

APPENDIX 5

DESKTOP GEOTECHNICAL & HYDROGEOLOGICAL ASSESSMENT REPORT



March 18, 2024

To: Andrew McGregor
Senior Planner / Project Manager, EA & Approvals
R.V. Anderson Associates Limited
43 Church Street, Suite 104, St. Catharines ON L2R 7E1

From: Frank C. Liu, P.Eng. & Jason Cole, M.Sc., P.Geo.

Re: **Desktop Geotechnical and Hydrogeological Assessment for Water Storage Facility Sites
Carlisle Well**

1. Introduction

Palmer Environmental Consulting Group Inc. (Palmer) was retained by R.V. Anderson Associates Limited (RVA) (the “client”) to complete a desktop geotechnical and hydrogeological assessment in support of the proposed Municipal Class Environmental Assessment (EA) and Conceptual Design for the Carlisle Water Storage Facility located in the Town of Carlisle, within the City of Hamilton (the “project” or the “sites”) (**Figure 1**).

Based on information from the Client, two (2) potential Water Storage Facility Sites are proposed (**Figure 1**). *Site 1* is Tower Park, located at 42-46 Woodend Drive, Carlisle and *Site 2* is located at 1535 Centre Road, Flamborough.

The purpose of this desktop geotechnical and hydrogeological assessment is to provide a preliminary characterization of regional setting and subsurface conditions for the two sites and to assess the expected soil mechanic properties, assess potential groundwater issues, identify data gaps and to make recommendations for additional, site-specific work, if required.

Based on information from the client and knowledge of the existing water supply system of Carlisle, the proposed storage facility is expected to include:

- Water Storage Building/Structure;
- Access road and vehicle parking; and
- Associated appurtenances, which may include a treatment facility and/or pump house.

It is noted that the Municipal Class Environmental Assessment has not determined the type of storage facility, and both elevated storage tank and underground storage tank are possible options.

2. Methodology and Data Sources

The geotechnical and hydrogeological assessment was conducted through a desktop study based on data available from multiple public sources, and consulting reports provided by the client.

The major source of data for the current study are the well records collected from Ministry of the Environment, Conservation and Parks (MECP) water well dataset (**Figure 1** and **Appendix A**).

Data from other public sources include, but not limited to, Ontario Geological Survey (OGS: physiography, geology and geotechnical boreholes), MECP, municipalities, conservation authorities, and other public agencies. The data was collected, analysed and synthesized to characterize physical and environmental settings of the two proposed sites. The physical and environmental setting will direct site subsurface condition characterization.

3. Summary of Site Soil and Groundwater Conditions

3.1 Stratigraphy

Surficial geology of both Sites was mapped by OGS as Ice-Contact Stratified deposits of gravel and sand, minor till, in the facies of esker, kame, end moraine, ice-marginal delta and subaqueous fan (**Figure 2**). Bedrock under the overburden below both sites was mapped by OGS as Amabel Formation bedrock, which is described a thick-bedded, crinoidal, locally biohermal and locally bituminous dolostone. Solution enhanced fractures and karstic features are known to be present within the Amabel Formation.

Three (3) well records were identified near Site 1 and four (4) well records were identified adjacent to Site 2 (**Appendix A**). Only well record (6807563) had stratigraphic data for Site 1. All of the well records near Site 2 have stratigraphic data, but only well records (6806877 and 6809946) were used for this assessment as they are located closes to the Site. FDC03RR is 700 m east of Site 1. The information from the well log of FDC03RR can be used to predict stratigraphy at Site 1 and Site 2.

Based on the information from the well records identified at the two Sites with reference to information from other sources, the stratigraphy conditions for the two Sites are summarized in **Table 1**. The elevation data was acquired from Conservation Halton and is also shown in **Figure 1**. While expected to be generally accurate, it should be noted that the stratigraphic data from the MECP water well records should be viewed with a degree of uncertainty.

Table 1. Anticipated Stratigraphy for Site 1 and Site 2

Data	Site 1 (WWR #6807563)	Site 2 (WWR #6806877 & #6809946)
Ground Surface Elevation (masl)	271.0 – 276.0 masl	265.0 – 272.0 masl
Stratigraphic Unit and Depth Range (m)	Sand and gravel, trace boulders: 0 – 12.7 m	Sand, some gravel: 0 – 18.3 m
	Limestone: 12.7 – 38.1 m	Limestone: 18.3 – 26.5 m
	Shale: 38.1 – 39.6+ m	Shale: 26.5 – 30.5+ m

3.2 Groundwater

Groundwater information recorded in the well records is related to the groundwater in bedrock supply aquifers. Groundwater table in overburden should be much shallower. Based on observations during drilling the redundant well, FDC03RR, as well as the well log for the sentry well (CM-03-03S/D) located to the east of the two sites (**Appendix A** and **Figure 1**), the groundwater table in the area close to the two Sites should range from 4.0 to 6.0 meters below groundwater surface (mbgs), however a shallower water table may occur seasonally.

3.3 Source Water Protection

The two Sites are situated in the Halton Region Source Protection Area and is subject to the Source Protection Plan of Halton-Hamilton Source Protection Region. The Source Water Protection Plan identifies four main regulatory factors under the *Clean Water Act (2006)* relating to local hydrogeology to consider: Significant Groundwater Recharge Areas (SGRAs), Highly Vulnerable Aquifers (HVAs), and Wellhead Protection Areas (WHPAs), and Intake Protection Zones (IPZs).

A Wellhead Protection Area (WHPA) is the area around the wellhead where land use activities have the potential to affect the quality or quantity of water that flows into the well. These areas are delineated into zones of vulnerability (A, B, C, and D) based on the time of travel of water into the well, and zones around a surface water body influencing a Groundwater Under Direct Influence (GUDI: E, F). Other zones (Q1, and Q2) are defined as the areas where new water takings or reduced recharge could impact the quantity of water available to municipal supply wells. IPZs are the area on water and land surrounding a municipal surface water intake. HVAs are aquifers that are susceptible to contamination as a result of the soil structure/material or due its location near the ground surface. Lastly, SGRAs are areas where recharge is important to maintain the water level in a community drinking water aquifer.

Based on the provincial dataset, Site 1 is located within a WHPA-A with a score of 10 and a SGRA, and the east part of Site 2 is located within a SGRA (**Figure 3**). WHPA-A with a score of 10 indicates that Site 1 is located within 100 m from the existing supply wells, and that surficial contaminants at Site 1 have a higher risk to migrate into the well screen of the nearby supply well(s).

3.4 Natural Heritage

Based on provincial dataset, no natural heritage features were identified within the two Sites. A wetland is identified adjacent to the south boundary of Site 2. The wetland is a not identified as a Provincially Significant Wetland.

The majority of Site 2 is located within the regulation limit for Conservation Halton. Site 1 is outside of the regulation limit for Conservation Halton.

4. Desktop Geotechnical Assessment

4.1 Foundation Considerations

As presented above, the proposed storage facility may include an elevated storage tank or underground storage tank and associated appurtenances. The major lithological units under both Sites are coarse grained sand and gravel underlain by limestone and shale. Based on the structural forms of the proposed development and the site stratigraphy, spreading footings are expected to be feasible for both elevated storage tank and underground storage tank options. The spread footing for the elevated storage tank may take forms of reinforced concrete raft foundation and reinforced concrete ring. The spread footing for the underground storage tank may take forms of reinforced concrete raft. The spread footing for supporting appurtenances may be conventional strip footing.

It should be noted that the spread footings have to be founded in native soil. The depth of fill will be determined through Site-specific borehole drilling completed during later design stages.

Bearing capacity of foundation soil should be determined through borehole drilling and in-situ testing such as Standard Penetration Test (SPT) and soil classification as part of a geotechnical drilling program.

Bearing capacity of foundation soil for appurtenances structures can be determined either with in-situ SPT test and soil classification as part of a geotechnical drilling program, or with in-situ footing soil inspection conducted by Palmer's experienced geotechnical staff during excavation. Footing soil inspection is usually conducted with a steel rod or a penetrometer coupled with soil classification. The experience of the inspector plays a key role in determining the bearing capacity, and the estimated bearing capacity values can only be used to apparently firm foundation soil and appurtenant structures.

Based on information of lithology as recorded in well records within and adjacent to the sites and well logs from the nearby site, for a preliminary foundation design, 70 to 100 KPa bearing capacity of soil under the sites for spread footings can be considered subjected to confirmation by geotechnical inspection that the soil must be native ice-contact deposits and over 1.5 m deep.

The foundation should be founded on firm native mineral soil and with a depth of more than 1.2 m to be below the front line.

In case the thickness of fill is greater than 1.2 m, the fill should be excavated and removed. The space should be backfilled with lean concrete to grade. Lean concrete should be designed to have a compressive strength over 5.0 MPa.

4.2 Frost Penetration and Foundation Depth

The frozen depth at the two sites is prescribed as 1.2 m on the Ontario Provincial Standard Drawing (OPSD 3090.101), which is the same as the minimal foundation depth of 1.2 m provided in Table 9.12.2.2 of the Ontario Building Code (2012). The spread footing depth of 1.2 m or more should be adequate to protect the foundation from frost heave. Based on the above site characterization, the overburden soil is not frost heave or adfreezing prone. Consequently, frost heave and adfreezing should not be a significant issue if the foundation is founded to a depth of 1.2 m or more and backfilled with well drained materials.

4.3 Excavation and Groundwater Control

The majority of excavation will go through sand and gravel. Based on the density of soil and classification as observed during drilling the redundant well, a light to medium duty backhoe excavator should be adequate to execute the excavation.

Excavation sequence, cutting slope forms and support system should be implemented in accordance with Regulation 213/91 under the Ontario Occupational Health and Safety Act (OHSA) and Ontario Building Code. For the purpose of Regulation 213/91, the soil to be excavated at the Site can be classified as Type 3. The following lists the major criteria that a support system is required by the regulation for Type 3 soil:

- Excavation is deeper than 1.2 m below grade;
- Cutting walls are not sloped from its bottom with a slope having a minimum gradient of one vertical to one horizontal; and
- There will be workers working close to cutting walls for all excavation.

As presented above, the excavation may extend to deeper than 1.2 mbgs, and there must be workers working in the trench to build foundation. Therefore, supporting system has to be considered if the cutting slope is not to be flattened to one vertical to one horizontal (1:1 slope or 45 degree natural slope).

Excavation should be closely inspected by qualified geotechnical staff. If soil condition exposed is different from findings from boreholes, excavation process and shoring system might have to be modified.

Excavated soil should be stockpiled at least 3 m away from the cutting wall crest if space is available.

Unsupported excavation sequence should be arranged such as to minimize the time of the exposure of cutting slopes to elements and to execute the excavation in dry season if possible. Tarping may be needed during extended period of rainfall to prevent erosion and soaking of the slope. Care should be taken to direct surface water away from the open excavations.

Excess soil should be disposed of according to Ontario Regulation 406/19 under the Ontario Environmental Protection Act and associated guidelines.

As mentioned above, the groundwater table under the two sites should be deeper than 3.0 m. If the excavation depth does not extend deeper than 3.0 m, groundwater seepage into the excavation pit is not anticipated. However, considering the coarse grain size of the overburden soil, perched groundwater seepage during precipitation is anticipated, which will be discussed below.

