 0.0 | | | 0.0 | | | | | | | 0.0 |
| Total Lost Time (s) | | 5.5 | | | 5.5 | | | | | | | 5.4 |
| Lead/Lag | | | | | | | | | | | | |
| Lead-Lag Optimize? | | | | | | | | | | | | |
| Walk Time (s) | | 17.0 | | 17.0 | 17.0 | | | | | 7.0 | 7.0 | |
| Flash Dont Walk (s) | | 10.0 | | 10.0 | 10.0 | | | | | 11.0 | 11.0 | |
| Pedestrian Calls (#/hr) | | 0 | | 0 | 0 | | | | | 0 | 0 | |
| Act Effct Green (s) | | 40.5 | | | 40.5 | | | | | | | 38.6 |
| Actuated g/C Ratio | | 0.45 | | | 0.45 | | | | | | | 0.43 |

Lanes, Volumes, Timings
15: Brant Street & Birch Avenue

AM Peak Period
06-07-2019



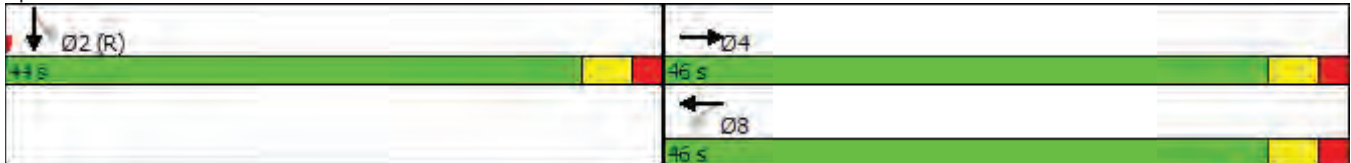
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|------------------------|-----|-------|-----|-----|------|-----|-----|------|-----|-----|-----|-------|
| v/c Ratio | | 0.11 | | | 0.11 | | | | | | | 0.16 |
| Control Delay | | 7.7 | | | 15.0 | | | | | | | 7.3 |
| Queue Delay | | 0.0 | | | 0.0 | | | | | | | 0.0 |
| Total Delay | | 7.7 | | | 15.0 | | | | | | | 7.3 |
| LOS | | A | | | B | | | | | | | A |
| Approach Delay | | 7.7 | | | 15.0 | | | | | | | 7.3 |
| Approach LOS | | A | | | B | | | | | | | A |
| Queue Length 50th (m) | | 2.6 | | | 7.0 | | | | | | | 16.9 |
| Queue Length 95th (m) | | 9.8 | | | 14.7 | | | | | | | 24.7 |
| Internal Link Dist (m) | | 133.8 | | | 82.5 | | | 51.7 | | | | 218.7 |
| Turn Bay Length (m) | | | | | | | | | | | | |
| Base Capacity (vph) | | 654 | | | 667 | | | | | | | 1917 |
| Starvation Cap Reductn | | 0 | | | 0 | | | | | | | 0 |
| Spillback Cap Reductn | | 0 | | | 0 | | | | | | | 0 |
| Storage Cap Reductn | | 0 | | | 0 | | | | | | | 0 |
| Reduced v/c Ratio | | 0.11 | | | 0.11 | | | | | | | 0.16 |

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 0 (0%), Referenced to phase 2:SBTL and 6:, Start of Green
 Natural Cycle: 60
 Control Type: Pretimed
 Maximum v/c Ratio: 0.16
 Intersection Signal Delay: 8.6
 Intersection Capacity Utilization 27.6%
 Analysis Period (min) 15

Intersection LOS: A
 ICU Level of Service A

Splits and Phases: 15: Brant Street & Birch Avenue



Lanes, Volumes, Timings
25: Hillyard Street & Brant Street

AM Peak Period
06-07-2019



| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|----------------------------|------|-------|-------|------|-------|-------|------|-------|-------|------|-------|-------|
| Lane Configurations | | ↕ | | | ↕ | | | ↕ | | | ↕ | |
| Traffic Volume (vph) | 7 | 28 | 30 | 54 | 14 | 5 | 5 | 12 | 12 | 1 | 8 | 9 |
| Future Volume (vph) | 7 | 28 | 30 | 54 | 14 | 5 | 5 | 12 | 12 | 1 | 8 | 9 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt | | 0.937 | | | 0.991 | | | 0.943 | | | 0.932 | |
| Flt Protected | | 0.994 | | | 0.964 | | | 0.992 | | | 0.998 | |
| Satd. Flow (prot) | 0 | 1682 | 0 | 0 | 1587 | 0 | 0 | 1649 | 0 | 0 | 1514 | 0 |
| Flt Permitted | | 0.994 | | | 0.964 | | | 0.992 | | | 0.998 | |
| Satd. Flow (perm) | 0 | 1682 | 0 | 0 | 1587 | 0 | 0 | 1649 | 0 | 0 | 1514 | 0 |
| Link Speed (k/h) | | 48 | | | 48 | | | 48 | | | 48 | |
| Link Distance (m) | | 103.4 | | | 96.5 | | | 111.2 | | | 105.1 | |
| Travel Time (s) | | 7.8 | | | 7.2 | | | 8.3 | | | 7.9 | |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles (%) | 0% | 4% | 0% | 10% | 8% | 25% | 0% | 10% | 0% | 0% | 14% | 13% |
| Adj. Flow (vph) | 8 | 30 | 33 | 59 | 15 | 5 | 5 | 13 | 13 | 1 | 9 | 10 |
| Shared Lane Traffic (%) | | | | | | | | | | | | |
| Lane Group Flow (vph) | 0 | 71 | 0 | 0 | 79 | 0 | 0 | 31 | 0 | 0 | 20 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(m) | | 0.0 | | | 0.0 | | | 0.0 | | | 0.0 | |
| Link Offset(m) | | 0.0 | | | 0.0 | | | 0.0 | | | 0.0 | |
| Crosswalk Width(m) | | 1.6 | | | 1.6 | | | 1.6 | | | 1.6 | |
| Two way Left Turn Lane | | | | | | | | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | 24 | | 14 | 24 | | 14 | 24 | | 14 | 24 | | 14 |
| Sign Control | | Free | | | Free | | | Stop | | | Stop | |

Intersection Summary

| | |
|-----------------------------------|--------------|
| Area Type: | Other |
| Control Type: | Unsignalized |
| Intersection Capacity Utilization | 20.7% |
| ICU Level of Service | A |
| Analysis Period (min) | 15 |

Lanes, Volumes, Timings
27: Birch Avenue & Site Access #1

AM Peak Period
06-07-2019



| Lane Group | EBL | EBR | NBL | NBT | SBT | SBR |
|----------------------------|-------|-------|------|-------|------|-------|
| Lane Configurations | | | | | | |
| Traffic Volume (vph) | 0 | 6 | 0 | 0 | 299 | 0 |
| Future Volume (vph) | 0 | 6 | 0 | 0 | 299 | 0 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 0.91 | 0.91 |
| Fr _t | 0.850 | | | | | |
| Fl _t Protected | | | | | | |
| Satd. Flow (prot) | 1837 | 781 | 0 | 0 | 4558 | 0 |
| Fl _t Permitted | | | | | | |
| Satd. Flow (perm) | 1837 | 781 | 0 | 0 | 4558 | 0 |
| Link Speed (k/h) | 48 | | | 50 | 48 | |
| Link Distance (m) | 51.6 | | | 315.9 | 75.7 | |
| Travel Time (s) | 3.9 | | | 22.7 | 5.7 | |
| Peak Hour Factor | 0.70 | 0.70 | 0.92 | 0.92 | 0.92 | 0.70 |
| Heavy Vehicles (%) | 0% | 100% | 0% | 0% | 10% | 100% |
| Adj. Flow (vph) | 0 | 9 | 0 | 0 | 325 | 0 |
| Shared Lane Traffic (%) | | | | | | |
| Lane Group Flow (vph) | 0 | 9 | 0 | 0 | 325 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No |
| Lane Alignment | Left | Right | Left | Left | Left | Right |
| Median Width(m) | 3.3 | | | 0.0 | 0.0 | |
| Link Offset(m) | 0.0 | | | 0.0 | 0.0 | |
| Crosswalk Width(m) | 1.6 | | | 1.6 | 1.6 | |
| Two way Left Turn Lane | | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | 24 | 14 | 24 | | | 14 |
| Sign Control | Stop | | | Free | Free | |