In case the groundwater is encountered at shallower depth, construction dewatering will have to be considered. Hydrogeological assessment will be recommended for each site to determine if construction dewatering is needed.

4.4 Consolidation and Settlement

Based on available lithological information of overburden soil, the founding soil and the underlying soil are not anticipated to be compressible. Drainage-consolidation due to structure loading (primary settlement) is not anticipated. Initial and secondary settlement due to soil grain re-arrangement caused by structure loading will be determined with in-situ SPT tests and soil classification as part of a geotechnical drilling program recommended below.

4.5 Seismic Considerations

The 2012 Ontario Building Code (OBC 2012) came into effect on January 1, 2014 and contains updated seismic analysis and design methodology. The seismic site classification methodology outlined in the code is based on subsurface conditions within the upper 30 m below grade.

As shown in Table 4.1.8.4A of the OBC, three methods of determining the site class are provided in the code: method 1 based on average shear wave velocity, method 2 based on average standard penetration resistance (N-value), and method 3 based on undrained shear strength. Because no information and data about the shear wave velocity, N-values and undrained shear strength are available, seismic conditions can not be assessed at this stage.

5. Desktop Hydrogeological Assessment

As presented above, Site 1 is located within a WHPA-A, and Site 1 and part of Site 2 are located in a Significant Groundwater Recharge Area (SGRA). Moreover, both sites are located in Carlisle Well Field, and Site 1 contains existing supply wells.

Groundwater levels are anticipated to be deeper than 3.0 m for both sites. If the excavation does not extend deeper than 3.0 m, construction dewatering for groundwater seepage should be minimal. If no construction dewatering for control of groundwater seepage is required, other related issues such as a Permit To Take Water (PTTW), an Environmental Activity and Sector Registry (EASR), impacts to natural heritage and interference with other water users, and dewatering-induced soil settlement would not be expected to occur.

As presented above, the shallow overburden is composed of ice-contact stratified deposits. The stratified deposits are expected to have high horizontal hydraulic conductivity values (K-values) in the range of 10^{-5} m/s. Therefore, a large quantity of transient groundwater seepage during precipitation events is anticipated. The contractor should have a sump pump with adequate capacity in place if the excavation occurs during the wet season to deal with potential perched, transient groundwater seepage and stormwater accumulation. Depending on the excavation depth and the rate of groundwater ingress, active dewatering methods such as well points or eductors may be required. Any construction dewatering in excess of 50,000 L/day is required to be registered on the MECP EASR system. Any construction dewatering in excess of 400,000 L/day requires a Category 3 PTTW from the MECP.

Based on the above preliminary characterization of site subsurface conditions, the hydrogeological conditions are considered to be suitable for shallow subsurface construction of the proposed storage facility and appurtenances. No significant groundwater constraints were identified through the desktop assessment for either Site 1 or Site 2. A Site-specific hydrogeological field program is recommended during later design

stages to confirm water table depth, soil permeability and our interpretation of the low potential for adverse effects.

The following are recommendations for the prevention of potential contamination caused by construction activities within a WHPA and SGRA:

- Appropriate awareness training of field staff on the vulnerability of the existing supply wells;
- Spill management plan has to be formulated to meet construction requirement and pass the review of the Risk Management Officer (RMO) of the City of Hamilton; and
- The construction area should be fenced and marked with clear signage for protection of existing supply wells; and
- The City's staff should inspect the construction site periodically for the purpose of onsite contamination prevention.

6. Conclusion and Recommendations

This desktop geotechnical and hydrogeological assessment was based on regional data, secondary source data and available MECP water well records within and close to the Sites. Through the desktop study, preliminary physical and environmental settings, as well as site conditions are characterized, and a preliminary assessment of geotechnical and hydrogeological conditions was provided.

The results of the geotechnical and hydrogeological assessment shows that soil mechanical properties and groundwater conditions at both Site 1 and Site 2 are considered generally suitable for the proposed water storage facility and appurtenance structures. Significant geotechnical and hydrogeological constraints are not anticipated. Geotechnically, both Site 1 and Site 2 will be appropriate for the proposed development. Hydrogeologically, Site 1 is moderately preferred to Site 2 as groundwater levels under Site 1 are predicted to be deeper, and the possibility of construction dewatering requirement is lower.

The above assessment and discussion were based on desktop studies only and should be verified or confirmed with further investigations, including Site-specific field investigations. The following is the recommended steps:

- Geotechnical and hydrogeological drilling should be conducted for the site selected. The drilling should extend to dense to very dense soils or bedrock expected at approximately 30 m depth. The drilling program should include at least three (3) boreholes outside but adjacent to the footprint of the storage tank. SPT testing, soil sampling and lab testing and classification should be completed. Groundwater monitoring wells should be installed in all boreholes to measure stabilized groundwater levels;
- At least one borehole for each appurtenance structure should be drilled to a depth of 6 m and completed as a groundwater monitoring well. SPT testing, soil sampling and lab testing and classification should be completed; and

- To facilitate soil management during excavation as required by O.Reg. 406/19, an Assessment of Past Uses (AP) is recommended during later design stages.

7. Closure

We trust that the information contained in this assessment meets your requirements. Should you have any questions, please do not hesitate to contact our office. The report is subject to the statement of limitations provided at the end of the report.

Yours truly,

Palmer™



Prepared By: _____
Frank C. Liu, P. Eng. & P. Geo.
Senior Hydrogeologist



Reviewed By: _____
Jason Cole, M.Sc., P. Geo.
VP, Principal Hydrogeologist

Statement of Limitations

The extent of this study was limited to the specific scope of work for which we were retained and that is described in this report. Palmer has assumed that the information provided by the client or any secondary sources of information are factual and accurate. Palmer accepts no responsibility for any deficiency, misstatement or inaccuracy contained in this report as a result of omissions, misinterpretations or negligent acts from relied upon data. Judgment has been used by Palmer in the interpretation of the information provided but subsurface physical and chemical characteristics may differ from regional scale geology mapping and vary between or beyond well/borehole locations given the inherent variability in geological conditions.

Palmer is not a guarantor of the geological or groundwater conditions at the subject site, but warrants only that its work was undertaken and its report prepared in a manner consistent with the level of skill and diligence normally exercised by competent geoscience professionals practicing in the Province of Ontario. Our findings, conclusions and recommendations should be evaluated in light of the limited scope of our work.

The information and opinions expressed in the Report are for the sole benefit of the Client. NO OTHER PARTY MAY USE OR RELY UPON THE REPORT OR ANY PORTION THEREOF WITHOUT PALMER'S WRITTEN CONSENT AND SUCH USE SHALL BE ON SUCH TERMS AND CONDITIONS AS PALMER MAY EXPRESSLY APPROVE. Ownership in and copyright for the contents of the Report belongs to Palmer. Any use which a third party makes of the Report is the sole responsibility of such third party. Palmer accepts no responsibility whatsoever for damages suffered by any third party resulting from use of the Report without Palmer's express written permission. Should the project design change following issuance of the Report, Palmer must be provided the opportunity to review and revise the Report in light of such alteration or variation.

Attachment:

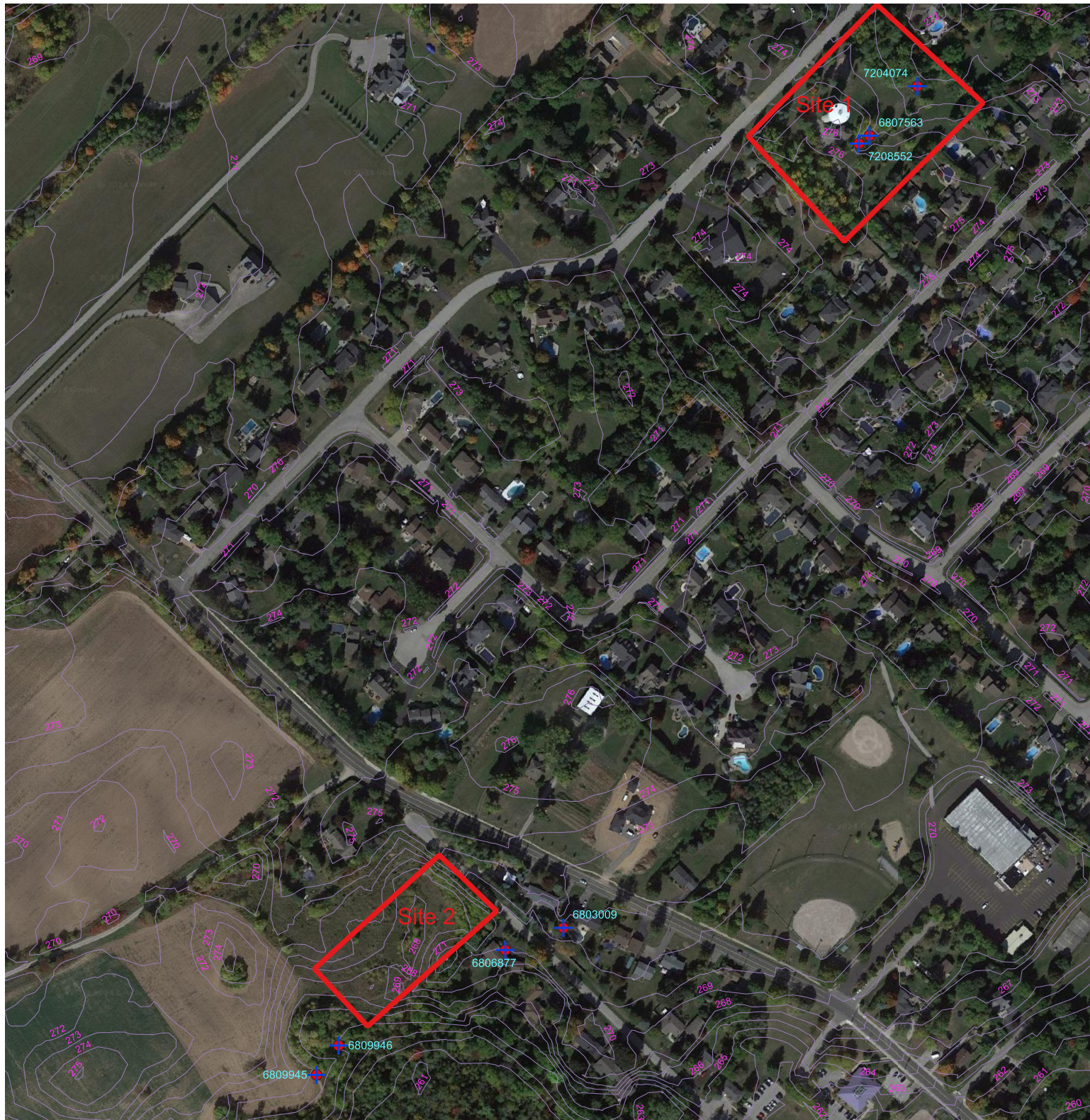
Figure 1. Site Investigation Plan

Figure 2. Surficial Geology




Figure 3. Source Water Protection

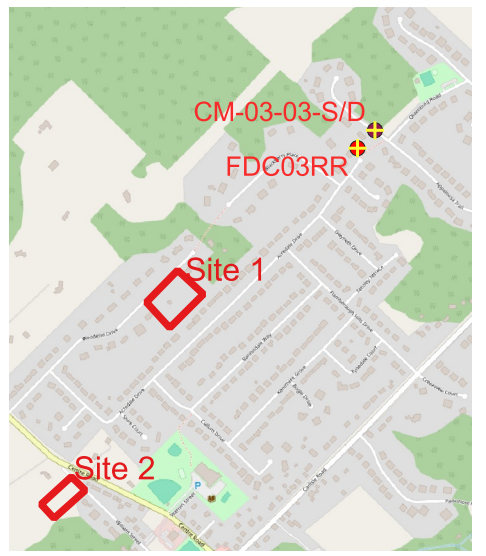
Appendix A: Well Records and Well Logs

Figures



LEGEND

-  Storage Facility
-  Well Record
-  Top Contours (1 m)




100 100 m

North American Datum 1983
 Universal Transverse Mercator Projection Zone 17

Scale: 1:4,000
 Page Size: Letter (11 x 8.5 inches)

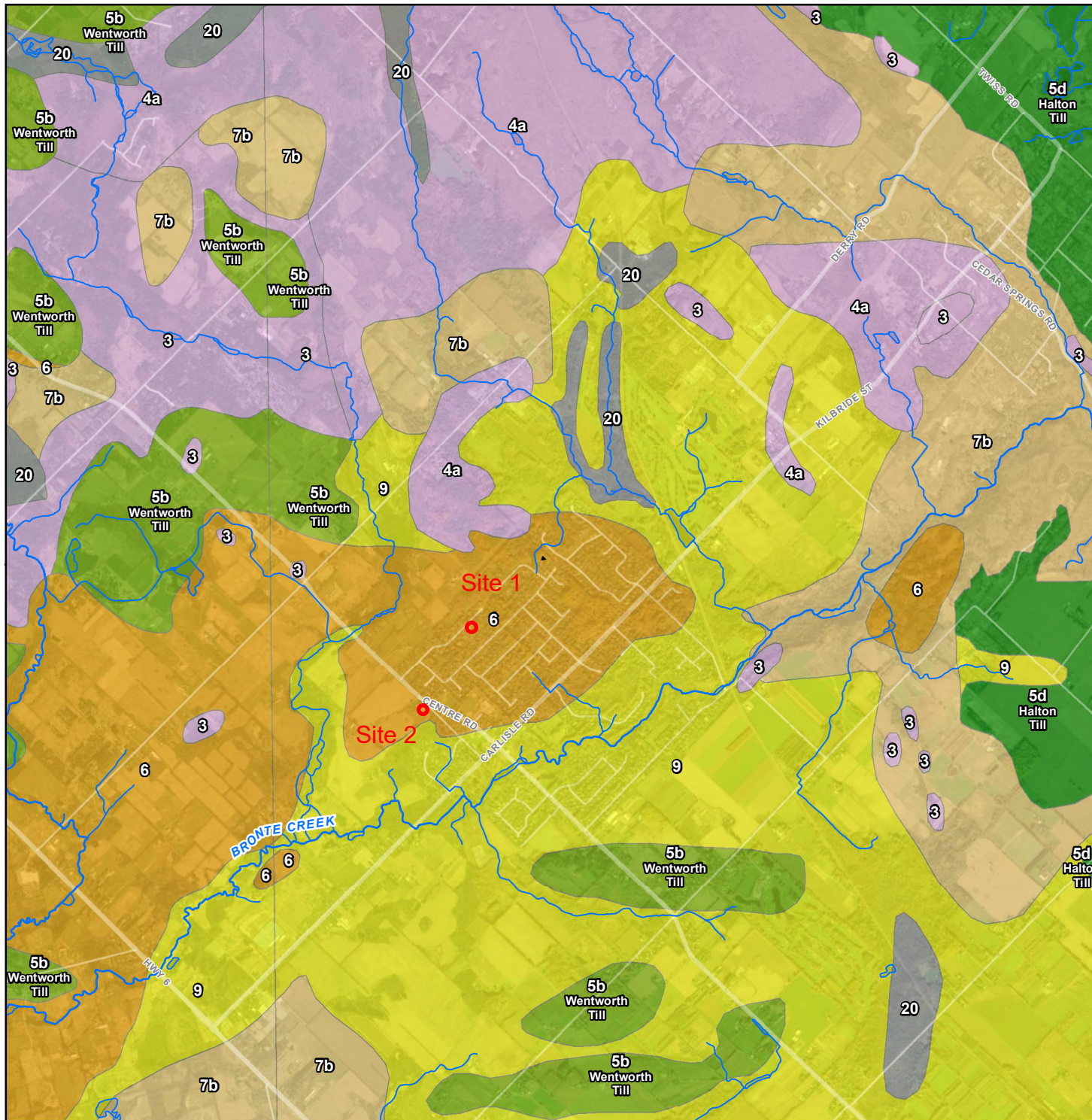
Drawn: FL
 Checked: JC
 Date: Dec 2, 2023

Source Notes:
 Basement - Google Satellite Image
 Topographical Contours - Conservation Halton



CLIENT	R.V. Anderson Associates Limited
PROJECT	Carlisle Water Storage EA
TITLE	Site Investigation Plan

 PART OF SLR	REF. NO: 2108704
	Figure 1



LEGEND

Subject Site
~ Watercourse¹

Surficial Geology²

Phanerozoic / Cenozoic / Quaternary / Recent

20: Organic deposits (peat, muck, marl)

Phanerozoic / Cenozoic / Quaternary / Pleistocene

9: Coarse-textured glaciolacustrine deposits (sand, gravel, minor silt and clay)

7b: Glaciofluvial deposits (Gravelly deposits)

6: Ice-contact stratified deposits (sand and gravel, minor silt, clay and till)

5b: Till (Stone-poor, sandy silt to silty sand-textured till on Paleozoic terrain)

5d: Till (Clay to silt-textured till [derived from glaciolacustrine deposits or shale])

Phanerozoic / Paleozoic

4a: Bedrock-drift complex in Paleozoic terrain (Primarily till cover)

3: Paleozoic bedrock

1. LIO/MNRF
 2. Ontario Geological Survey 2010 (Mapped at 1:50,000). Surficial geology of southern Ontario; Ontario Geological Survey, Miscellaneous Release- Data

0 200 400 800 1,200 1,600 2,000
 METRES

North American Datum 1983
 Universal Transverse Mercator Projection Zone 17

Scale: 1:40,000
 Page Size: Letter (8.5 x 11 inches)

Drawn: CV
 Checked: LB
 Date: Mar 19, 2023

Source Notes:
 Imagery (2021) provided by Esri basemap service.
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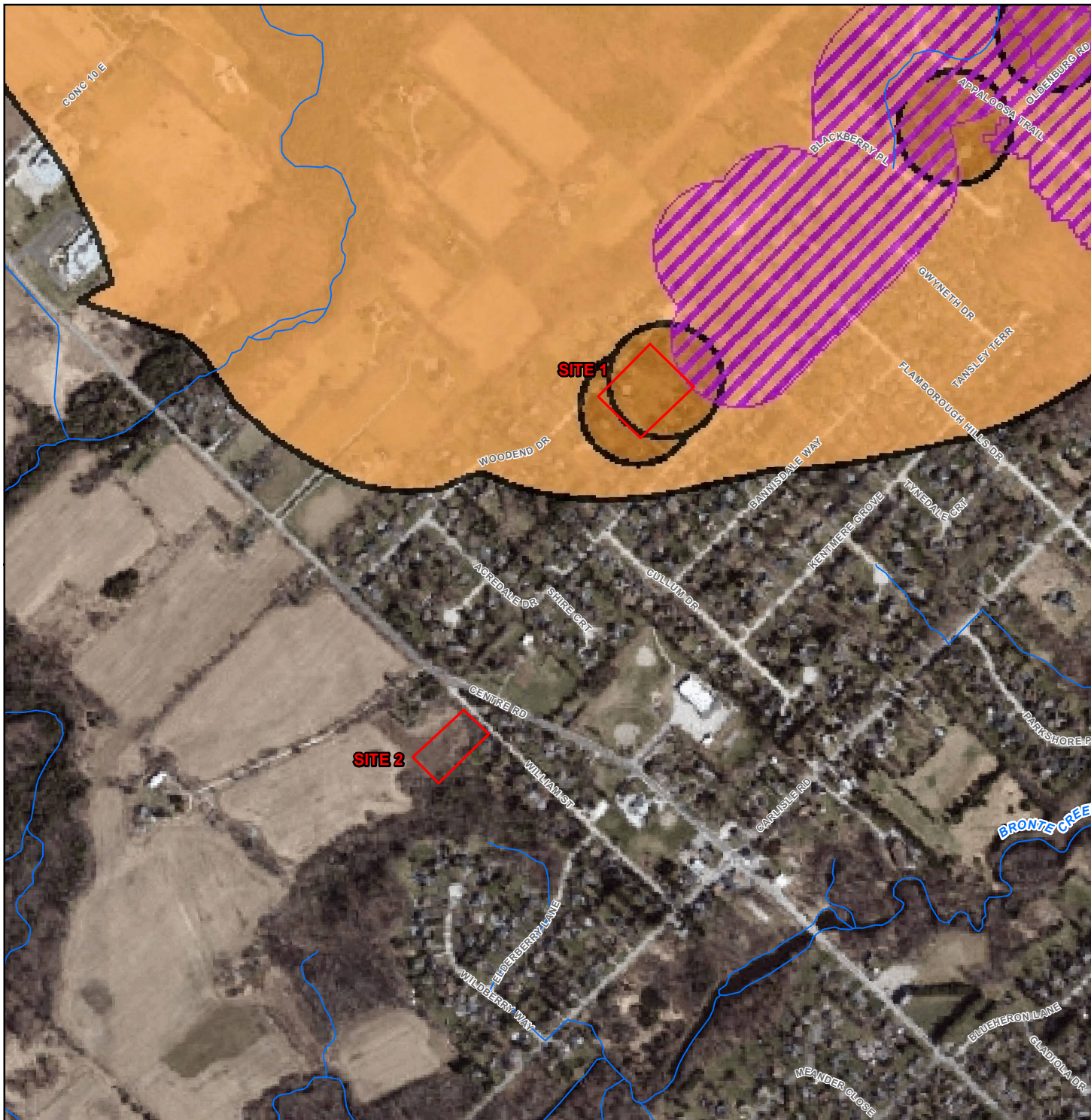
CLIENT
 R.V. Anderson Associates

PROJECT
 Carlisle Water Storage EA

TITLE
Surficial Geology

Palmer™

REF. NO. 2108704
Figure 2



LEGEND

- Subject Site
- ~ Watercourse¹

Source Water Protection²

- Wellhead Protection Area - A
- Wellhead Protection Area - B
- WHPA Groundwater Under Direct Influence (WHPA-E)