Intersection Summary

| | |
|-----------------------------------|------------------------|
| Area Type: | Other |
| Control Type: | Unsignalized |
| Intersection Capacity Utilization | 15.8% |
| Analysis Period (min) | 15 |
| | ICU Level of Service A |

Lanes, Volumes, Timings
 31: Wentworth Street N & Munroe Street

AM Peak Period
 06-07-2019



| Lane Group | WBL | WBR | NBT | NBR | SBL | SBT |
|----------------------------|-------|-------|-------|-------|------|-------|
| Lane Configurations | | | | | | |
| Traffic Volume (vph) | 30 | 9 | 195 | 52 | 2 | 343 |
| Future Volume (vph) | 30 | 9 | 195 | 52 | 2 | 343 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Util. Factor | 1.00 | 1.00 | 0.95 | 0.95 | 0.95 | 0.95 |
| Frt | 0.969 | | 0.968 | | | |
| Flt Protected | 0.963 | | | | | |
| Satd. Flow (prot) | 1468 | 0 | 3071 | 0 | 0 | 3195 |
| Flt Permitted | 0.963 | | | | | |
| Satd. Flow (perm) | 1468 | 0 | 3071 | 0 | 0 | 3195 |
| Link Speed (k/h) | 48 | | 50 | | | 50 |
| Link Distance (m) | 118.1 | | 172.4 | | | 236.6 |
| Travel Time (s) | 8.9 | | 12.4 | | | 17.0 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles (%) | 13% | 29% | 10% | 10% | 50% | 9% |
| Adj. Flow (vph) | 33 | 10 | 212 | 57 | 2 | 373 |
| Shared Lane Traffic (%) | | | | | | |
| Lane Group Flow (vph) | 43 | 0 | 269 | 0 | 0 | 375 |
| Enter Blocked Intersection | No | No | No | No | No | No |
| Lane Alignment | Left | Right | Left | Right | Left | Left |
| Median Width(m) | 3.3 | | 0.0 | | | 0.0 |
| Link Offset(m) | 0.0 | | 0.0 | | | 0.0 |
| Crosswalk Width(m) | 1.6 | | 1.6 | | | 1.6 |
| Two way Left Turn Lane | | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | 24 | 14 | | 14 | 24 | |
| Sign Control | Stop | | Free | | | Free |

Intersection Summary

| | |
|-----------------------------------|------------------------|
| Area Type: | Other |
| Control Type: | Unsignalized |
| Intersection Capacity Utilization | 20.9% |
| Analysis Period (min) | 15 |
| | ICU Level of Service A |

Lanes, Volumes, Timings
34: Hillyard Street & Site Access #3

AM Peak Period
06-07-2019



| Lane Group | WBL | WBR | NBT | NBR | SBL | SBT |
|----------------------------|-------|-------|-------|-------|------|-------|
| Lane Configurations | | | | | | |
| Traffic Volume (vph) | 2 | 8 | 35 | 17 | 68 | 36 |
| Future Volume (vph) | 2 | 8 | 35 | 17 | 68 | 36 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt | 0.894 | | 0.948 | | | |
| Flt Protected | 0.989 | | | | | 0.966 |
| Satd. Flow (prot) | 1624 | 0 | 1641 | 0 | 0 | 1725 |
| Flt Permitted | 0.989 | | | | | 0.966 |
| Satd. Flow (perm) | 1624 | 0 | 1641 | 0 | 0 | 1725 |
| Link Speed (k/h) | 48 | | 48 | | | 48 |
| Link Distance (m) | 48.3 | | 26.1 | | | 111.2 |
| Travel Time (s) | 3.6 | | 2.0 | | | 8.3 |
| Peak Hour Factor | 0.70 | 0.70 | 0.92 | 0.70 | 0.70 | 0.92 |
| Heavy Vehicles (%) | 0% | 0% | 10% | 0% | 0% | 10% |
| Adj. Flow (vph) | 3 | 11 | 38 | 24 | 97 | 39 |
| Shared Lane Traffic (%) | | | | | | |
| Lane Group Flow (vph) | 14 | 0 | 62 | 0 | 0 | 136 |
| Enter Blocked Intersection | No | No | No | No | No | No |
| Lane Alignment | Left | Right | Left | Right | Left | Left |
| Median Width(m) | 3.3 | | 0.0 | | | 0.0 |
| Link Offset(m) | 0.0 | | 0.0 | | | 0.0 |
| Crosswalk Width(m) | 1.6 | | 1.6 | | | 1.6 |
| Two way Left Turn Lane | | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | 24 | 14 | | 14 | 24 | |
| Sign Control | Stop | | Free | | | Free |

Intersection Summary

Area Type: Other
 Control Type: Unsignalized
 Intersection Capacity Utilization 22.3% ICU Level of Service A
 Analysis Period (min) 15

Lanes, Volumes, Timings
3: Wentworth Street N & Burlington Street E

PM Peak Period
06-07-2019



| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|----------------------------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|
| Lane Configurations | | | | | | | | | | | | |
| Traffic Volume (vph) | 11 | 704 | 39 | 173 | 715 | 11 | 16 | 7 | 117 | 11 | 14 | 9 |
| Future Volume (vph) | 11 | 704 | 39 | 173 | 715 | 11 | 16 | 7 | 117 | 11 | 14 | 9 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (m) | 60.0 | | 0.0 | 50.0 | | 0.0 | 0.0 | | 0.0 | 0.0 | | 0.0 |
| Storage Lanes | 1 | | 0 | 1 | | 0 | 0 | | 0 | 0 | | 0 |
| Taper Length (m) | 15.0 | | | 15.0 | | | 15.0 | | | 15.0 | | |
| Lane Util. Factor | 1.00 | 0.95 | 0.95 | 1.00 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 1.00 | 1.00 | 1.00 |
| Frt | | 0.992 | | | 0.998 | | | 0.875 | | | 0.964 | |
| Flt Protected | 0.950 | | | 0.950 | | | | 0.994 | | | 0.984 | |
| Satd. Flow (prot) | 1572 | 3074 | 0 | 1572 | 3161 | 0 | 0 | 2677 | 0 | 0 | 1423 | 0 |
| Flt Permitted | 0.353 | | | 0.193 | | | | 0.929 | | | 0.909 | |
| Satd. Flow (perm) | 584 | 3074 | 0 | 319 | 3161 | 0 | 0 | 2502 | 0 | 0 | 1315 | 0 |
| Right Turn on Red | | | Yes | | | Yes | | | Yes | | | Yes |
| Satd. Flow (RTOR) | | 7 | | | 3 | | | 127 | | | 10 | |
| Link Speed (k/h) | | 50 | | | 50 | | | 50 | | | 50 | |
| Link Distance (m) | | 125.0 | | | 716.7 | | | 215.5 | | | 91.4 | |
| Travel Time (s) | | 9.0 | | | 51.6 | | | 15.5 | | | 6.6 | |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles (%) | 11% | 12% | 24% | 11% | 10% | 22% | 0% | 0% | 16% | 22% | 11% | 40% |
| Adj. Flow (vph) | 12 | 765 | 42 | 188 | 777 | 12 | 17 | 8 | 127 | 12 | 15 | 10 |
| Shared Lane Traffic (%) | | | | | | | | | | | | |
| Lane Group Flow (vph) | 12 | 807 | 0 | 188 | 789 | 0 | 0 | 152 | 0 | 0 | 37 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(m) | | 3.3 | | | 3.3 | | | 0.0 | | | 0.0 | |
| Link Offset(m) | | 0.0 | | | 0.0 | | | 0.0 | | | 0.0 | |
| Crosswalk Width(m) | | 4.9 | | | 4.9 | | | 4.9 | | | 4.9 | |
| Two way Left Turn Lane | | | | | Yes | | | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | 24 | | 14 | 24 | | 14 | 24 | | 14 | 24 | | 14 |
| Turn Type | Perm | NA | | pm+pt | NA | | Perm | NA | | Perm | NA | |
| Protected Phases | | 2 | | 1 | 6 | | | 4 | | | 8 | |
| Permitted Phases | 2 | | | 6 | | | 4 | | | 8 | | |
| Minimum Split (s) | 29.0 | 29.0 | | 9.5 | 37.2 | | 35.0 | 35.0 | | 35.0 | 35.0 | |
| Total Split (s) | 38.0 | 38.0 | | 17.0 | 55.0 | | 35.0 | 35.0 | | 35.0 | 35.0 | |
| Total Split (%) | 42.2% | 42.2% | | 18.9% | 61.1% | | 38.9% | 38.9% | | 38.9% | 38.9% | |
| Maximum Green (s) | 32.0 | 32.0 | | 13.0 | 48.8 | | 29.0 | 29.0 | | 29.0 | 29.0 | |
| Yellow Time (s) | 3.3 | 3.3 | | 3.0 | 3.3 | | 3.3 | 3.3 | | 3.3 | 3.3 | |
| All-Red Time (s) | 2.7 | 2.7 | | 1.0 | 2.9 | | 2.7 | 2.7 | | 2.7 | 2.7 | |
| Lost Time Adjust (s) | 0.0 | 0.0 | | 0.0 | 0.0 | | 0.0 | 0.0 | | 0.0 | 0.0 | |
| Total Lost Time (s) | 6.0 | 6.0 | | 4.0 | 6.2 | | 6.0 | 6.0 | | 6.0 | 6.0 | |
| Lead/Lag | Lag | Lag | | Lead | | | | | | | | |
| Lead-Lag Optimize? | Yes | Yes | | Yes | | | | | | | | |
| Walk Time (s) | 7.0 | 7.0 | | | 15.0 | | 11.0 | 11.0 | | 11.0 | 11.0 | |
| Flash Dont Walk (s) | 16.0 | 16.0 | | | 16.0 | | 18.0 | 18.0 | | 18.0 | 18.0 | |
| Pedestrian Calls (#/hr) | 0 | 0 | | | 0 | | 0 | 0 | | 0 | 0 | |
| Act Effct Green (s) | 32.0 | 32.0 | | 51.0 | 48.8 | | 29.0 | 29.0 | | 29.0 | 29.0 | |
| Actuated g/C Ratio | 0.36 | 0.36 | | 0.57 | 0.54 | | 0.32 | 0.32 | | 0.32 | 0.32 | |

Lanes, Volumes, Timings
3: Wentworth Street N & Burlington Street E

PM Peak Period
06-07-2019

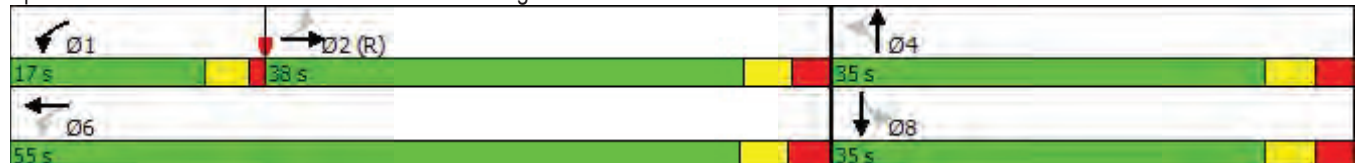


| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|------------------------|------|-------|-----|------|-------|-----|-----|-------|-----|-----|-----|------|
| v/c Ratio | 0.06 | 0.74 | | 0.52 | 0.46 | | | 0.17 | | | | 0.09 |
| Control Delay | 20.3 | 29.9 | | 14.9 | 13.6 | | | 6.5 | | | | 17.6 |
| Queue Delay | 0.0 | 0.0 | | 0.0 | 0.0 | | | 0.0 | | | | 0.0 |
| Total Delay | 20.3 | 29.9 | | 14.9 | 13.6 | | | 6.5 | | | | 17.6 |
| LOS | C | C | | B | B | | | A | | | | B |
| Approach Delay | | 29.8 | | | 13.8 | | | 6.5 | | | | 17.6 |
| Approach LOS | | C | | | B | | | A | | | | B |
| Queue Length 50th (m) | 1.4 | 62.6 | | 14.9 | 40.9 | | | 1.5 | | | | 3.2 |
| Queue Length 95th (m) | 5.1 | 84.1 | | 25.6 | 54.7 | | | 8.0 | | | | 9.9 |
| Internal Link Dist (m) | | 101.0 | | | 692.7 | | | 191.5 | | | | 67.4 |
| Turn Bay Length (m) | 60.0 | | | 50.0 | | | | | | | | |
| Base Capacity (vph) | 207 | 1097 | | 361 | 1715 | | | 892 | | | | 430 |
| Starvation Cap Reductn | 0 | 0 | | 0 | 0 | | | 0 | | | | 0 |
| Spillback Cap Reductn | 0 | 0 | | 0 | 0 | | | 0 | | | | 0 |
| Storage Cap Reductn | 0 | 0 | | 0 | 0 | | | 0 | | | | 0 |
| Reduced v/c Ratio | 0.06 | 0.74 | | 0.52 | 0.46 | | | 0.17 | | | | 0.09 |

Intersection Summary

| | |
|------------------------------------|--|
| Area Type: | Other |
| Cycle Length: | 90 |
| Actuated Cycle Length: | 90 |
| Offset: | 0 (0%), Referenced to phase 2:EBTL, Start of Green |
| Natural Cycle: | 75 |
| Control Type: | Pretimed |
| Maximum v/c Ratio: | 0.74 |
| Intersection Signal Delay: | 19.9 |
| Intersection LOS: | B |
| Intersection Capacity Utilization: | 63.3% |
| ICU Level of Service: | B |
| Analysis Period (min): | 15 |