1. LIO/MNRF
 2. Source Protection Information Atlas, MECP © King's Printer for Ontario 2023

0 50 100 200 300 400 500
 METRES

North American Datum 1983
 Universal Transverse Mercator Projection Zone 17

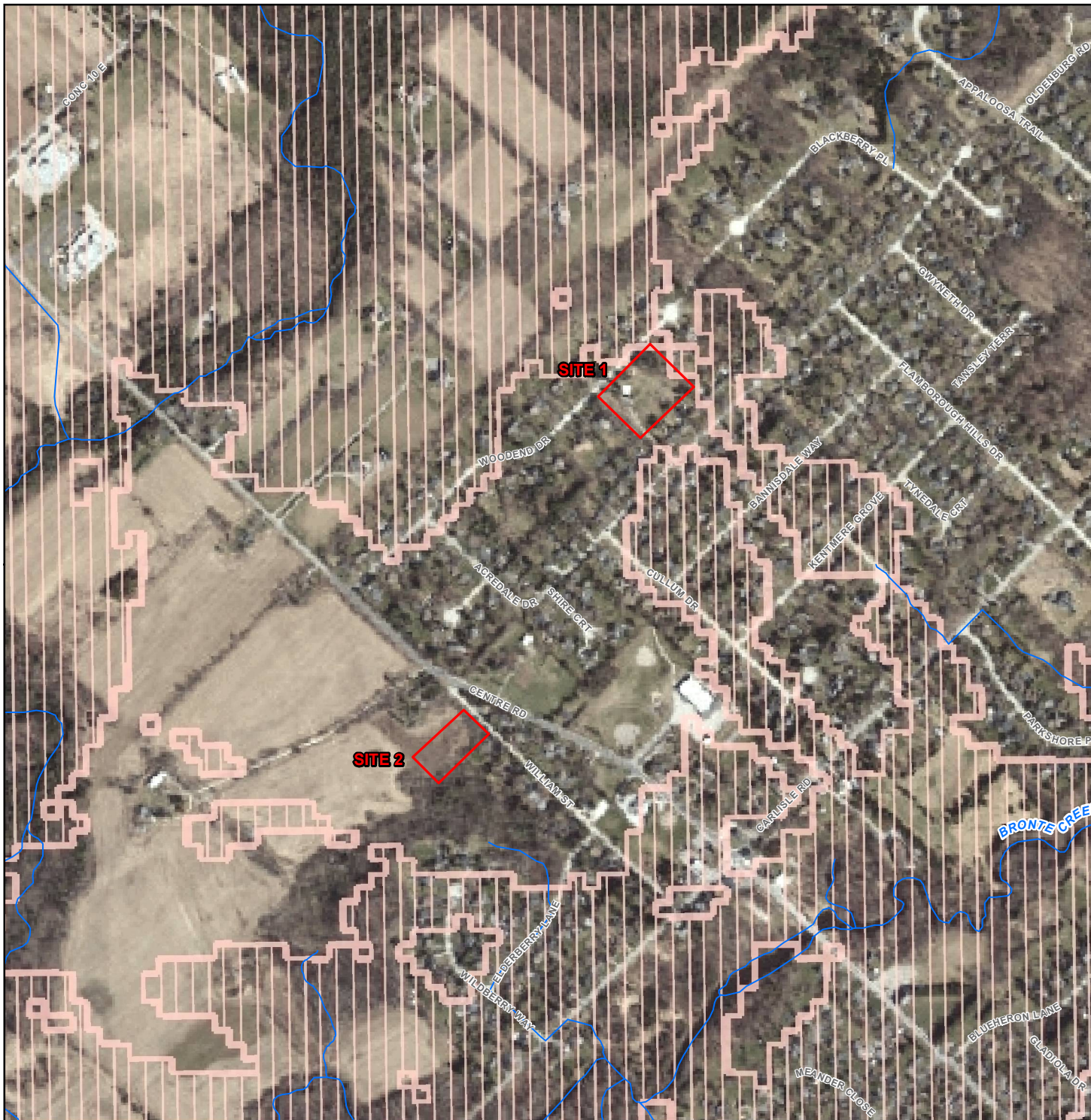
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Drawn: CV
 Checked: FL
 Date: Dec 1, 2023

Source Notes:
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↑
 NORTH

CLIENT	R.V. Anderson Associates
PROJECT	Carlisle Water Storage Facility, Hamilton, ON
TITLE	Source Water Protection
Palmer <small>PART OF</small> SLR	REF. NO. 2108704
Figure 3a	



LEGEND

- Subject Site
- ~ Watercourse¹

Source Water Protection²

- Highly Vulnerable Aquifer

1. LIO/MNRF
 2. Source Protection Information Atlas, MECP © King's Printer for Ontario 2023

0 50 100 200 300 400 500
 METRES

North American Datum 1983
 Universal Transverse Mercator Projection Zone 17

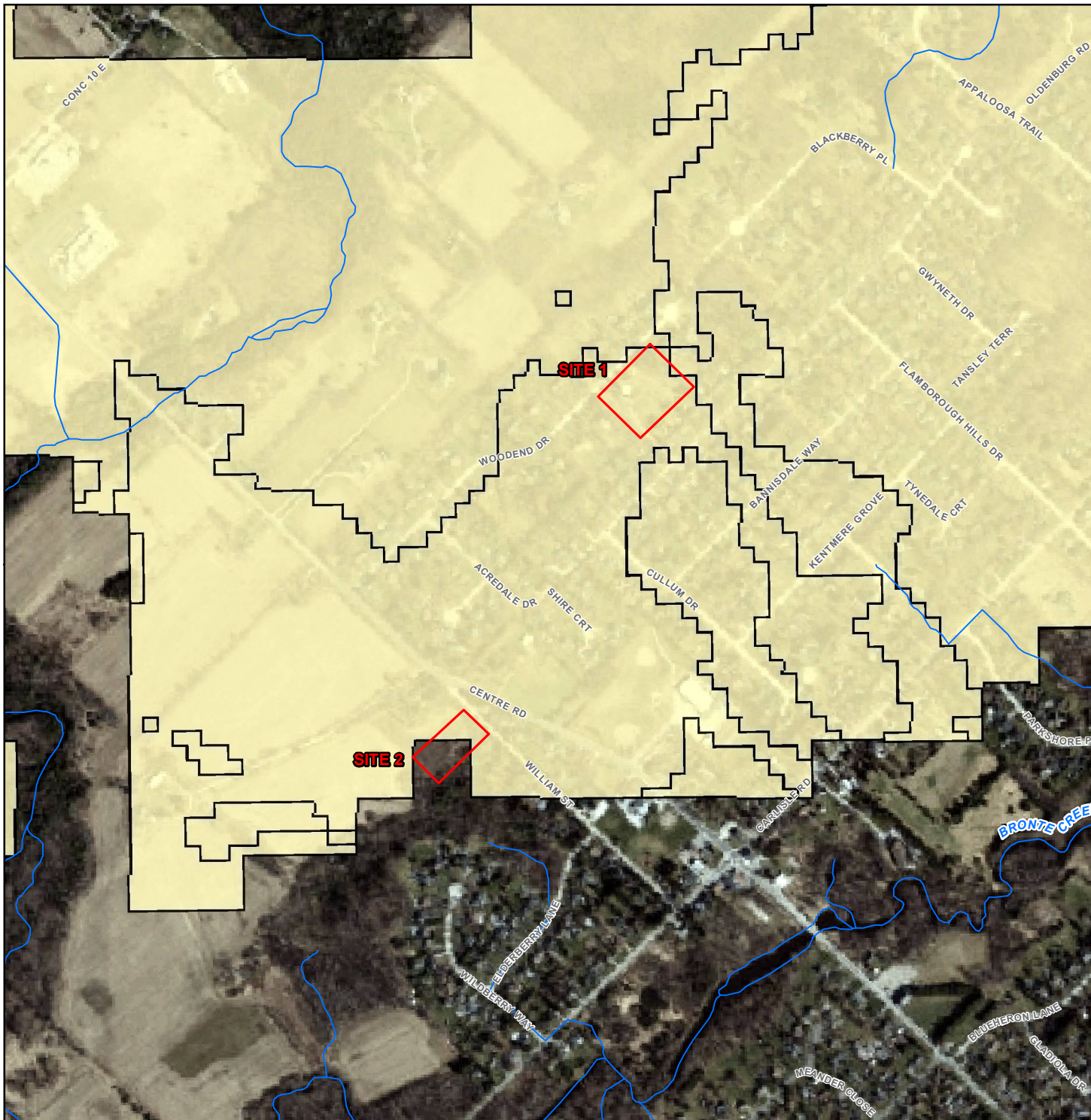
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 Page Size: Letter (8.5 x 11 inches)

Drawn: CV
 Checked: FL
 Date: Dec 1, 2023

Source Notes:
 Imagery (2021) provided by Esri basemap service.
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↑
 NORTH

CLIENT	R.V. Anderson Associates
PROJECT	Carlisle Water Storage Facility, Hamilton, ON
TITLE	Source Water Protection
REF. NO.	2108704-2b-1
Palmer <small>PART OF</small> SLR	Figure 3b



LEGEND

- Subject Site
- ~ Watercourse¹

Source Water Protection²

Significant Groundwater Recharge Area

- Score - N/A

1. LIO/MNRF
 2. *Source Protection Information Atlas*, MECP © King's Printer for Ontario 2023

0 50 100 200 300 400 500
METRES

North American Datum 1983
 Universal Transverse Mercator Projection Zone 17

Scale: 1:10,000
 Page Size: Letter (8.5 x 11 inches)

Drawn: CV
 Checked: FL
 Date: Dec 1, 2023

NORTH

Source Notes:
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CLIENT	R.V. Anderson Associates
PROJECT	Carlisle Water Storage Facility, Hamilton, ON
TITLE	Source Water Protection
REF. NO.	2108704-2c-1

Figure 3c

Appendix A

MECP Water Well Records

Well Logs (FDC03RR, CM-03-03S/D)



WATER WELL RECORD

30M/5e

Water management in Ontario

1. PRINT ONLY IN SPACES PROVIDED
2. CHECK CORRECT BOX WHERE APPLICABLE

11

6807563

MUNICIPAL

68005

CON.