Splits and Phases: 3: Wentworth Street N & Burlington Street E



Lanes, Volumes, Timings
9: Birch Avenue & Burlington Street E

PM Peak Period
06-07-2019



| Lane Group | EBT | EBR | WBL | WBT | NBL | NBR |
|----------------------------|-------|-------|-------|-------|-------|-------|
| Lane Configurations | ↑↑↑ | | ↵↵ | ↑↑↑ | | |
| Traffic Volume (vph) | 1238 | 32 | 287 | 1353 | 0 | 0 |
| Future Volume (vph) | 1238 | 32 | 287 | 1353 | 0 | 0 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (m) | | 0.0 | 125.0 | | 0.0 | 0.0 |
| Storage Lanes | | 0 | 2 | | 0 | 0 |
| Taper Length (m) | | | 15.0 | | 15.0 | |
| Lane Util. Factor | 0.91 | 0.91 | 0.97 | 0.91 | 1.00 | 1.00 |
| Frt | 0.996 | | | | | |
| Flt Protected | | | 0.950 | | | |
| Satd. Flow (prot) | 4718 | 0 | 3134 | 4643 | 0 | 0 |
| Flt Permitted | | | 0.950 | | | |
| Satd. Flow (perm) | 4718 | 0 | 3134 | 4643 | 0 | 0 |
| Right Turn on Red | | Yes | | | | Yes |
| Satd. Flow (RTOR) | 9 | | | | | |
| Link Speed (k/h) | 50 | | | 50 | 50 | |
| Link Distance (m) | 716.7 | | | 130.8 | 242.7 | |
| Travel Time (s) | 51.6 | | | 9.4 | 17.5 | |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles (%) | 6% | 0% | 8% | 8% | 0% | 0% |
| Adj. Flow (vph) | 1346 | 35 | 312 | 1471 | 0 | 0 |
| Shared Lane Traffic (%) | | | | | | |
| Lane Group Flow (vph) | 1381 | 0 | 312 | 1471 | 0 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No |
| Lane Alignment | Left | Right | Left | Left | Left | Right |
| Median Width(m) | 6.6 | | | 6.6 | 0.0 | |
| Link Offset(m) | 0.0 | | | 0.0 | 0.0 | |
| Crosswalk Width(m) | 1.6 | | | 1.6 | 1.6 | |
| Two way Left Turn Lane | Yes | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | | 14 | 24 | | 24 | 14 |
| Turn Type | NA | | Prot | NA | | |
| Protected Phases | 2 | | 4 | 2 4 | | |
| Permitted Phases | | | | | | |
| Minimum Split (s) | 47.0 | | 15.1 | | | |
| Total Split (s) | 64.0 | | 26.0 | | | |
| Total Split (%) | 71.1% | | 28.9% | | | |
| Maximum Green (s) | 59.0 | | 20.9 | | | |
| Yellow Time (s) | 3.7 | | 3.7 | | | |
| All-Red Time (s) | 1.3 | | 1.4 | | | |
| Lost Time Adjust (s) | 0.0 | | 0.0 | | | |
| Total Lost Time (s) | 5.0 | | 5.1 | | | |
| Lead/Lag | | | | | | |
| Lead-Lag Optimize? | | | | | | |
| Walk Time (s) | 30.0 | | | | | |
| Flash Dont Walk (s) | 12.0 | | | | | |
| Pedestrian Calls (#/hr) | 0 | | | | | |
| Act Effct Green (s) | 59.0 | | 20.9 | 90.0 | | |
| Actuated g/C Ratio | 0.66 | | 0.23 | 1.00 | | |

Lanes, Volumes, Timings
 9: Birch Avenue & Burlington Street E

PM Peak Period
 06-07-2019



| Lane Group | EBT | EBR | WBL | WBT | NBL | NBR |
|------------------------|-------|-----|-------|-------|-------|-----|
| v/c Ratio | 0.45 | | 0.43 | 0.32 | | |
| Control Delay | 15.3 | | 31.6 | 0.2 | | |
| Queue Delay | 0.0 | | 0.0 | 0.0 | | |
| Total Delay | 15.3 | | 31.6 | 0.2 | | |
| LOS | B | | C | A | | |
| Approach Delay | 15.3 | | | 5.7 | | |
| Approach LOS | B | | | A | | |
| Queue Length 50th (m) | 55.8 | | 23.7 | 0.0 | | |
| Queue Length 95th (m) | 72.2 | | 35.7 | 0.0 | | |
| Internal Link Dist (m) | 692.7 | | | 106.8 | 218.7 | |
| Turn Bay Length (m) | | | 125.0 | | | |
| Base Capacity (vph) | 3096 | | 727 | 4643 | | |
| Starvation Cap Reductn | 0 | | 0 | 0 | | |
| Spillback Cap Reductn | 0 | | 0 | 0 | | |
| Storage Cap Reductn | 0 | | 0 | 0 | | |
| Reduced v/c Ratio | 0.45 | | 0.43 | 0.32 | | |

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 0 (0%), Referenced to phase 2:EBWB and 6:, Start of Green
 Natural Cycle: 65
 Control Type: Pretimed
 Maximum v/c Ratio: 0.45
 Intersection Signal Delay: 9.9
 Intersection Capacity Utilization 41.4%
 Analysis Period (min) 15
 Intersection LOS: A
 ICU Level of Service A

Splits and Phases: 9: Birch Avenue & Burlington Street E



Lanes, Volumes, Timings
13: Brant Street & Wentworth Street N

PM Peak Period
06-07-2019



| Lane Group | WBL | WBR | NBT | NBR | SBL | SBT |
|----------------------------|-------|-------|-------|-------|------|-------|
| Lane Configurations | | | | | | |
| Traffic Volume (vph) | 57 | 9 | 148 | 39 | 14 | 261 |
| Future Volume (vph) | 57 | 9 | 148 | 39 | 14 | 261 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Util. Factor | 1.00 | 1.00 | 0.95 | 0.95 | 0.95 | 0.95 |
| Frt | 0.981 | | 0.969 | | | |
| Flt Protected | 0.959 | | | | | 0.997 |
| Satd. Flow (prot) | 1630 | 0 | 3173 | 0 | 0 | 3229 |
| Flt Permitted | 0.959 | | | | | 0.997 |
| Satd. Flow (perm) | 1630 | 0 | 3173 | 0 | 0 | 3229 |
| Link Speed (k/h) | 48 | | 48 | | | 48 |
| Link Distance (m) | 104.2 | | 236.6 | | | 215.5 |
| Travel Time (s) | 7.8 | | 17.7 | | | 16.2 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles (%) | 7% | 0% | 7% | 5% | 22% | 7% |
| Adj. Flow (vph) | 62 | 10 | 161 | 42 | 15 | 284 |
| Shared Lane Traffic (%) | | | | | | |
| Lane Group Flow (vph) | 72 | 0 | 203 | 0 | 0 | 299 |
| Enter Blocked Intersection | No | No | No | No | No | No |
| Lane Alignment | Left | Right | Left | Right | Left | Left |
| Median Width(m) | 3.3 | | 0.0 | | | 0.0 |
| Link Offset(m) | 0.0 | | 0.0 | | | 0.0 |
| Crosswalk Width(m) | 1.6 | | 1.6 | | | 1.6 |
| Two way Left Turn Lane | | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | 24 | 14 | | 14 | 24 | |
| Sign Control | Stop | | Free | | | Free |

Intersection Summary

Area Type: Other
 Control Type: Unsignalized
 Intersection Capacity Utilization 26.7% ICU Level of Service A
 Analysis Period (min) 15

Lanes, Volumes, Timings
14: Site Access #2 & Brant Street

PM Peak Period
06-07-2019



| Lane Group | EBT | EBR | WBL | WBT | NBL | NBR |
|----------------------------|------|-------|------|-------|------|-------|
| Lane Configurations | | | | | | |
| Traffic Volume (vph) | 105 | 0 | 1 | 69 | 0 | 0 |
| Future Volume (vph) | 105 | 0 | 1 | 69 | 0 | 0 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt | | | | | | |
| Flt Protected | | | | 0.999 | | |
| Satd. Flow (prot) | 1655 | 0 | 0 | 1580 | 918 | 918 |
| Flt Permitted | | | | 0.999 | | |
| Satd. Flow (perm) | 1655 | 0 | 0 | 1580 | 918 | 918 |
| Link Speed (k/h) | 48 | | | 48 | 48 | |
| Link Distance (m) | 77.1 | | | 157.8 | 61.8 | |
| Travel Time (s) | 5.8 | | | 11.8 | 4.6 | |
| Peak Hour Factor | 0.92 | 0.70 | 0.70 | 0.92 | 0.70 | 0.70 |
| Heavy Vehicles (%) | 11% | 100% | 100% | 15% | 100% | 100% |
| Adj. Flow (vph) | 114 | 0 | 1 | 75 | 0 | 0 |
| Shared Lane Traffic (%) | | | | | | |
| Lane Group Flow (vph) | 114 | 0 | 0 | 76 | 0 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No |
| Lane Alignment | Left | Right | Left | Left | Left | Right |
| Median Width(m) | 0.0 | | | 0.0 | 3.3 | |
| Link Offset(m) | 0.0 | | | 0.0 | 0.0 | |
| Crosswalk Width(m) | 1.6 | | | 1.6 | 1.6 | |
| Two way Left Turn Lane | | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | | 14 | 24 | | 24 | 14 |
| Sign Control | Free | | | Free | Stop | |

Intersection Summary

| | |
|-----------------------------------|------------------------|
| Area Type: | Other |
| Control Type: | Unsignalized |
| Intersection Capacity Utilization | 8.9% |
| Analysis Period (min) | 15 |
| | ICU Level of Service A |

Lanes, Volumes, Timings
15: Brant Street & Birch Avenue

PM Peak Period
06-07-2019



| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|----------------------------|------|-------|-------|-------|-------|-------|------|------|-------|-------|-------|-------|
| Lane Configurations | | | | | | | | | | | | |
| Traffic Volume (vph) | 0 | 14 | 72 | 61 | 33 | 0 | 0 | 0 | 0 | 8 | 241 | 21 |
| Future Volume (vph) | 0 | 14 | 72 | 61 | 33 | 0 | 0 | 0 | 0 | 8 | 241 | 21 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (m) | 0.0 | | 0.0 | 0.0 | | 0.0 | 0.0 | | 0.0 | 0.0 | | 150.0 |
| Storage Lanes | 0 | | 0 | 0 | | 0 | 0 | | 0 | 0 | | 0 |
| Taper Length (m) | 15.0 | | | 15.0 | | | 15.0 | | | 15.0 | | |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.91 | 0.91 | 0.91 |
| Frt | | 0.887 | | | | | | | | | | 0.988 |
| Flt Protected | | | | | 0.969 | | | | | | | 0.998 |
| Satd. Flow (prot) | 0 | 1544 | 0 | 0 | 1692 | 0 | 0 | 0 | 0 | 0 | 4703 | 0 |
| Flt Permitted | | | | | 0.795 | | | | | | | 0.998 |
| Satd. Flow (perm) | 0 | 1544 | 0 | 0 | 1388 | 0 | 0 | 0 | 0 | 0 | 4703 | 0 |
| Right Turn on Red | | | Yes | | | Yes | | | Yes | | | Yes |
| Satd. Flow (RTOR) | | 78 | | | | | | | | | | 18 |
| Link Speed (k/h) | | 50 | | | 50 | | | 50 | | | | 50 |
| Link Distance (m) | | 157.8 | | | 106.5 | | | 75.7 | | | | 242.7 |
| Travel Time (s) | | 11.4 | | | 7.7 | | | 5.5 | | | | 17.5 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles (%) | 0% | 8% | 5% | 2% | 11% | 2% | 0% | 0% | 0% | 29% | 3% | 20% |
| Adj. Flow (vph) | 0 | 15 | 78 | 66 | 36 | 0 | 0 | 0 | 0 | 9 | 262 | 23 |
| Shared Lane Traffic (%) | | | | | | | | | | | | |
| Lane Group Flow (vph) | 0 | 93 | 0 | 0 | 102 | 0 | 0 | 0 | 0 | 0 | 294 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(m) | | 0.0 | | | 0.0 | | | 0.0 | | | | 0.0 |
| Link Offset(m) | | 0.0 | | | 0.0 | | | 0.0 | | | | 0.0 |
| Crosswalk Width(m) | | 1.6 | | | 1.6 | | | 1.6 | | | | 1.6 |
| Two way Left Turn Lane | | | | | | | | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | 24 | | 14 | 24 | | 14 | 24 | | 14 | 24 | | 14 |
| Turn Type | | NA | | Perm | NA | | | | | | Perm | NA |
| Protected Phases | | 4 | | | 8 | | | | | | | 2 |
| Permitted Phases | | | | 8 | | | | | | 2 | | |
| Minimum Split (s) | | 32.5 | | 32.5 | 32.5 | | | | | 23.4 | 23.4 | |
| Total Split (s) | | 50.0 | | 50.0 | 50.0 | | | | | 40.0 | 40.0 | |
| Total Split (%) | | 55.6% | | 55.6% | 55.6% | | | | | 44.4% | 44.4% | |
| Maximum Green (s) | | 44.5 | | 44.5 | 44.5 | | | | | 34.6 | 34.6 | |
| Yellow Time (s) | | 3.3 | | 3.3 | 3.3 | | | | | 3.3 | 3.3 | |
| All-Red Time (s) | | 2.2 | | 2.2 | 2.2 | | | | | 2.1 | 2.1 | |
| Lost Time Adjust (s) | | 0.0 | | | 0.0 | | | | | | | 0.0 |
| Total Lost Time (s) | | 5.5 | | | 5.5 | | | | | | | 5.4 |
| Lead/Lag | | | | | | | | | | | | |
| Lead-Lag Optimize? | | | | | | | | | | | | |
| Walk Time (s) | | 17.0 | | 17.0 | 17.0 | | | | | 7.0 | 7.0 | |
| Flash Dont Walk (s) | | 10.0 | | 10.0 | 10.0 | | | | | 11.0 | 11.0 | |
| Pedestrian Calls (#/hr) | | 0 | | 0 | 0 | | | | | 0 | 0 | |
| Act Effct Green (s) | | 44.5 | | | 44.5 | | | | | | | 34.6 |
| Actuated g/C Ratio | | 0.49 | | | 0.49 | | | | | | | 0.38 |

Lanes, Volumes, Timings
 15: Brant Street & Birch Avenue

PM Peak Period
 06-07-2019



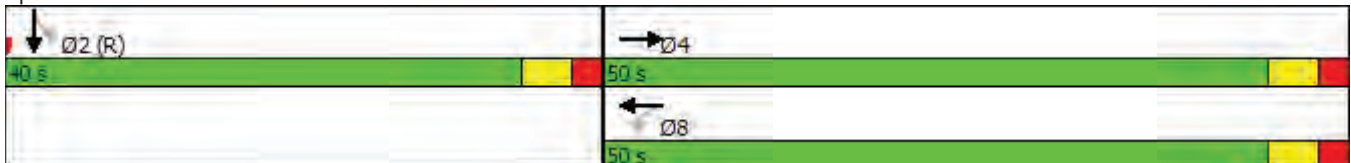
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|------------------------|-----|-------|-----|-----|------|-----|-----|------|-----|-----|-----|-------|
| v/c Ratio | | 0.12 | | | 0.15 | | | | | | | 0.16 |
| Control Delay | | 4.3 | | | 13.2 | | | | | | | 6.0 |
| Queue Delay | | 0.0 | | | 0.0 | | | | | | | 0.0 |
| Total Delay | | 4.3 | | | 13.2 | | | | | | | 6.0 |
| LOS | | A | | | B | | | | | | | A |
| Approach Delay | | 4.3 | | | 13.2 | | | | | | | 6.0 |
| Approach LOS | | A | | | B | | | | | | | A |
| Queue Length 50th (m) | | 1.3 | | | 9.2 | | | | | | | 12.1 |
| Queue Length 95th (m) | | 8.5 | | | 17.9 | | | | | | | 12.8 |
| Internal Link Dist (m) | | 133.8 | | | 82.5 | | | 51.7 | | | | 218.7 |
| Turn Bay Length (m) | | | | | | | | | | | | |
| Base Capacity (vph) | | 802 | | | 686 | | | | | | | 1819 |
| Starvation Cap Reductn | | 0 | | | 0 | | | | | | | 0 |
| Spillback Cap Reductn | | 0 | | | 0 | | | | | | | 0 |
| Storage Cap Reductn | | 0 | | | 0 | | | | | | | 0 |
| Reduced v/c Ratio | | 0.12 | | | 0.15 | | | | | | | 0.16 |