Cdn

09

COUNTY OR DISTRICT Wentworth	TOWNSHIP, BOROUGH, CITY, TOWN, VILLAGE East Flamboro	CON., BLOCK, TRACT, SURVEY, ETC. 9	LOT 806
DATE COMPLETED 27 Aug 70			DATE COMPLETED 27 Aug 70
G 05680		RC 4	ELEVATION 0900
RC 5		BASIN CODE 24	

LOG OF OVERBURDEN AND BEDROCK MATERIALS (SEE INSTRUCTIONS)

GENERAL COLOUR	MOST COMMON MATERIAL	OTHER MATERIALS	GENERAL DESCRIPTION	DEPTH - FEET	
				FROM	TO
Brown	Sand	Boulders		0	15
Brown	Sand	Gravelly		15	41 1/2
Brown	Limestone			41 1/2	80
Grey	Limestone			80	84
Brown	Limestone			84	97
White	Limestone			97	114
Grey	Limestone			114	125
Blue	Shale			125	130
APL					

31	001560913	004260911	0080615	0084215	0097615	0114115	1
32	0125215	0130317					

41 WATER RECORD

WATER FOUND AT FEET	KIND OF WATER
0051	1 <input checked="" type="checkbox"/> FRESH 3 <input type="checkbox"/> SULPHUR 2 <input type="checkbox"/> SALTY 4 <input type="checkbox"/> MINERAL
15-18	1 <input type="checkbox"/> FRESH 3 <input type="checkbox"/> SULPHUR 2 <input type="checkbox"/> SALTY 4 <input type="checkbox"/> MINERAL
20-23	1 <input type="checkbox"/> FRESH 3 <input type="checkbox"/> SULPHUR 2 <input type="checkbox"/> SALTY 4 <input type="checkbox"/> MINERAL
25-28	1 <input type="checkbox"/> FRESH 3 <input type="checkbox"/> SULPHUR 2 <input type="checkbox"/> SALTY 4 <input type="checkbox"/> MINERAL
30-33	1 <input type="checkbox"/> FRESH 3 <input type="checkbox"/> SULPHUR 2 <input type="checkbox"/> SALTY 4 <input type="checkbox"/> MINERAL

51 CASING & OPEN HOLE RECORD

INSIDE DIAM. INCHES	MATERIAL	WALL THICKNESS INCHES	DEPTH - FEET	
			FROM	TO
5 1/4	STEEL	1/88	0	42
06	GALVANIZED			42
	CONCRETE			0130
	OPEN HOLE			

SCREEN

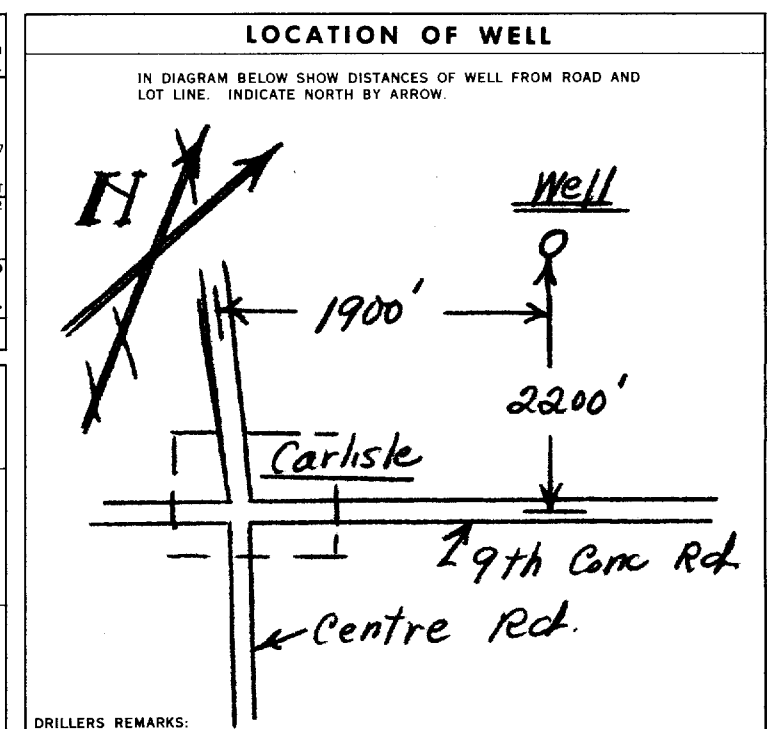
SIZE(S) OF OPENING (SLOT NO.)	DIAMETER	LENGTH
	INCHES	FEET
MATERIAL AND TYPE	DEPTH TO TOP OF SCREEN	
	INCHES	FEET

61 PLUGGING & SEALING RECORD

DEPTH SET AT - FEET	MATERIAL AND TYPE (CEMENT GROUT, LEAD PACKER, ETC.)
FROM TO	
10-13	
14-17	
18-21	
22-25	
26-29	
30-33	

71 PUMPING TEST

PUMPING TEST METHOD 1 <input checked="" type="checkbox"/> PUMP 2 <input type="checkbox"/> BAILER	PUMPING RATE 0025 GPM	DURATION OF PUMPING 01 HOURS 00 MINS.
STATIC LEVEL 038 FEET	WATER LEVEL END OF PUMPING 041 FEET	WATER LEVELS DURING PUMPING
15 MINUTES 040 FEET	30 MINUTES 041 FEET	45 MINUTES 041 FEET
60 MINUTES 041 FEET		
IF FLOWING, GIVE RATE	PUMP INTAKE SET AT	WATER AT END OF TEST
	120 GPM	1 <input checked="" type="checkbox"/> CLEAR 2 <input type="checkbox"/> CLOUDY
RECOMMENDED PUMP TYPE 1 <input type="checkbox"/> SHALLOW 2 <input checked="" type="checkbox"/> DEEP	RECOMMENDED PUMP SETTING 120 FEET	RECOMMENDED PUMPING RATE 88 GPM
50-53 008.3 GPM./FT. SPECIFIC CAPACITY		



FINAL STATUS OF WELL

1 <input checked="" type="checkbox"/> WATER SUPPLY	5 <input type="checkbox"/> ABANDONED, INSUFFICIENT SUPPLY
2 <input type="checkbox"/> OBSERVATION WELL	6 <input type="checkbox"/> ABANDONED, POOR QUALITY
3 <input type="checkbox"/> TEST HOLE	7 <input type="checkbox"/> UNFINISHED
4 <input type="checkbox"/> RECHARGE WELL	

WATER USE

1 <input type="checkbox"/> DOMESTIC	5 <input type="checkbox"/> COMMERCIAL
2 <input type="checkbox"/> STOCK	6 <input type="checkbox"/> MUNICIPAL
3 <input type="checkbox"/> IRRIGATION	7 <input checked="" type="checkbox"/> PUBLIC SUPPLY
4 <input type="checkbox"/> INDUSTRIAL	8 <input type="checkbox"/> COOLING OR AIR CONDITIONING
<input type="checkbox"/> OTHER	9 <input type="checkbox"/> NOT USED

METHOD OF DRILLING

1 <input checked="" type="checkbox"/> TABLE TOOL	6 <input type="checkbox"/> BORING
2 <input type="checkbox"/> ROTARY (CONVENTIONAL)	7 <input type="checkbox"/> DIAMOND
3 <input type="checkbox"/> ROTARY (REVERSE)	8 <input type="checkbox"/> JETTING
4 <input type="checkbox"/> ROTARY (AIR)	9 <input type="checkbox"/> DRIVING
5 <input type="checkbox"/> AIR PERCUSSION	

CONTRACTOR

NAME OF WELL CONTRACTOR C.J. Wallis	LICENCE NUMBER 5417
ADDRESS RR #2, Stoney Creek	
NAME OF DRILLER OR BORER Same	LICENCE NUMBER
SIGNATURE OF CONTRACTOR Louise J. Wallis	SUBMISSION DATE DAY 28 MO 12 YR 70

OFFICE USE ONLY

DATA SOURCE 1	CONTRACTOR 5417	DATE RECEIVED 160970
DATE OF INSPECTION	INSPECTOR	
REMARKS: Well in field - distance 1900' from Carlisle road - 2200' from 19th Conc Rd.		P E
		WI

Measurements recorded in: Metric Imperial

Well Owner's Information

First Name CITY OF HAMILTON	Last Name / Organization	E-mail Address	<input type="checkbox"/> Well Constructed by Well Owner
Mailing Address (Street Number/Name) 120 KING ST. W.	Municipality HAMILTON	Province ONT	Postal Code L8P4V2
		Telephone No. (inc. area code) 9055462424	

Well Location

Address of Well Location (Street Number/Name) ACREDALE DR.	Township EAST FLAMBOROUGH	Lot 06	Concession 09
County/District/Municipality WENTWORTH	City/Town/Village CARLISLE	Province Ontario	Postal Code
UTM Coordinates NAD 83 Zone 17 Easting 582447 Northing 4805928	Municipal Plan and Sublot Number	Other	

Overburden and Bedrock Materials/Abandonment Sealing Record (see instructions on the back of this form)

General Colour	Most Common Material	Other Materials	General Description	Depth (m/ft) From To
		RE: WELL FDC02	CONVERT EXISTING 10" CASING TO 12" BARREL PITLESS ADAPTER WITH VERMIN PROOF LID	

Annular Space		
Depth Set at (m/ft) From To	Type of Sealant Used (Material and Type)	Volume Placed (m³/ft³)

Method of Construction	Well Use
<input type="checkbox"/> Cable Tool <input type="checkbox"/> Rotary (Conventional) <input type="checkbox"/> Rotary (Reverse) <input type="checkbox"/> Boring <input type="checkbox"/> Air percussion <input type="checkbox"/> Other, specify _____	<input checked="" type="checkbox"/> Public <input type="checkbox"/> Domestic <input type="checkbox"/> Livestock <input type="checkbox"/> Irrigation <input type="checkbox"/> Industrial <input type="checkbox"/> Other, specify _____
<input type="checkbox"/> Diamond <input type="checkbox"/> Jetting <input type="checkbox"/> Driving <input type="checkbox"/> Digging	<input type="checkbox"/> Commercial <input type="checkbox"/> Municipal <input type="checkbox"/> Test Hole <input type="checkbox"/> Cooling & Air Conditioning <input type="checkbox"/> Not used <input type="checkbox"/> Dewatering <input type="checkbox"/> Monitoring

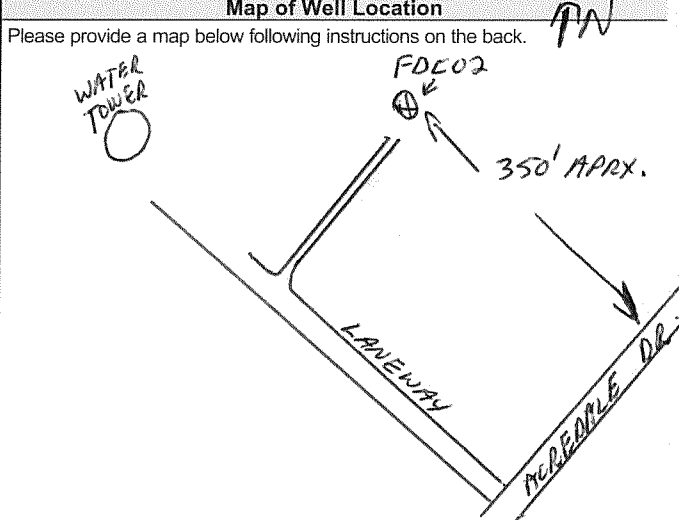
Construction Record - Casing				Status of Well	
Inside Diameter (cm/in)	Open Hole OR Material (Galvanized, Fibreglass, Concrete, Plastic, Steel)	Wall Thickness (cm/in)	Depth (m/ft)		<input type="checkbox"/> Water Supply <input type="checkbox"/> Replacement Well <input type="checkbox"/> Test Hole <input type="checkbox"/> Recharge Well <input type="checkbox"/> Dewatering Well <input type="checkbox"/> Observation and/or Monitoring Hole <input checked="" type="checkbox"/> Alteration (Construction) <input type="checkbox"/> Abandoned, Insufficient Supply <input type="checkbox"/> Abandoned, Poor Water Quality <input type="checkbox"/> Abandoned, other, specify _____ <input type="checkbox"/> Other, specify _____
			From	To	
12	STEEL	.385	+2.5	7.7	

Construction Record - Screen					
Outside Diameter (cm/in)	Material (Plastic, Galvanized, Steel)	Slot No.	Depth (m/ft)		<input type="checkbox"/> Other, specify _____
			From	To	