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 0 (0%), Referenced to phase 2:SBTL and 6:, Start of Green
 Natural Cycle: 60
 Control Type: Pretimed
 Maximum v/c Ratio: 0.16
 Intersection Signal Delay: 7.2
 Intersection Capacity Utilization 29.2%
 Analysis Period (min) 15

Intersection LOS: A
 ICU Level of Service A

Splits and Phases: 15: Brant Street & Birch Avenue



Lanes, Volumes, Timings
25: Hillyard Street & Brant Street

PM Peak Period
06-07-2019



| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|----------------------------|------|-------|-------|------|-------|-------|------|-------|-------|------|-------|-------|
| Lane Configurations | | ↕ | | | ↕ | | | ↕ | | | ↕ | |
| Traffic Volume (vph) | 1 | 22 | 15 | 30 | 26 | 0 | 31 | 6 | 53 | 8 | 5 | 15 |
| Future Volume (vph) | 1 | 22 | 15 | 30 | 26 | 0 | 31 | 6 | 53 | 8 | 5 | 15 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt | | 0.947 | | | | | | 0.921 | | | 0.928 | |
| Flt Protected | | 0.999 | | | 0.974 | | | 0.983 | | | 0.985 | |
| Satd. Flow (prot) | 0 | 1632 | 0 | 0 | 1789 | 0 | 0 | 1537 | 0 | 0 | 1679 | 0 |
| Flt Permitted | | 0.999 | | | 0.974 | | | 0.983 | | | 0.985 | |
| Satd. Flow (perm) | 0 | 1632 | 0 | 0 | 1789 | 0 | 0 | 1537 | 0 | 0 | 1679 | 0 |
| Link Speed (k/h) | | 48 | | | 48 | | | 48 | | | 48 | |
| Link Distance (m) | | 103.4 | | | 96.5 | | | 111.2 | | | 105.1 | |
| Travel Time (s) | | 7.8 | | | 7.2 | | | 8.3 | | | 7.9 | |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles (%) | 0% | 11% | 0% | 0% | 0% | 0% | 0% | 0% | 14% | 0% | 0% | 0% |
| Adj. Flow (vph) | 1 | 24 | 16 | 33 | 28 | 0 | 34 | 7 | 58 | 9 | 5 | 16 |
| Shared Lane Traffic (%) | | | | | | | | | | | | |
| Lane Group Flow (vph) | 0 | 41 | 0 | 0 | 61 | 0 | 0 | 99 | 0 | 0 | 30 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(m) | | 0.0 | | | 0.0 | | | 0.0 | | | 0.0 | |
| Link Offset(m) | | 0.0 | | | 0.0 | | | 0.0 | | | 0.0 | |
| Crosswalk Width(m) | | 1.6 | | | 1.6 | | | 1.6 | | | 1.6 | |
| Two way Left Turn Lane | | | | | | | | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | 24 | | 14 | 24 | | 14 | 24 | | 14 | 24 | | 14 |
| Sign Control | | Free | | | Free | | | Stop | | | Stop | |

Intersection Summary

| | |
|-----------------------------------|--------------|
| Area Type: | Other |
| Control Type: | Unsignalized |
| Intersection Capacity Utilization | 23.9% |
| ICU Level of Service | A |
| Analysis Period (min) | 15 |

Lanes, Volumes, Timings
27: Birch Avenue & Site Access #1

PM Peak Period
06-07-2019



| Lane Group | EBL | EBR | NBL | NBT | SBT | SBR |
|----------------------------|------|-------|------|-------|-------|-------|
| Lane Configurations | | | | | | |
| Traffic Volume (vph) | 0 | 0 | 0 | 0 | 373 | 2 |
| Future Volume (vph) | 0 | 0 | 0 | 0 | 373 | 2 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 0.91 | 0.91 |
| Fr _t | | | | | 0.999 | |
| Fl _t Protected | | | | | | |
| Satd. Flow (prot) | 1837 | 918 | 0 | 0 | 4830 | 0 |
| Fl _t Permitted | | | | | | |
| Satd. Flow (perm) | 1837 | 918 | 0 | 0 | 4830 | 0 |
| Link Speed (k/h) | 48 | | | 50 | 48 | |
| Link Distance (m) | 49.2 | | | 315.9 | 75.7 | |
| Travel Time (s) | 3.7 | | | 22.7 | 5.7 | |
| Peak Hour Factor | 0.70 | 0.70 | 0.92 | 0.92 | 0.92 | 0.70 |
| Heavy Vehicles (%) | 0% | 100% | 0% | 0% | 3% | 100% |
| Adj. Flow (vph) | 0 | 0 | 0 | 0 | 405 | 3 |
| Shared Lane Traffic (%) | | | | | | |
| Lane Group Flow (vph) | 0 | 0 | 0 | 0 | 408 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No |
| Lane Alignment | Left | Right | Left | Left | Left | Right |
| Median Width(m) | 3.3 | | | 0.0 | 0.0 | |
| Link Offset(m) | 0.0 | | | 0.0 | 0.0 | |
| Crosswalk Width(m) | 1.6 | | | 1.6 | 1.6 | |
| Two way Left Turn Lane | | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | 24 | 14 | 24 | | | 14 |
| Sign Control | Stop | | | Free | Free | |

Intersection Summary

| | |
|-----------------------------------|------------------------|
| Area Type: | Other |
| Control Type: | Unsignalized |
| Intersection Capacity Utilization | 10.6% |
| Analysis Period (min) | 15 |
| | ICU Level of Service A |

Lanes, Volumes, Timings
 31: Wentworth Street N & Munroe Street

PM Peak Period
 06-07-2019



| Lane Group | WBL | WBR | NBT | NBR | SBL | SBT |
|----------------------------|-------|-------|-------|-------|------|-------|
| Lane Configurations | | | | | | |
| Traffic Volume (vph) | 64 | 1 | 217 | 44 | 1 | 342 |
| Future Volume (vph) | 64 | 1 | 217 | 44 | 1 | 342 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Util. Factor | 1.00 | 1.00 | 0.95 | 0.95 | 0.95 | 0.95 |
| Frt | 0.998 | | 0.975 | | | |
| Flt Protected | 0.953 | | | | | |
| Satd. Flow (prot) | 1665 | 0 | 3165 | 0 | 0 | 3232 |
| Flt Permitted | 0.953 | | | | | |
| Satd. Flow (perm) | 1665 | 0 | 3165 | 0 | 0 | 3232 |
| Link Speed (k/h) | 48 | | 48 | | | 48 |
| Link Distance (m) | 118.1 | | 172.4 | | | 236.6 |
| Travel Time (s) | 8.9 | | 12.9 | | | 17.7 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles (%) | 5% | 0% | 7% | 10% | 0% | 8% |
| Adj. Flow (vph) | 70 | 1 | 236 | 48 | 1 | 372 |
| Shared Lane Traffic (%) | | | | | | |
| Lane Group Flow (vph) | 71 | 0 | 284 | 0 | 0 | 373 |
| Enter Blocked Intersection | No | No | No | No | No | No |
| Lane Alignment | Left | Right | Left | Right | Left | Left |
| Median Width(m) | 3.3 | | 0.0 | | | 0.0 |
| Link Offset(m) | 0.0 | | 0.0 | | | 0.0 |
| Crosswalk Width(m) | 1.6 | | 1.6 | | | 1.6 |
| Two way Left Turn Lane | | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | 24 | 14 | | 14 | 24 | |
| Sign Control | Stop | | Free | | | Free |

Intersection Summary

| | |
|-----------------------------------|------------------------|
| Area Type: | Other |
| Control Type: | Unsignalized |
| Intersection Capacity Utilization | 20.4% |
| Analysis Period (min) | 15 |
| | ICU Level of Service A |

Lanes, Volumes, Timings
34: Hillyard Street & Site Access #3

PM Peak Period
06-07-2019



| Lane Group | WBL | WBR | NBT | NBR | SBL | SBT |
|----------------------------|-------|-------|-------|-------|------|-------|
| Lane Configurations | | | | | | |
| Traffic Volume (vph) | 18 | 72 | 35 | 9 | 36 | 48 |
| Future Volume (vph) | 18 | 72 | 35 | 9 | 36 | 48 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt | 0.892 | | 0.966 | | | |
| Flt Protected | 0.990 | | | | | 0.976 |
| Satd. Flow (prot) | 1622 | 0 | 1651 | 0 | 0 | 1706 |
| Flt Permitted | 0.990 | | | | | 0.976 |
| Satd. Flow (perm) | 1622 | 0 | 1651 | 0 | 0 | 1706 |
| Link Speed (k/h) | 48 | | 48 | | | 48 |
| Link Distance (m) | 48.3 | | 26.1 | | | 111.2 |
| Travel Time (s) | 3.6 | | 2.0 | | | 8.3 |
| Peak Hour Factor | 0.70 | 0.70 | 0.92 | 0.70 | 0.70 | 0.92 |
| Heavy Vehicles (%) | 0% | 0% | 10% | 0% | 0% | 10% |
| Adj. Flow (vph) | 26 | 103 | 38 | 13 | 51 | 52 |
| Shared Lane Traffic (%) | | | | | | |
| Lane Group Flow (vph) | 129 | 0 | 51 | 0 | 0 | 103 |
| Enter Blocked Intersection | No | No | No | No | No | No |
| Lane Alignment | Left | Right | Left | Right | Left | Left |
| Median Width(m) | 3.3 | | 0.0 | | | 0.0 |
| Link Offset(m) | 0.0 | | 0.0 | | | 0.0 |
| Crosswalk Width(m) | 1.6 | | 1.6 | | | 1.6 |
| Two way Left Turn Lane | | | | | | |
| Headway Factor | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 | 1.04 |
| Turning Speed (k/h) | 24 | 14 | | 14 | 24 | |
| Sign Control | Stop | | Free | | | Free |

Intersection Summary

















| | |
|-----------------------------------|------------------------|
| Area Type: | Other |
| Control Type: | Unsignalized |
| Intersection Capacity Utilization | 23.3% |
| Analysis Period (min) | 15 |
| | ICU Level of Service A |

APPENDIX G

Synchro Outputs – Future Improvements 2027 Conditions

HCM Unsignalized Intersection Capacity Analysis
 25: Hillyard Street & Brant Street

AM Peak Period
 06-07-2019

| |  |  |  |  |  |  |  |  |  |  |  |  |
|-----------------------------------|---|---|---|---|---|---|--|---|---|---|---|---|
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | |  | | |  | | |  | | |  | |
| Sign Control | | Stop | | | Stop | | | Stop | | | Stop | |
| Traffic Volume (vph) | 7 | 28 | 15 | 54 | 14 | 5 | 5 | 12 | 12 | 1 | 8 | 9 |
| Future Volume (vph) | 7 | 28 | 15 | 54 | 14 | 5 | 5 | 12 | 12 | 1 | 8 | 9 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Hourly flow rate (vph) | 8 | 30 | 16 | 59 | 15 | 5 | 5 | 13 | 13 | 1 | 9 | 10 |
| Direction, Lane # | EB 1 | WB 1 | NB 1 | SB 1 | | | | | | | | |
| Volume Total (vph) | 54 | 79 | 31 | 20 | | | | | | | | |
| Volume Left (vph) | 8 | 59 | 5 | 1 | | | | | | | | |
| Volume Right (vph) | 16 | 5 | 13 | 10 | | | | | | | | |
| Hadj (s) | -0.11 | 0.29 | -0.15 | -0.07 | | | | | | | | |
| Departure Headway (s) | 4.0 | 4.4 | 4.1 | 4.2 | | | | | | | | |
| Degree Utilization, x | 0.06 | 0.10 | 0.04 | 0.02 | | | | | | | | |
| Capacity (veh/h) | 884 | 812 | 847 | 835 | | | | | | | | |
| Control Delay (s) | 7.2 | 7.8 | 7.2 | 7.3 | | | | | | | | |
| Approach Delay (s) | 7.2 | 7.8 | 7.2 | 7.3 | | | | | | | | |
| Approach LOS | A | A | A | A | | | | | | | | |
| Intersection Summary | | | | | | | | | | | | |
| Delay | | | 7.5 | | | | | | | | | |
| Level of Service | | | A | | | | | | | | | |
| Intersection Capacity Utilization | | | 20.7% | ICU Level of Service | A | | | | | | | |
| Analysis Period (min) | | | 15 | | | | | | | | | |

HCM Unsignalized Intersection Capacity Analysis
 25: Hillyard Street & Brant Street

PM Peak Period
 06-07-2019



| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|------------------------|------|------|------|------|------|------|------|------|------|------|------|------|
| Lane Configurations | | ↔ | | | ↔ | | | ↔ | | | ↔ | |
| Sign Control | | Stop | | | Stop | | | Stop | | | Stop | |
| Traffic Volume (vph) | 1 | 22 | 15 | 30 | 26 | 0 | 31 | 6 | 53 | 8 | 5 | 15 |
| Future Volume (vph) | 1 | 22 | 15 | 30 | 26 | 0 | 31 | 6 | 53 | 8 | 5 | 15 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Hourly flow rate (vph) | 1 | 24 | 16 | 33 | 28 | 0 | 34 | 7 | 58 | 9 | 5 | 16 |

| Direction, Lane # | EB 1 | WB 1 | NB 1 | SB 1 |
|-----------------------|-------|------|-------|-------|
| Volume Total (vph) | 41 | 61 | 99 | 30 |
| Volume Left (vph) | 1 | 33 | 34 | 9 |
| Volume Right (vph) | 16 | 0 | 58 | 16 |
| Hadj (s) | -0.12 | 0.11 | -0.14 | -0.26 |
| Departure Headway (s) | 4.1 | 4.3 | 4.0 | 4.0 |
| Degree Utilization, x | 0.05 | 0.07 | 0.11 | 0.03 |
| Capacity (veh/h) | 840 | 805 | 866 | 877 |
| Control Delay (s) | 7.3 | 7.7 | 7.5 | 7.1 |
| Approach Delay (s) | 7.3 | 7.7 | 7.5 | 7.1 |
| Approach LOS | A | A | A | A |

| Intersection Summary | | | |
|-----------------------------------|-------|-----|------------------------|
| Delay | | 7.5 | |
| Level of Service | | A | |
| Intersection Capacity Utilization | 23.9% | | ICU Level of Service A |
| Analysis Period (min) | | 15 | |

APPENDIX H

Parking Demand Survey Memorandum



IBI GROUP
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Memorandum

To/Attention Eric Czerniak **Date** May 21, 2019
From Gary Yeung **Project No** 115096
cc Scott Johnston
Subject Parking Demand & Supply Review

Introduction

IBI Group has been retained by the City of Hamilton to conduct a parking demand and supply review to aid in the design and environmental assessment study of the new Maintenance and Storage Facility (MSF).

A parking supply approach was developed with findings summarized in this memorandum. The analysis includes a review of the parking demand at the existing Mountain Transit Centre (MTC) facility and a comparison of the employee numbers between the MSF and MTC.

Employee Comparison

Currently, the MTC is the only transit operations and maintenance facility within the City of Hamilton. The MTC employs a total of 762 staffs with details summarized in Exhibit 1.