Water Details		Hole Diameter	
Water found at Depth (m/ft) <input type="checkbox"/> Gas <input type="checkbox"/> Other, specify _____	Kind of Water: <input type="checkbox"/> Fresh <input type="checkbox"/> Untested	Depth (m/ft) From To	Diameter (cm/in)
Water found at Depth (m/ft) <input type="checkbox"/> Gas <input type="checkbox"/> Other, specify _____	Kind of Water: <input type="checkbox"/> Fresh <input type="checkbox"/> Untested		
Water found at Depth (m/ft) <input type="checkbox"/> Gas <input type="checkbox"/> Other, specify _____	Kind of Water: <input type="checkbox"/> Fresh <input type="checkbox"/> Untested		

Well Contractor and Well Technician Information			
Business Name of Well Contractor W. D. Hopper and Sons Ltd.	Well Contractor's Licence No. 2604		
Business Address (Street Number/Name) RR#2, #32 Harpurhey Rd.	Municipality Seaforth		
Province Ont	Postal Code N0K1V0	Business E-mail Address wdhopper@tcc.on.ca	
Bus. Telephone No. (inc. area code) 5195221737	Name of Well Technician (Last Name, First Name) PAUL HOPPER		
Well Technician's Licence No. 2577	Signature of Technician and/or Contractor	Date Submitted Y 21/06/2013 D D	

Results of Well Yield Testing				
After test of well yield, water was: <input type="checkbox"/> Clear and sand free <input type="checkbox"/> Other, specify _____	Draw Down		Recovery	
	Time (min)	Water Level (m/ft)	Time (min)	Water Level (m/ft)
If pumping discontinued, give reason: Pump intake set at (m/ft) Pumping rate (l/min / GPM) Duration of pumping ____ hrs + ____ min Final water level end of pumping (m/ft) If flowing give rate (l/min / GPM) Recommended pump depth (m/ft) Recommended pump rate (l/min / GPM) Well production (l/min / GPM) Disinfected? <input checked="" type="checkbox"/> Yes <input type="checkbox"/> No	Static Level			
	1		1	
	2		2	
	3		3	
	4		4	
	5		5	
10		10		
15		15		
20		20		
25		25		
30		30		
40		40		
50		50		
60		60		



Comments:
PUMP BY W.D.HOPPER

Well owner's information package delivered <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No	Date Package Delivered Y Y Y Y M M D D Y 12/06/2013 D D	Ministry Use Only Audit No. z 151351 Received 02 2013
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Measurements recorded in: Metric Imperial

Page of

Well Owner's Information

First Name, Last Name / Organization, E-mail Address, CITY OF HAMILTON, Mailing Address (Street Number/Name), Municipality, Province, Postal Code, Telephone No. (inc. area code)

Well Location

Address of Well Location (Street Number/Name), Township, Lot, Concession, County/District/Municipality, City/Town/Village, Province, Postal Code, UTM Coordinates, Zone, Easting, Northing, Municipal Plan and Sublot Number, Other

Overburden and Bedrock Materials/Abandonment Sealing Record (see instructions on the back of this form)

Table with columns: General Colour, Most Common Material, Other Materials, General Description, Depth (m/ft) From, To. Rows include: PACK AND SEAL EXISTING BEDROCK, OVERDRILL EXISTING 6" CASING, REMOVE PACK & SEAL THEN FLUSH, BEDROCK, HEAVY DUTY DRIVE SHOE INSTALL, ON END OF CASING.

Annular Space table with columns: Depth Set at (m/ft) From, To; Type of Sealant Used (Material and Type); Volume Placed (m³/ft³). Row: 6, 41, NEET CEMENT, 400GA.

Method of Construction and Well Use checkboxes. Method of Construction includes Cable Tool, Rotary (Conventional/Reverse), Boring, Air percussion, Other. Well Use includes Public, Commercial, Not used, Domestic, Municipal, Dewatering, Livestock, Test Hole, Monitoring, Irrigation, Cooling & Air Conditioning, Industrial, Other.

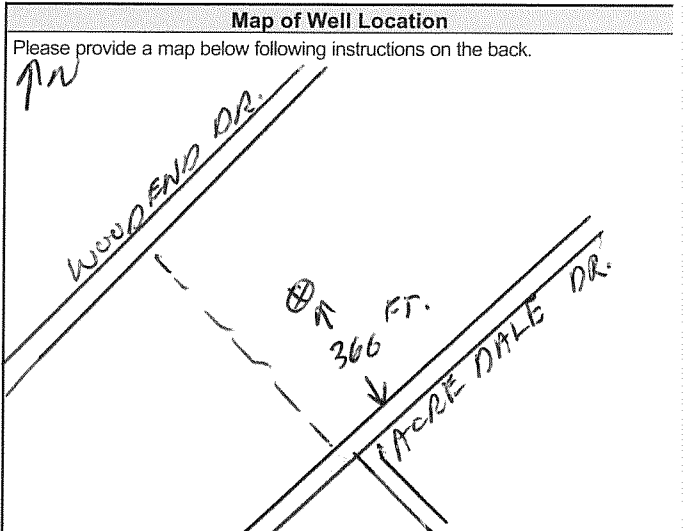
Construction Record - Casing table with columns: Inside Diameter (cm/in), Open Hole OR Material, Wall Thickness (cm/in), Depth (m/ft) From, To, Status of Well. Row: 6.2, STEEL, .219, +3, 41, Alteration Construction.

Construction Record - Screen table with columns: Outside Diameter (cm/in), Material (Plastic, Galvanized, Steel), Slot No., Depth (m/ft) From, To, Status of Well.

Water Details and Hole Diameter tables. Water Details: Water found at Depth (m/ft), Kind of Water (Fresh, Untested, Gas, Other). Hole Diameter: Depth (m/ft) From, To, Diameter (cm/in). Row: 0, 41, 13.

Well Contractor and Well Technician Information. Business Name of Well Contractor: W. D. Hopper and Sons Ltd. Well Contractor's Licence No.: 2604. Business Address: RR#2, #32 Harpurhey Rd. Business E-mail Address: wdhopper@tec-on.ca. Name of Well Technician: ALLAN HOPPER. Signature of Technician and/or Contractor: [Signature]. Date Submitted: 12/08/2013.

Results of Well Yield Testing table. Columns: Draw Down (Time (min), Water Level (m/ft)), Recovery (Time (min), Water Level (m/ft)). Rows show pumping rate (168 L/S), duration (8 hrs), final water level (43.73), and flow rate (168 l/min / GPM).



Comments: WELL# FDC01,PUMP & PITLESS, W.D.HOPPER, MEASURED FROM TOG. Ministry Use Only: Audit No. z 151373. Date Package Delivered: 12/08/2013. Date Work Completed: 01/08/2013.

UTM 117Z 582120E

CON 1X
L 18
CODED



6806877

DIVISION OF
WATER RESOURCES
JAN 9 1969
ONTARIO WATER
RESOURCES COMMISSION

4R 48 05 070N

The Ontario Water Resources Commission Act

WATER WELL RECORD

Elev. 648 08 85

Basin 24
County or District *North York*

Township, Village, Town or City *Flamborough*

Con. 9 IX Lot 7 8

Date completed 12 Nov 68
(day month year)

Owner [Redacted]
(print in block letters)

Address *R.R. 2 Watford
Ontario*

Casing and Screen Record

Pumping Test

Inside diameter of casing *6 1/4*
Total length of casing ~~66~~ *66*
Type of screen _____
Length of screen _____
Depth to top of screen _____
Diameter of finished hole *6 1/4*

Static level *29*
Test-pumping rate *15* G.P.M.
Pumping level *60*
Duration of test pumping *1 hr*
Water clear or cloudy at end of test *clear*
Recommended pumping rate *10* G.P.M.
with pump setting of *60* feet below ground surface

Well Log

Water Record

Overburden and Bedrock Record

	From ft.	To ft.	Depth(s) at which water(s) found	Kind of water (fresh, salty, sulphur)
<i>Stony clay fill</i>	0	8	<i>67</i>	<i>fresh</i>
<i>Sandy clay some gravel</i>	8	35		
<i>dirty sand</i>	35	60		
<i>clay hardpan</i>	60	64		
<i>Bed Rock</i>	64	68		

For what purpose(s) is the water to be used? *house*

Is well on upland, in valley, or on hillside? *upland*

Drilling or Boring Firm *W. Pockham*

Address *R.R. 2 Ormeau*

Licence Number *2794*

Name of Driller or Borer *W. Pockham*

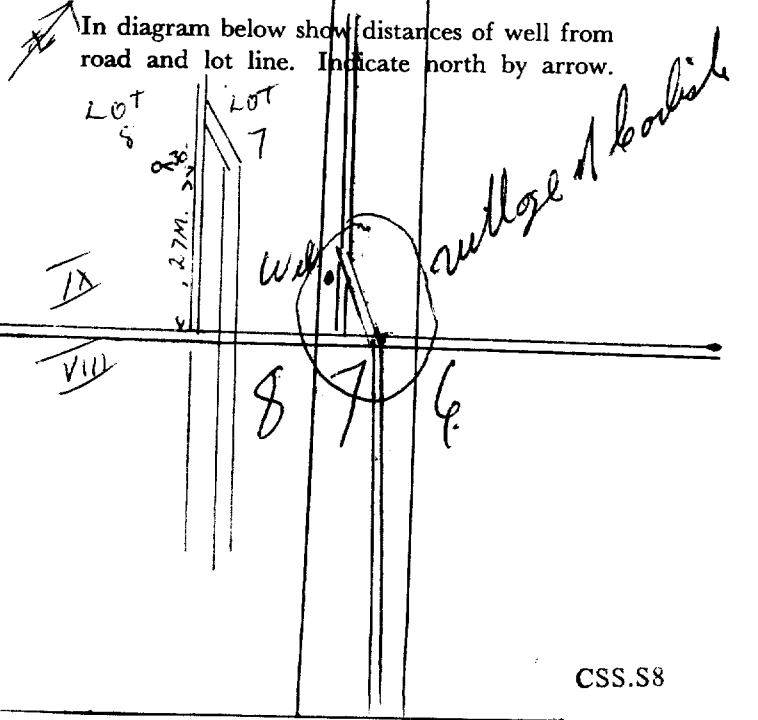
Address *R.R. 2 Ormeau*

Date *Nov 12/68*

Wesley Pockham
(Signature of Licensed Drilling or Boring Contractor)

Form 7
OWRC COPY *Large new house across the road from a duplex*

Location of Well



14/M

1. PRINT ONLY IN SPACES PROVIDED
 2. CHECK CORRECT BOX WHERE APPLICABLE

11

6809946

MUNICIPALITY 68.005

CON. CCM

09

COUNTY OR DISTRICT: **Hamilton-Wentworth** TOWNSHIP, BOROUGH, CITY, TOWN, VILLAGE: **E. Flamboro** CON. BLOCK, TRACT, SURVEY, ETC: **9** LOT: **808**

City, Town, Village: **Hamilton, Ontario** REGISTERED MEMBER OF ONTARIO: **386392 LTD.** DATE COMPLETED: DAY **24** MO **May** YR **79**

NG: **05.000** RC: **4** ELEVATION: **0860** RC: **4** BASIN CODE: **24**

LOG OF OVERBURDEN AND BEDROCK MATERIALS (SEE INSTRUCTIONS)

GENERAL COLOUR	MOST COMMON MATERIAL	OTHER MATERIALS	GENERAL DESCRIPTION	DEPTH - FEET	
				FROM	TO
	Top Soil			0	1
M. Brown	Clay	Stones		1	10
D. Brown	Clay Sand	Stones		10	20
D. Brown	Clay Sand	Gravels		20	40
M. Brown	Sand	Gravels		40	52
		Gravels		52	55
	Brown Clay Sand	Gravels		55	60
M. Gray	Rock			60	70
D. Gray	Rock			70	87
	Blue Shales			87	96
	Red Shales			96	100
T.W. 1					

31 0001 02 001060512 00206052812 00406052811 005242811 0055 11 1

32 00606052811 007021278 008721265 0096317 0100717

41 WATER RECORD

WATER FOUND AT - FEET	KIND OF WATER
10-13	1 <input checked="" type="checkbox"/> FRESH 3 <input type="checkbox"/> SULPHUR 2 <input type="checkbox"/> SALTY 4 <input type="checkbox"/> MINERAL
15-18	1 <input type="checkbox"/> FRESH 3 <input type="checkbox"/> SULPHUR 2 <input type="checkbox"/> SALTY 4 <input type="checkbox"/> MINERAL
20-23	1 <input type="checkbox"/> FRESH 3 <input type="checkbox"/> SULPHUR 2 <input type="checkbox"/> SALTY 4 <input type="checkbox"/> MINERAL
25-28	1 <input type="checkbox"/> FRESH 3 <input type="checkbox"/> SULPHUR 2 <input type="checkbox"/> SALTY 4 <input type="checkbox"/> MINERAL
30-33	1 <input type="checkbox"/> FRESH 3 <input type="checkbox"/> SULPHUR 2 <input type="checkbox"/> SALTY 4 <input type="checkbox"/> MINERAL

51 CASING & OPEN HOLE RECORD

INSIDE DIAM. INCHES	MATERIAL	WALL THICKNESS INCHES	DEPTH - FEET	
			FROM	TO
10-11	1 <input checked="" type="checkbox"/> STEEL 2 <input type="checkbox"/> GALVANIZED 3 <input type="checkbox"/> CONCRETE 4 <input type="checkbox"/> OPEN HOLE	.250	0	0062
17-18	1 <input type="checkbox"/> STEEL 2 <input type="checkbox"/> GALVANIZED 3 <input type="checkbox"/> CONCRETE 4 <input checked="" type="checkbox"/> OPEN HOLE		62	0100
24-25	1 <input type="checkbox"/> STEEL 2 <input type="checkbox"/> GALVANIZED 3 <input type="checkbox"/> CONCRETE 4 <input type="checkbox"/> OPEN HOLE			

SCREEN

SIZE(S) OF OPENING (SLOT NO.)	DIAMETER INCHES	LENGTH FEET
MATERIAL AND TYPE		DEPTH TO TOP OF SCREEN FEET

61 PLUGGING & SEALING RECORD

DEPTH SET AT - FEET		MATERIAL AND TYPE (CEMENT GROUT LEAD PACKER, ETC.)
FROM	TO	
10-13	16-17	
18-21	22-25	
26-29	30-33	

71 PUMPING TEST

PUMPING TEST METHOD: 1 PUMP 2 BAILER

PUMPING RATE: 0040 GPM DURATION OF PUMPING: 04 HOURS 00 MINS

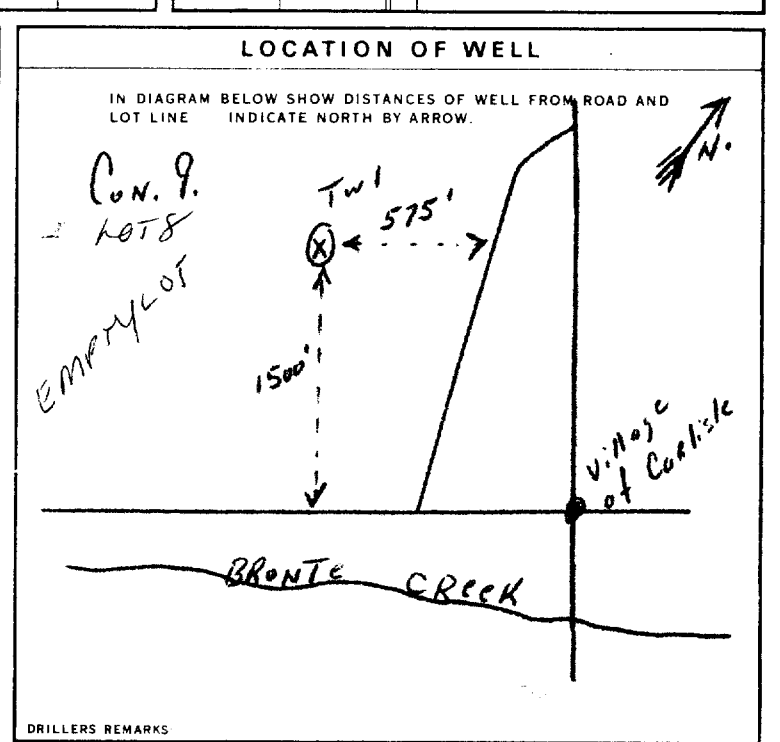
STATIC LEVEL	WATER LEVEL END OF PUMPING	WATER LEVELS DURING			
19-21	22-24	15 MINUTES	30 MINUTES	45 MINUTES	60 MINUTES
-bt	Flowing 004	001	001	001	001

IF FLOWING GIVE RATE: 0001 GPM

RECOMMENDED PUMP TYPE: SHALLOW DEEP

RECOMMENDED PUMP SETTING: 040 FEET

RECOMMENDED PUMPING RATE: 0040 GPM



FINAL STATUS OF WELL 1 WATER SUPPLY

WATER USE 05

METHOD OF DRILLING 1 CABLE TOOL

CONTRACTOR NAME OF WELL CONTRACTOR: **Graham Well Drilling Ltd.** LICENCE NUMBER: **2336**

ADDRESS: **Guelph, Ontario**

NAME OF DRILLER OR BORER: **Jim Hawkins 22 W** LICENCE NUMBER:

SIGNATURE OF CONTRACTOR: *[Signature]* SUBMISSION DATE: DAY **30** MO **May** YR **79**

OFFICE USE ONLY

DATA SOURCE: **1** CONTRACTOR: **1336** DATE RECEIVED: **010679**

DATE OF INSPECTION: INSPECTOR: *[Signature]*

REMARKS:

30m/se



The Ontario Water Resources Act WATER WELL RECORD

1. PRINT ONLY IN SPACES PROVIDED
2. CHECK CORRECT BOX WHERE APPLICABLE

11 6809945 68.005 Cqn 07

COUNTY OR DISTRICT: **Westworth Hamilton** TOWNSHIP, BOROUGH, CITY, TOWN, VILLAGE: **F. Flamboro** CON., BLOCK, TRACT, SURVEY ETC.: **9** **008**

Wilsie, Ontario REGISTERED DIV # **386392** DATE COMPLETED **05** 48-53
DAY **24** MO **May** YR **79**

RC **04980** ELEVATION **4** **0860** RC **4** BASIN CODE **24**

LOG OF OVERBURDEN AND BEDROCK MATERIALS (SEE INSTRUCTIONS)					
GENERAL COLOUR	MOST COMMON MATERIAL	OTHER MATERIALS	GENERAL DESCRIPTION	DEPTH - FEET	
				FROM	TO
	Top soil			0	1
Brown	Clay	Stones		1	10
Brown	Clay Sand	Stones		10	20
Brown	Clay Sand	Gravels		20	40
Brown	Clay Sand	Gravels		40	60
Br. Gray	Rock			60	73
Gray	Rock			73	87
Blue	Shales			87	95
Red	Shales			95	100
T.W. 2					

31 0001 02 001060512 00206052812 00606052811 007361278 008721265 1

32 0095317 0100717 Shoe

41 WATER RECORD

WATER FOUND AT - FEET	KIND OF WATER
0061	1 <input checked="" type="checkbox"/> FRESH 3 <input type="checkbox"/> SULPHUR 2 <input type="checkbox"/> SALTY 4 <input type="checkbox"/> MINERAL
0083	1 <input checked="" type="checkbox"/> FRESH 3 <input type="checkbox"/> SULPHUR 2 <input type="checkbox"/> SALTY 4 <input type="checkbox"/> MINERAL
	1 <input type="checkbox"/> FRESH 3 <input type="checkbox"/> SULPHUR 2 <input type="checkbox"/> SALTY 4 <input type="checkbox"/> MINERAL
	1 <input type="checkbox"/> FRESH 3 <input type="checkbox"/> SULPHUR 2 <input type="checkbox"/> SALTY 4 <input type="checkbox"/> MINERAL
	1 <input type="checkbox"/> FRESH 3 <input type="checkbox"/> SULPHUR 2 <input type="checkbox"/> SALTY 4 <input type="checkbox"/> MINERAL

51 CASING & OPEN HOLE RECORD

INSIDE DIAM INCHES	MATERIAL	WALL THICKNESS INCHES	DEPTH - FEET	
			FROM	TO
08"	1 <input checked="" type="checkbox"/> STEEL 2 <input type="checkbox"/> GALVANIZED 3 <input type="checkbox"/> CONCRETE 4 <input type="checkbox"/> OPEN HOLE	.250	0	0061
08"	1 <input type="checkbox"/> STEEL 2 <input type="checkbox"/> GALVANIZED 3 <input type="checkbox"/> CONCRETE 4 <input checked="" type="checkbox"/> OPEN HOLE		61	0100
	1 <input type="checkbox"/> STEEL 2 <input type="checkbox"/> GALVANIZED 3 <input type="checkbox"/> CONCRETE 4 <input type="checkbox"/> OPEN HOLE			

SCREEN

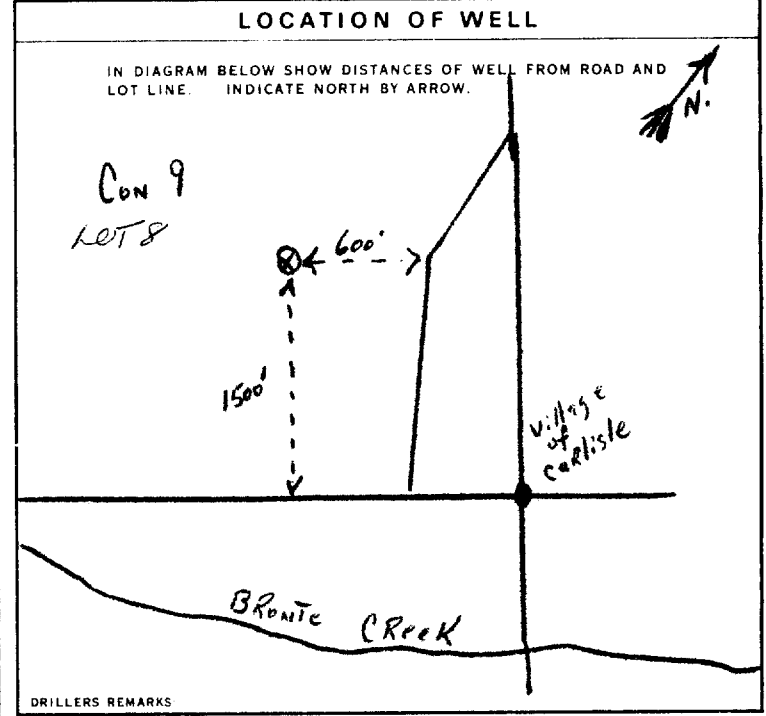
SIZE(S) OF OPENING (SLOT NO.)	DIAMETER	LENGTH
	INCHES	FEET
MATERIAL AND TYPE		DEPTH TO TOP OF SCREEN
		FEET