Exhibit 1: Total Number of Employees between MTC and MSF

| Description | MTC* | MSF** | Difference |
|----------------------------------|------------|------------|------------|
| Operations | 574 | 640 | +66 |
| Transit IT Costs | 5 | 0 | -5 |
| Director of Transit | 3 | 1 | -2 |
| Customer Experience & Innovation | 8 | 0 | -8 |
| Planning & Infrastructure | 16 | 0 | -16 |
| Support Services | 15 | 36 | +21 |
| ATS Service Manager | 9 | 0 | -9 |
| Transit Maintenance | 132 | 140 | +8 |
| Total: | 762 | 817 | +55 |

*MTC values accounts for Full-Time Equivalence

**Some departments will remain at MTC

Employee numbers for MTC were provided by the City as part of the 2019 budget. MSF values correspond to the total number of employees taken from the final Space Program (Version 7). These values show that the two facilities are comparable in terms of staff size, therefore, parking supply should also be comparable.

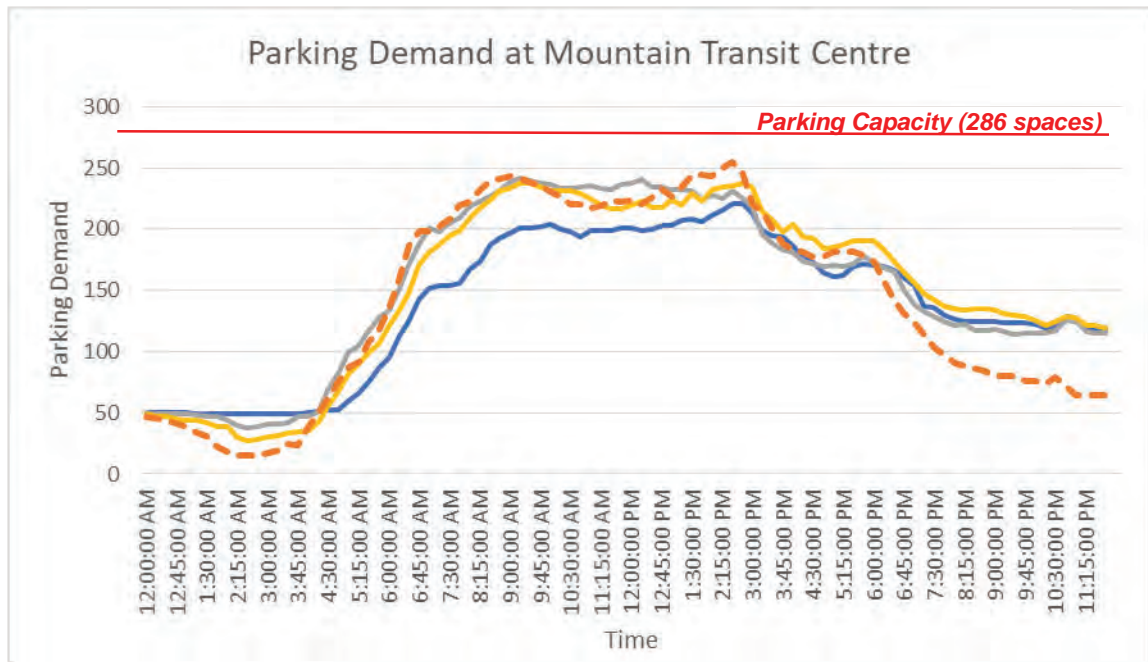
Eric Czerniak – May 21, 2019

Parking Supply and Demand Review

As stated in the *Future Transit Facility Needs Assessment* technical memo submitted to the City, parking space ratios for facility planning typically fall between 0.5 to 0.7 spaces per employee. This parking demand ratio is dependent on several factors including location of the facility, availability of transit service, and shift times. With 762 employees, the MTC currently supplies a total of 286 parking spaces for employees, plus an additional 68 spaces for visitors and commuters at the park-and-ride lot (354 spaces total). This represents a ratio of 0.465 inclusive of the park-and-ride spaces. Despite adding all of the park-and-ride spaces, the ratio is relatively low in comparison (<0.5) and suggest that parking is currently undersupplied at MTC.

To determine the current utilization of MTC, two turning movement counts (TMC) were completed, one for the north and one for the south employee lot. The TMCs capture inbound and outbound vehicle movements which translate to employees parking and leaving. As approved by City of Hamilton staff, the TMC was completed on March 19th (Tuesday) from 12 AM to 12 AM the following day. Furthermore, the City also provided access card data which counts employees' entrance and exit times to/from the parking lots. Only Tuesday, Wednesday and Thursday data were used for the analysis as it is more representative of a typical work week. The parking demands are summarized below in Exhibit 2.

Exhibit 2: Existing Parking Utilization of Mountain Transit Centre



As shown above, a maximum of 255 spaces were occupied on the Tuesday. This represents a maximum utilization rate of 89%, which occurred at approximately 3 PM in the afternoon. This behaviour is expected as operators are needed on the bus before the PM peak for the background traffic (4-6 PM). The result of this analysis shows that the existing site is close to/at capacity. Therefore, the existing low parking ratio of 0.465 seems justified at this location.

For the purpose of determining parking supply of MSF, the above ratio was recalculated to a value of 0.375 (286 spaces / 762 employees) to exclude the park-and-ride spaces. This is to ensure that ratio applied to the final space program employee numbers will not oversupply

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parking at MSF as it will not have a park-and-ride facility. It is noted, however that visitors will need to be re-added in later calculations.

As mentioned previously, parking demand is dependent on other factors such as facility location and availability of transit. There are some uncertainty with solely using the 0.375 ratio as the new MSF facility is located closer to the downtown area, whereas the MTC location lies outside the City and out of the way for vehicular traffic, contributing to higher parking demand. Also, the new MSF does not have a park-and-ride facility on site which would better accommodate commuters to take transit to work. Due to these considerations, it is difficult to determine how much the ratio needs to be increased. Therefore, for the design purposes, the approach taken was to apply the MTC parking ratio to the MSF employee numbers and to make additional adjustments as outlined below:

- Based on a background review of similar facilities, it was determined that 15 visitor spaces is sufficient for day-to-day operation. This is to account for visitors that were originally omitted in 0.375 ratio.
- City staff agreed that an additional 10 spaces should be included for supervisor vehicles.
- To design for full build-out conditions, an additional 45 parking spots was added. This symbolizes growth for future employees that will occupy planned vacant office space.
- A supply buffer of 25 additional spaces (approximately 5-10% of supply) was added to accommodate daily demand fluctuations, particularly during the shift change times.

Following this approach, a total of 402 spaces is the recommended minimum parking requirement for MSF, with a parking ratio of 0.492 (402 spaces / 817 employees). Comparing to the typical range, this ratio suggest that parking provision is sustainable and is align with City's goals and objectives by providing minimum parking supply, thereby limiting single occupant vehicles trips. A summary of the parking supply calculation is provided in Exhibit 3.

Exhibit 3: Parking Supply Calculation for MSF

| Description | Value / Calculation | # of spaces |
|-----------------------------|---------------------|------------------------------|
| Employee (817 total staff) | 817 x 0.375 | 307 (rounded) |
| Visitor parking | +15 | 322 |
| Supervisor vehicles | +10 | 332 |
| Future Growth | +45 | 377 |
| Supply Buffer | +25 | 402 |
| Total Parking Supply | - | 402 (10 barrier-free) |

**As per Zoning By-law 05-200, barrier-free requirements – minimum 2 spaces + 2% of the total number of required parking spaces.*

Conclusion & Recommendation

In conclusion, it is recommended that the new Maintenance and Storage Facility should provide a minimum of 402 parking spaces, 10 of which are barrier-free. These values should be taken into consideration in determining the final parking design of this facility.