61 PLUGGING & SEALING RECORD

DEPTH SET AT - FEET		MATERIAL AND TYPE (CEMENT GROUT LEAD PACKER ETC.)
FROM	TO	
10-13	14-17	
18-21	22-25	
26-29	30-33	

71 PUMPING TEST

PUMPING TEST METHOD	PUMPING RATE	DURATION OF PUMPING
1 <input checked="" type="checkbox"/> PUMP 2 <input type="checkbox"/> BAILER	0040 GPM	24 15-16 HOURS 00 17-18 MINS
STATIC LEVEL	WATER LEVEL END OF PUMPING	WATER LEVELS DURING
-bb	004	15 MINUTES 26-28 FEET 30 MINUTES 29-31 FEET 45 MINUTES 32-34 FEET 60 MINUTES 35-37 FEET
IF FLOWING, GIVE RATE	PUMP INTAKE SET AT	WATER AT END OF TEST
0001 GPM	FEET	1 <input checked="" type="checkbox"/> CLEAR 2 <input type="checkbox"/> CLOUDY
RECOMMENDED PUMP TYPE	RECOMMENDED PUMP SETTING	RECOMMENDED PUMPING RATE
<input type="checkbox"/> SHALLOW <input checked="" type="checkbox"/> DEEP	040 FEET	0040 GPM



52 FINAL STATUS OF WELL

1 WATER SUPPLY 5 ABANDONED, INSUFFICIENT SUPPLY
2 OBSERVATION WELL 6 ABANDONED POOR QUALITY
3 TEST HOLE 7 UNFINISHED
4 RECHARGE WELL

53-54 WATER USE

1 DOMESTIC 5 COMMERCIAL
2 STOCK 6 MUNICIPAL
3 IRRIGATION 7 PUBLIC SUPPLY
4 INDUSTRIAL 8 COOLING OR AIR CONDITIONING
9 OTHER 9 NOT USED

57 METHOD OF DRILLING

1 CABLE TOOL 5 BORING
2 ROTARY (CONVENTIONAL) 7 DIAMOND
3 ROTARY (REVERSE) 8 JETTING
4 ROTARY (AIR) 9 DRIVING
5 AIR PERCUSSION

CONTRACTOR

NAME OF WELL CONTRACTOR: **Graham Well Drilling Ltd.** LICENCE NUMBER: **2336**

ADDRESS: **Guelph, Ontario**

NAME OF DRILLER OR BORER: **Jim Hawkins 22W** LICENCE NUMBER:

SIGNATURE OF CONTRACTOR: *R. Hawkins* SUBMISSION DATE: **30 May 79**

OFFICE USE ONLY

DATA SOURCE: **1** CONTRACTOR: **2336** DATE RECEIVED: **010679**

DATE OF INSPECTION: **17 May 1979** INSPECTOR: *[Signature]*

REMARKS:

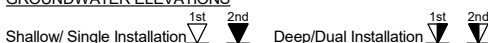
PROJECT: Carlisle Redundant Well
 CLIENT: R.V. Anderson Associates Limited
 PROJECT LOCATION: 84 Acredale Drive Carlisle, ON
 DATUM: Geodetic
 BH LOCATION:

Method: Cased rotary-percussion drilling
 Diameter: 400 mm
 Date: Apr-17-2023 to Apr-20-2023
 REF. NO.: 2108704
 ENCL NO.:

SOIL PROFILE		SAMPLES			GROUND WATER CONDITIONS	ELEVATION	DYNAMIC CONE PENETRATION RESISTANCE PLOT		PLASTIC LIMIT W _p	NATURAL MOISTURE CONTENT w	LIQUID LIMIT W _L	POCKET PEN. (Cu) (kPa)	NATURAL UNIT WT (kN/m ³)	REMARKS AND GRAIN SIZE DISTRIBUTION (%) GR SA SI CL
(m) ELEV DEPTH	DESCRIPTION	STRATA PLOT	NUMBER	TYPE			"N" BLOWS 0.3 m	SHEAR STRENGTH (kPa)						
273.0	Ground Surface						20 40 60 80 100							
0.0	FILL: sandy gravel to gravelly sand, trace clay, subrounded dolostone and sandstone gravel (<30 mm), brown, wet to moist, no stain and no odor. a few rootlets and anthropogenic debris.		1	GR										
269.2	SANDY GRAVEL TO GRAVELLY SAND: trace clay, subrounded to subangular dolostone, limestone and sandstone gravel (10 to 35 mm), brown, damp to dry at top, no stain and no odor. --- clean water was used as drilling fluid.		2	GR										
261.9	SANDY GRAVEL: subrounded to subangular dolostone and precambrian rock gavel (<30 mm), greyish brown, no stain and no odor.		3	GR										
258.5	DOLOSTONE: fine-crystallized, slightly weathered at upper part, a few vugs and solution holes, geyish white.		4	GR										
255.5	LIMESTONE: fine-crystallized, grey. ---fractures.		5	GR										
252.4	LIMESTONE and DOLOSTONE: interlayered limestone and dolostone, fine-crystallized, Vuggy and fossiliferous locally, a few fractures, light grey. --- fractures.		6	GR										
244.1	LIMESTONE: fine-crystallized, thinly bedded, with siltstone interlayers at lower part, grey.		7	GR										
240.6	SHALE: calcareous shale with siltstone interlayers, greenish grey.		8	GR										
230.0	END OF BOREHOLE		9	GR										

Notes:
 Grab samples of cuttings were taken with fine mesh strainer. Loss of fine materials is anticipated. The well log was based on cutting classification and records of construction.

GROUNDWATER ELEVATIONS



GRAPH NOTES

+ 3, × 3: Numbers refer to Sensitivity
 ○ = 3% Strain at Failure

1. SEE ATTACHED WELL LOG FOR SOIL PROFILES FROM WELL LOG 018
 2. DATE: 2023-04-20 BY: J. [unreadable]

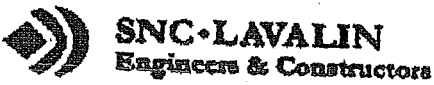
MAY 09 2003 08:53 FR

TO 1633125694164629 F.02/08

CM-03-03d

BOREHOLE LOG				Logged By:	Reviewed By:	Sheet Number:	Test Pit Number:		
Lantech Drilling Services				H. Sangam	Draft	1 of 2	CM-03-03		
Drawing Equipment: CME-75				Drawing Method: SSA and HQ-Core		Start Date: 27 Mar 03	End Date: 28 Mar 03		
Final Hole Depth (m):	Final Well Depth (m):	Hole Diameter (m):	Well Diameter (m):	Pipe Stickup (m):					
27.4	27.37	0.1	0.1	0.50					
Grounds Elevation (m):	TDC Elevation (m):	Depth to Water BTQC (m):	Depth to Water BGS (m):	Groundwater Elevation (m):					
n.a.	Not Surveyed	5.60	5.10	n.a.					
Sample Type Legend: SS Split Spoon Sample AU Auger Sample PS Pioner Sample VA Vane Test Interval GR Grab Sample SP Standard Penetration Test RX Core Sample SH Shelby Tube Sample				Borehole Location Description/Notes: Northing: 4808434 Easting: 563015 Zone 17T					
Sample Type No. and Interval	Blow Counts (N16-cou)	St-Value	Sample Recovery	Elevation (m) Depth (m)	Depth Scale (m)	Material	Stratigraphic Description	Well Materials	Well Construction Details
SS-1		n.a.		0.10	1	TOPSOIL-FILL	Brown, some gravel and clay		Backfill
SS-2		n.a.		3.05	2		Brown, layered, some gravel		
SS-3		n.a.		5.79	3		Brown, layered, some clay, damp		
RX-4		n.a.		5.94	4	BOULDER	-White grey -Water intermittently turning brown and milky white -granite at about 6.24 m to 6.47 m		Demolite Seal 0.20 m Steel Casing
RX-5		n.a.			5				
RX-6		n.a.			6				
RX-6		n.a.			7				
RX-7		n.a.		9.19	8		-Fractured with with occasional -brown soil material in fractures (washed out) by drilling fluid		
RX-8		n.a.		10.97	9	BEDROCK - DOLOSTONE	-grey -fractures at about every 0.15 m		
RX-8		n.a.			10				
RX-9		n.a.		12.19	11		-completely fractured -gravel like curtings		
RX-1		n.a.		12.95	12		-open fractures -filling soil washed out		
RX-1		n.a.		13.44	13		-highly fractured		
RX-1		n.a.		13.67	14		-gravel like curtings -fractures with filling soil wash out -dissolution cavities		

REVISIONS/CHANGES: NONE/NOT IN USE



Project Name: Hamilton Groundwater Study
Project Location: Cantata

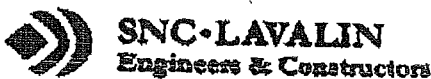
Project Number: 331256

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BOREHOLE LOG		Logged By:	Reviewed By:	Sheet Number:	Test PR Number:				
		H. Sangam	Draft	2 of 2	CM-03-03				
Sample Type Legend: SS Split Spoon Sample AU Auger Sample PS Packer Sample VA Vane Test Interval GR Grab Sample SP Standard Penetration Test RX Core Sample SH Shelby Tube Sample			Borehole Location Description/Notes: Northing: 4806434 Easting: 083010 Zone 17T						
Sample Type, No. and Interval	Blow Counts (N/15 cm)	R-Value	Sample Recovery	Elevation (m) Depth (m)	Depth Scale (m)	Material	Stratigraphic Description	Well Materials	Well Construction Details
RX-1		N.B.		15.24	16		-fractured, open fractures -abundant dissolution cavities		
RX-1		N.B.		17.22	18		-highly fractured -angular gravel like cuttings		
RX-1		N.A.		18.87	19		-micropores -rough core		
RX-1		N.A.		19.76	20		-highly fractured -gravel like cuttings		0.10 m Diameter Open Hole from 13.72 mbgs to 27.37 mbgs
RX-1		N.A.		21.13	21		-micropores -rough core surface		
RX-1		N.B.		22.60	22		-intact rock, no fractures -smooth core surface		
RX-1		N.B.		24.33	23		SHALE -dark grey, very stiff -plastic clay seams starting at about 25.5 mbgs -clay seams at every 0.1 to 0.20 m -becoming greenish with depth		
RX-2		N.B.			24				
RX-2		N.B.			25				
RX-2		N.B.			26				
RX-2		N.B.			27				
				27.37			End of Borehole		

TEST AND NEW CAULSING OF J PLYWOOD LOG BENCH



Project Name: Hamilton Groundwater Study
Project Location: Carleton Place

Project Number: 331256